

#### WEST VALLEY WATER DISTRICT 855 W. Base Line Road Rialto, CA

# ENGINEERING, OPERATIONS AND PLANNING COMMITTEE MEETING AGENDA

#### WEDNESDAY, OCTOBER 9, 2019 - 6:00 PM

**NOTICE IS HEREBY GIVEN** that West Valley Water District has called a meeting of the Engineering and Planning Committee to meet in the Administrative Conference Room, 855 W. Base Line Road, Rialto, CA 92376.

#### 1. CONVENE MEETING

#### 2. PUBLIC PARTICIPATION

The public may address the Board on matters within its jurisdiction. Speakers are requested to keep their comments to no more than three (3) minutes. However, the Board of Directors is prohibited by State Law to take action on items not included on the printed agenda.

#### 3. DISCUSSION ITEMS

- a. Update from Engineering, Operations and Planning Committee
- **b.** Deviation from District Policy Regarding the Installation of Water Facilities.
- c. Water Facilities Master Plan Update.
- **d.** Consider Notice of Completion Recordation for the Reservoir R2-3 Site Improvements and Modifications Project.
- e. Authorization to Approve Change Order No. 3 for the Customer Service Foyer Renovation Project.
- **f.** Consider Release of Overlying Easement on APN 0131-131-02, 08 and 09.
- **g.** Consider Water System Infrastructure Installation and Conveyance Agreement with Calatlantic Group, Inc Casa Grande Avenue and Sierra Avenue Waterline Crossing.
- **h.** Consider Water System Infrastructure Installation and Conveyance Agreement with Lennar Homes of California, Inc for Tract 20250.

#### 4. ADJOURN

#### **DECLARATION OF POSTING:**

I declare under penalty of perjury, that I am employed by the West Valley Water District and posted the foregoing Engineering, Operations and Planning Committee Agenda at the District Offices on October 6, 2019.

Crystal L. Escalera, Board Secretary



#### BOARD OF DIRECTORS ENGINEERING AND PLANNING COMMITTEE STAFF REPORT

DATE: October 9, 2019

TO: Engineering and Planning Committee FROM: Clarence Mansell Jr., General Manager

SUBJECT: DEVIATION FROM DISTRICT POLICY REGARDING THE

INSTALLATION OF WATER FACILITIES

#### **DISCUSSION:**

The Chandi Group USA is requesting District's approval to deviate from District policy of utilizing one of the District's pre-approved contractors for the installation of water facilities and allow their construction team, Black and Gold Builders Group to handle the construction of water facilities necessary to serve their Arco Gas Station and Hotel project. This project is on the northwest corner of Pepper Ave. and Valley Blvd. in the City of Colton. It is their intention to utilize their own construction forces to install approximately 48 feet of 10-inch diameter water main, five 2-inch water services, one 4-inch water service and two 10-inch fire service laterals.

#### **FISCAL IMPACT:**

No fiscal impact.

#### STAFF RECOMMENDATION:

The Board of Directors could approve the Chandi Group USA's request with the following conditions:

- 1) The Chandi Group USA and Black and Gold Builders Group shall provide insurance coverage certificates naming District as an additional insured.
- 2) The Chandi Group USA and Black and Gold Builders Group shall provide a material list submittal to the District for approval prior to construction.
- 3) The Chandi Group USA and Black and Gold Builders Group shall furnish a two-year warranty for the water facilities installed.
- 4) The Chandi Group USA shall deposit, in advance, all required funds and inspection fees.
- 5) The Chandi Group USA and Black and Gold Builders Group shall construct all facilities under District inspection in accordance with District standards.

6) At the completion and acceptance of new water facilities, the Chandi Group USA shall provide the District a document to dedicate all offsite water facilities to District.

Respectfully Submitted,

Clarace C. Manselly

Clarence Mansell Jr, General Manager

LJ:ce



#### BOARD OF DIRECTORS ENGINEERING AND PLANNING COMMITTEE STAFF REPORT

DATE: October 9, 2019

TO: Engineering and Planning Committee FROM: Clarence Mansell Jr., General Manager

SUBJECT: WATER FACILITIES MASTER PLAN UPDATE

#### **BACKGROUND:**

The purpose of a Water Facilities Master Plan ("Plan") is to determine the future water demands and supply requirements, and to identify the water facilities needed to produce, deliver, store and transport that supply to West Valley Water District's ("District") customers. The facilities are evaluated based on the projected highest water usage day when the District's service area is fully developed or built out. The Plan is a living document that is generally updated every five years.

The Akel Engineering Group, Inc. is the consultant that updated the Plan. In support of their planning effort, they created and calibrated a hydraulic water model of the District's distribution system utilizing existing Geographic Information System ("GIS") data provided by the District. Existing customer water demands were provided to the consultant and were geographically distributed within the model according to service addresses to enable them to perform an extended period simulation of the system.

Pipeline sizes were evaluated for their ability to convey flows, reservoirs were evaluated for storage adequacy by pressure zone and pump stations were evaluated on their ability to boost required flows. This evaluation was performed for both the existing facilities within the distribution system and for future demands to ensure that recommended facilities are sufficiently sized. Future water demands were distributed according to undeveloped areas within the District's service area, their projected land use based on the latest General Plans of the Cities and County areas and by updated water unit factors.

#### **DISCUSSION:**

Attached for your review, approval and eventual adoption is the draft 2019 Water Facilities Master Plan and 5-year Capital Improvement Program (Exhibit A). The following are highlights of the Plan:

• The water demand projections used for ultimate build-out of the District are based on land uses from the latest General Plan Land Use maps from the Cities of Rialto, Fontana, Colton and Counties of San Bernardino and Riverside. Actual consumption data for the various land uses were extracted from District billing information and used to project

future water demands. As a result, future water demands are lower than those projected in the previous Water Master Plan.

- The calculated water use rate per Equivalent Dwelling Unit (EDU) is 670 gallons per day (gpd). This usage reflects a decrease in consumption from the previous Water Master Plan, which utilized 750 gpd per EDU. Future demands are expected to decrease based upon water conservation programs employed by the District, by regional incentive programs, water conserving fixtures/appliances, Green Building Codes, new ordinances/laws, and general education of the public.
- The projected development within the District will require a large investment in new infrastructure. This study analyzes this future development and identifies the facilities needed to serve it. Residential lands are currently built to 59 percent of the proposed land use capacity, while non-residential lands are developed to 75 percent of the proposed capacity. Thus, approximately 66 percent of the overall land use plan is built out.
- Future water supplies will include additional groundwater, State Water Project water and purchased groundwater. This will require the District to drill additional wells, expand treatment capabilities at the Oliver P. Roemer Water Filtration Facility, install wellhead treatment, and enter into additional agreements for purchased groundwater supplies.
- A 5-year and a long-term (build-out) capital improvement plan ("CIP") was prepared to address facility replacement and recommended projects to support future growth. The 5-year CIP cost summary can be found in table ES.1 and the identified projects with costs and improvement phasing can be found in Table 8.7.

The Plan will enable the District to strategize planning and budgeting efforts and to implement water system improvements that will maintain a high level of distribution reliability and efficiency for current demands, future growth, and emergency situations.

Per Government Code 65403, the capital improvement program shall be adopted by, resolution of the governing body of the district. At least 60 days prior to its adoption, the capital improvement program shall be referred to the planning agency of each affected city and county within which the district operates, for review as to its consistency with the applicable general plan.

#### **FISCAL IMPACT:**

No fiscal impact.

#### **STAFF RECOMMENDATION:**

It is recommended that the Engineering, Operations and Planning Committee authorize staff to provide a copy of the Final Draft of the 2019 Water Facilities Master Plan to the planning agencies of each of the affected cities and counties within the District's jurisdiction for review and comment. They include the Cities of Rialto, Fontana, Colton, Jurupa Valley and the counties of San Bernardino and Riverside.

Respectfully Submitted,

Clarece C. Manselly

Clarence Mansell Jr, General Manager

LJ:ce

ATTACHMENT(S):
1. Exhibit A - Final Draft of the 2019 Water Facilities Master Plan

# **EXHIBIT A**



September 2019

# Water Facilities Master Plan





#### **WEST VALLEY WATER DISTRICT**

# 2019

# WATER FACILITIES MASTER PLAN

Final Draft (Revised)

September 2019





September 30, 2019

West Valley Water District 855 W. Base Line Road Rialto, CA 92377

Attention: Linda Jadeski

**Engineering Services Manager** 

Subject: 2019 Water Facilities Master Plan – Final Draft Report

Dear Linda:

We are pleased to submit this final draft report for the West Valley Water District Water Facilities Master Plan. This master plan is a standalone document intended to plan the orderly and phased growth of the water system. The master plan documents the following:

- Existing distribution system facilities, acceptable hydraulic performance criteria, and projected water demands
- Development and calibration of the District's GIS-based hydraulic water model.
- Capacity evaluation of the existing water system with improvements to mitigate existing deficiencies and to accommodate future growth.
- Capital Improvement Program (CIP) with an opinion of probable construction costs and suggestions for cost allocations to meet AB 1600.
- Potable water supply and regulations completed by Kleinfelder, Inc.

We extend our thanks to you, and other District staff whose courtesy and cooperation were valuable components in completing this study.

Sincerely,

AKEL ENGINEERING GROUP, INC.

Tony Akel, P.E. Principal

**Enclosure: Report** 



# **Acknowledgements**

#### **Board of Directors**

Dr. Michael Taylor, President

Mr. Kyle Crowther, Vice President

Dr Clifford O. Young, Sr.

Mr. Greg Young

Mr. Donald Olinger

#### **District Staff**

Mr. Clarence Mansell, Jr, General Manager
 Mr. Ricardo Pacheco, Assistant General Manager
 Ms. Linda Jadeski, Engineering Services Manager
 Ms. Joanne Chan, Operations Manager
 Mr. Joe Schaack, Production Supervisor
 Other District Engineering and Operations Staff

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#### **EXECUTIVE SUMMARY**

The purpose of this Water Facilities Master Plan is to determine the future water demands and supply requirements for West Valley Water District (District) and to identify the water facilities needed to produce, deliver, store and transport this supply to its customers. The facilities are based on the projected highest water usage day, when the District is fully developed.

This executive summary presents a brief background of the District's water distribution system, the planning area characteristics, the system performance and design criteria, the hydraulic model, and a capital improvement program. A hydraulic model of the District's existing water distribution system was created and used to evaluate the capacity adequacy of the existing distribution system and to recommend improvements to mitigate existing deficiencies, as well as servicing future growth.

The highlights of this Water Facilities Master Plan are listed as follows:

- 1. The water demand projections used for ultimate build-out of the District are based on land uses from the latest General Plan Land Use maps from the Cities of Rialto, Fontana, Colton and Counties of San Bernardino and Riverside. Actual consumption data for the various land uses were extracted from District billing information and used to project future water demands. As a result, future water demands are lower than those projected in the previous Water Master Plan.
- 2. The calculated water use rate per Equivalent Dwelling Unit (EDU) is 670 gallons per day (gpd). This usage reflects a decrease in consumption from the previous Water Master Plan, which utilized 750 gpd per EDU. Future demands are expected to decrease based upon water conservation programs employed by the District, by regional incentive programs, water conserving fixtures/appliances, Green Building Codes, new ordinances/laws, and general education of the public.
- 3. The projected development within the District will require a large investment in new infrastructure. This study analyzes this future development and identifies the facilities needed to serve it. Residential lands are currently built to 59 percent of the proposed land use capacity, while non-residential lands are developed to 75 percent of the proposed capacity. Thus, approximately 66 percent of the overall land use plan is built out.
- 4. Future water supplies will include additional groundwater, State Water Project (SWP) water and purchased groundwater. This will require the District to drill additional wells, expand treatment capabilities at the Oliver P. Roemer Water Filtration Facility (WFF), install wellhead treatment, and enter into additional agreements for purchased groundwater supplies.

- 5. To meet the ultimate peak day water demands, the District will have to expand treatment capabilities at the Oliver P. Roemer Water Filtration Facility (WFF) to maximize the use of State Water Project (SWP) water, drill new wells in the Bunker Hill groundwater basin and construct the reservoirs and pump stations needed to support these wells. The following 5-year Capital Improvement Projects are recommended:
  - Construct the expansion of the Oliver P. Roemer Water Filtration Facility.
  - Drilling four new wells in the Bunker Hill Basin.
  - Install wellhead treatment or create blending plans for existing wells.
  - Construct Reservoir R8-3.
  - Construct Booster Pump Station 4-3, 7-2 and a new Bunker Hill pump station.
  - Construct new transmission pipelines and replace aging pipelines.
  - Acquire property for needed facilities.

#### **ES.1 STUDY OBJECTIVES**

The District recognizes the importance of planning, developing, and financing the District's water system infrastructure. As such, District staff initiated an update to the Water Facilities Master Plan, most recently completed in 2012. This master plan included the following tasks:

- Summarizing the District's existing domestic water system facilities
- Documenting growth planning assumptions and known future developments
- Updating the domestic water system performance criteria
- Projecting future domestic water demands
- Creating and calibrating a new hydraulic model using Geographic Information Systems (GIS) data
- Evaluating the domestic water facilities to meet existing and projected demand requirements and fire flows
- Evaluating the existing groundwater conditions
- Performing a capacity analysis for major distribution mains
- Performing a fire flow analysis
- Recommending a capital improvement program (CIP) with an opinion of probable costs for 5-year and buildout growth
- Performing a capacity allocation analysis for cost sharing purposes

#### **ES.2 STUDY AREA**

The District provides domestic water service to customers throughout southwestern San Bernardino County and a small portion of northern Riverside County, as part of the greater San Bernardino-Riverside-Ontario metropolitan area. The service area, approximately 50 miles east of downtown Los Angeles, is generally bounded by U.S. Forest Service land to the north and Riverside County to the south, with the cities of San Bernardino and Colton serving as the eastern boundaries and the City of Fontana as the western boundary (Figure ES.1). The District Sphere of Influence encompass 18,076 acres, serving over 80,000 residents.

#### **ES.3 SYSTEM PERFORMANCE AND DESIGN CRITERIA**

This report documents the District's performance and design criteria that were used for evaluating the domestic water system. The system performance and design criteria are used to establish guidelines for determining future water demands, evaluating existing domestic water facilities, and for sizing future facilities. Chapter 3 discusses the system performance and design criteria for the domestic water system.

#### **ES.4 EXISTING WATER SYSTEM OVERVIEW**

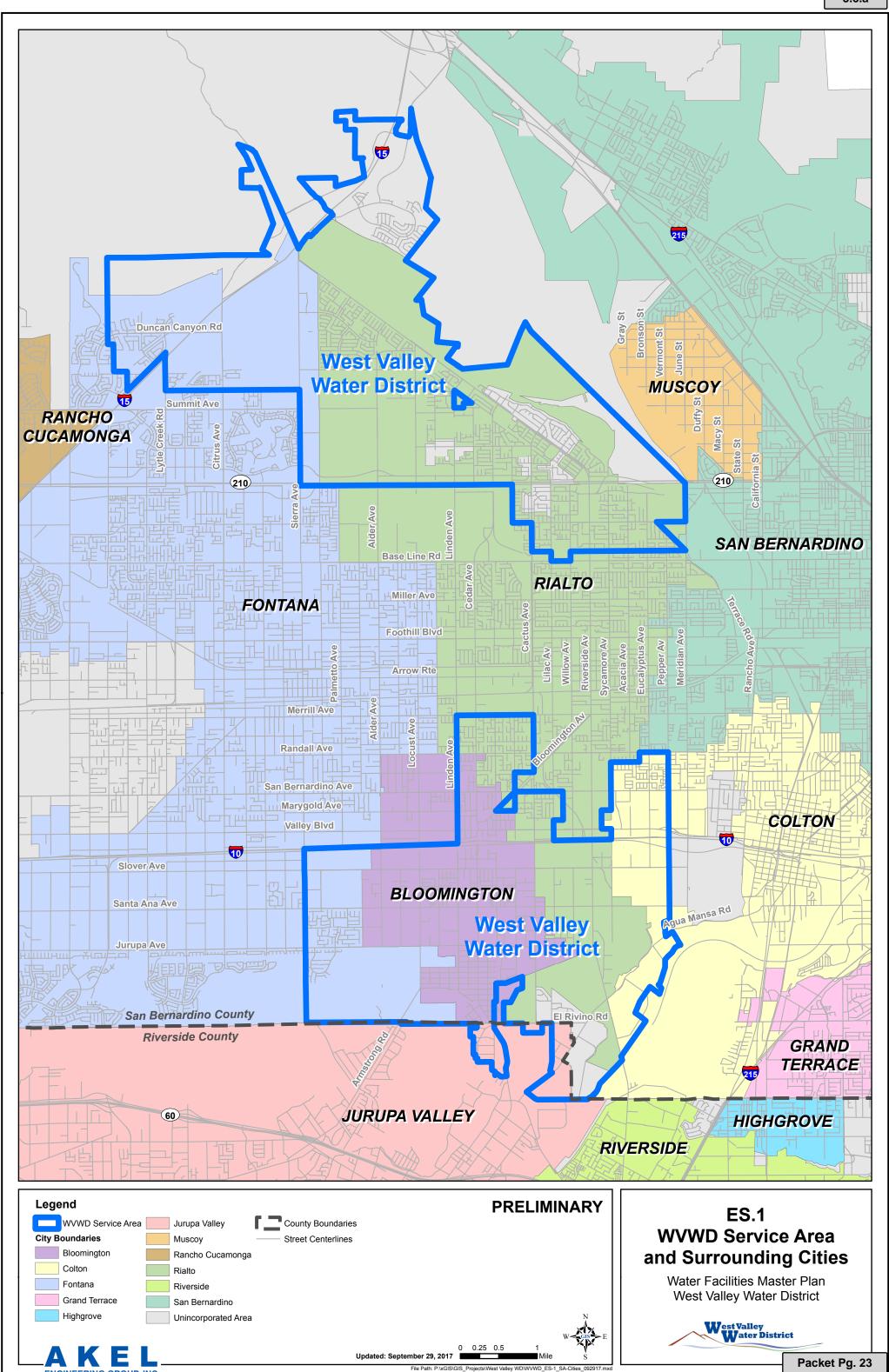
The District utilizes multiple sources of drinking water supply to serve its existing customers. The water distribution system is generally divided into two sections, commonly referred to as the North System and the South System. The existing water distribution is shown graphically on **Figure ES.2**, with a general color coding for the distribution mains as well as labeling the existing booster stations, valve stations, storage reservoirs, and supply facilities. Booster stations and valve stations are used to convey water between the District's multiple pressure zones, with storage tanks providing additional water supply for operational and emergency purposes.

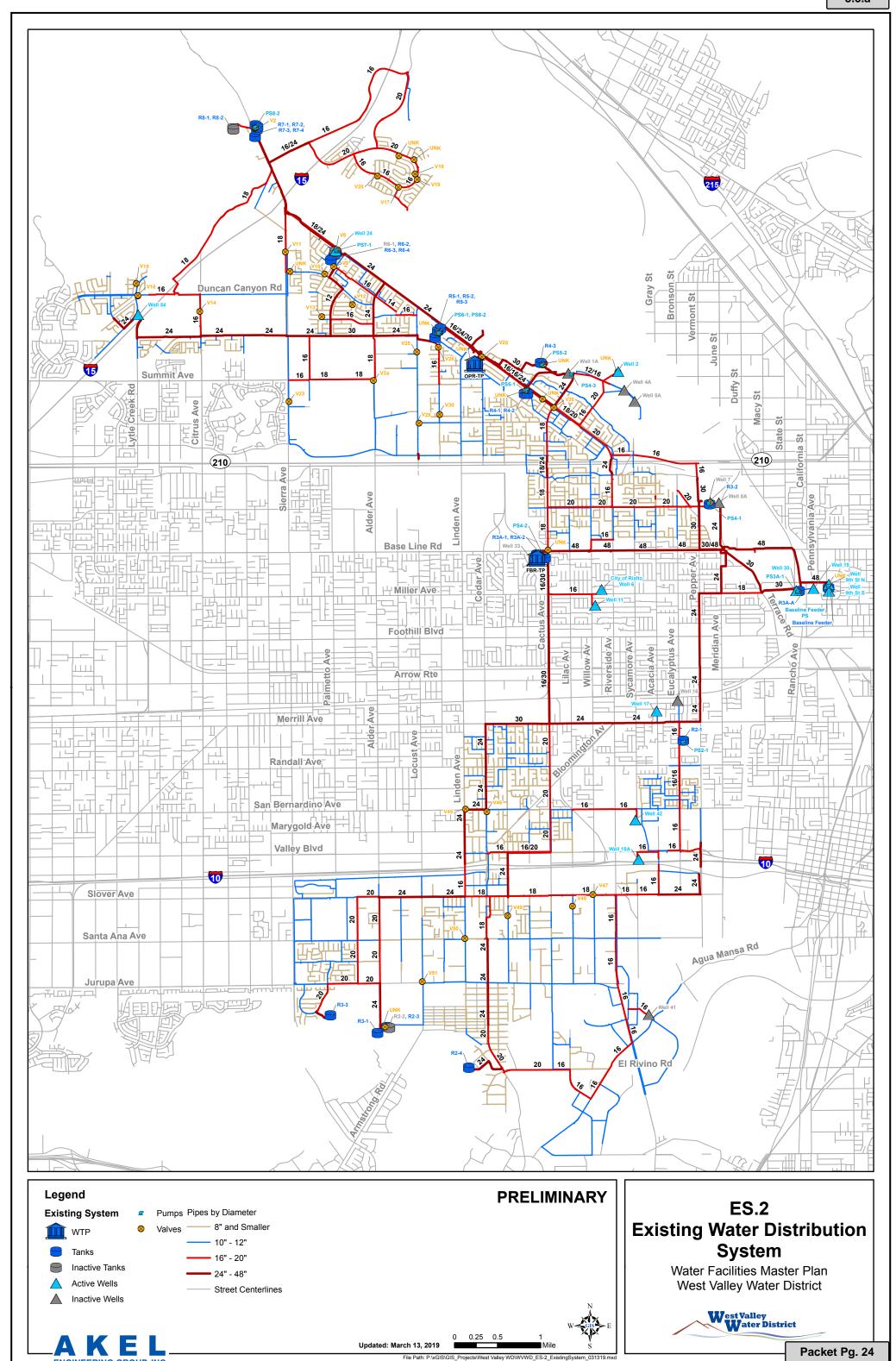
#### **ES.5 EXISTING AND FUTURE DOMESTIC WATER DEMANDS**

The existing water demands used for this master plan were based on the District's water billing consumption records and adjusted to match the annual production records and account for system loss. Additionally, future demands were developed based on known development expected to occur within the next five years as well as the expected buildout development identified by the counties of San Bernardino and Riverside.

#### **ES.6 WATER SUPPLY PLANNING**

In order to meet the existing domestic water demands the District utilizes several sources of supply, including groundwater and treated surface water. The District's existing wells extract groundwater from one of the following groundwater basins: Lytle Creek Basin, Bunker Hill Basin, Rialto-Colton Basin, Chino Basin, and Riverside-Arlington Basin. The District also treats the following two sources of surface water at the Oliver P. Roemer Water Filtration Facility (Roemer Water Filtration Facility): Lytle Creek and State Water Project.





In order to meet the growing demand requirements of the District service area and provide additional water supply reliability, the existing water supply capacity will require expansion; this expansion is

planned to include the rehabilitation of existing groundwater wells, the construction of new groundwater wells, and the expansion of the Roemer Water Filtration Facility.

#### **ES.7 HYDRAULIC MODEL DEVELOPMENT**

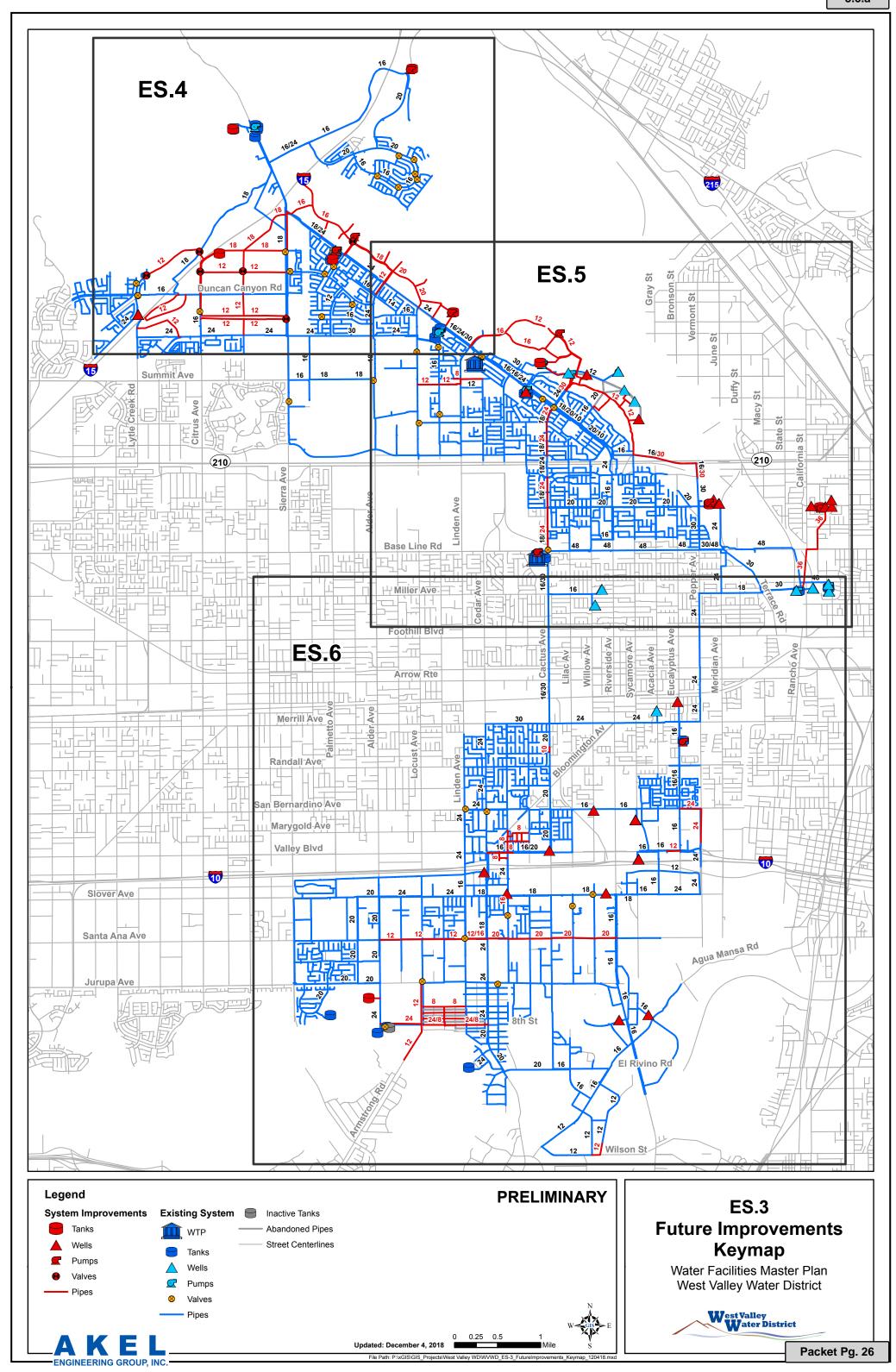
Hydraulic network analysis has become an effective and powerful tool in many aspects of water distribution planning, design, operation, management, emergency response planning, system reliability analysis, fire flow analysis, and water quality evaluations. As a part of this master plan a new hydraulic model was developed for the District's water distribution system, combining information on the physical characteristics of the water system (pipelines, groundwater wells, valves, booster stations, and storage reservoirs) and operational characteristics (how they operate). The hydraulic model development process included a thorough verification and calibration process with District staff to ensure the water model was consistent with the existing water distribution system and provided results consistent with real-world conditions.

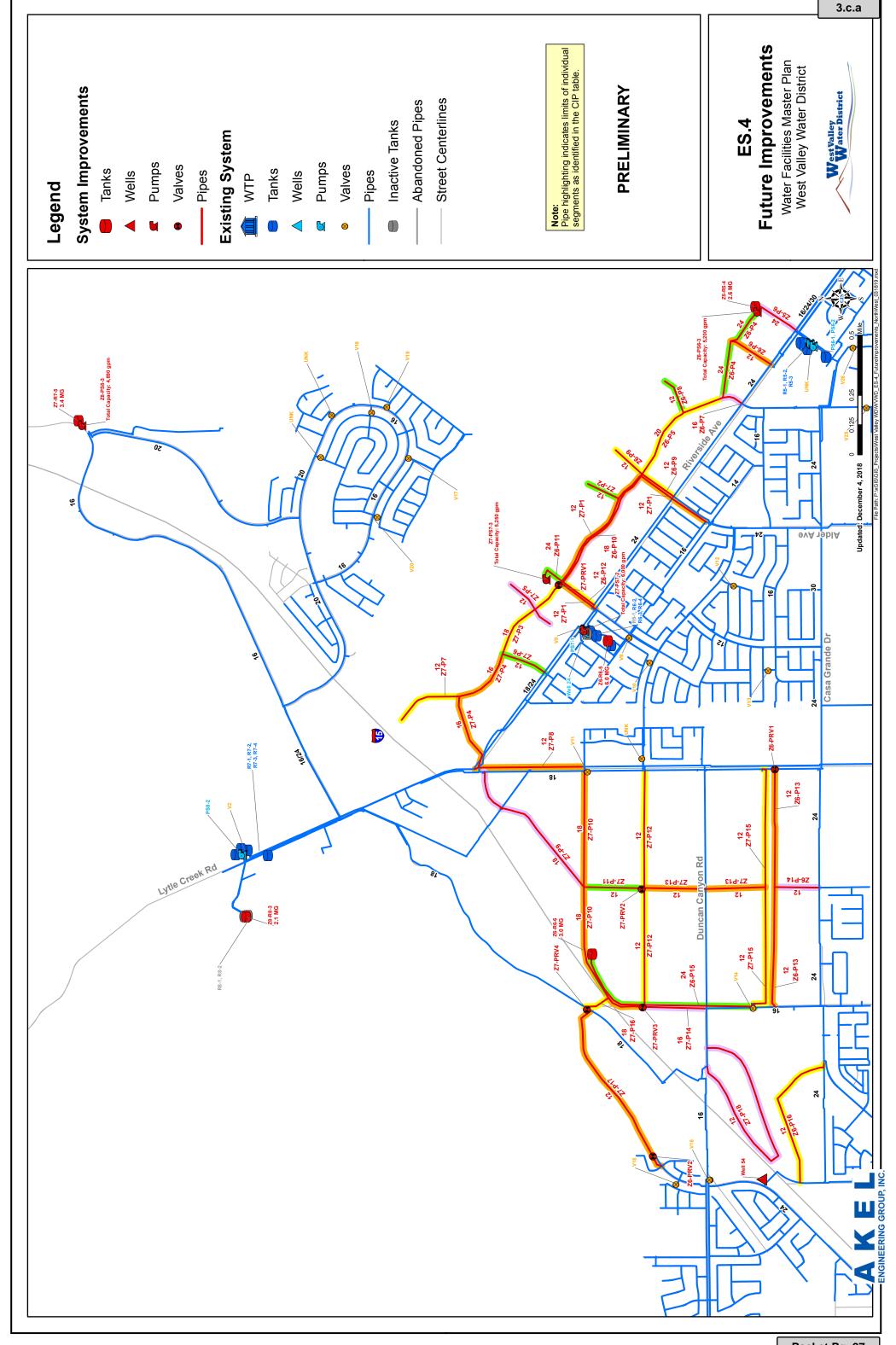
#### **ES.8 EXISTING SYSTEM EVALUATION**

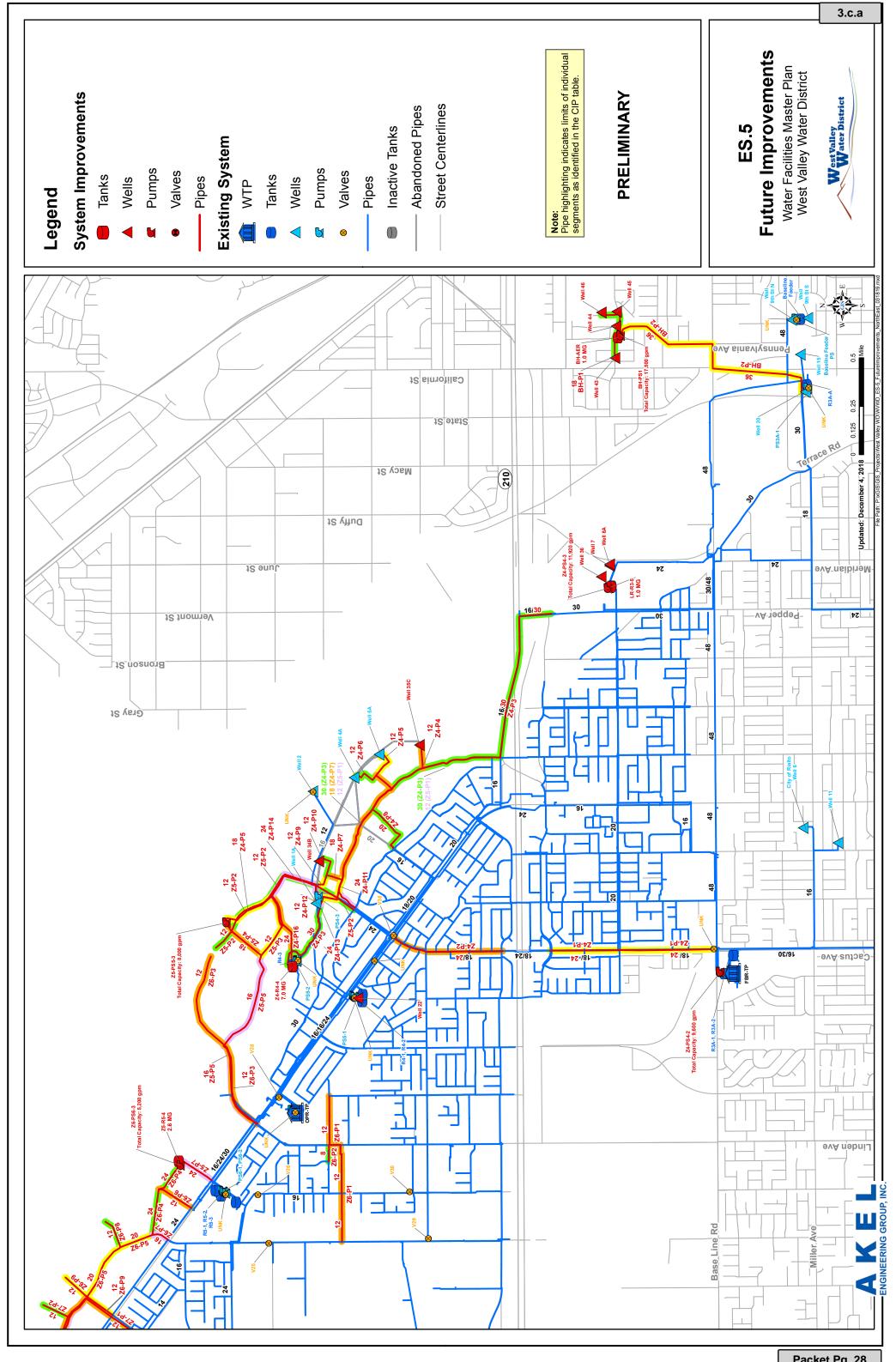
The District's master plan included a hydraulic evaluation of the District's existing water distribution system. This hydraulic evaluation included analyzing the system-wide pressures under various demand conditions comparing the existing storage capacity, booster station capacity, and supply capacity to the required amounts based on the master plan performance criteria. The District's existing system is generally able to meet the system performance criteria under existing conditions. Improvements will be recommended to mitigate the deficiencies identified as part of the evaluation.

#### **ES.9 CAPITAL IMPROVEMENT PROGRAM**

The Capital Improvement Program includes improvements consistent with ongoing projects planned by the District as well as improvements recommended for mitigating existing system deficiencies and servicing future growth. Figure ES.3 through Figure ES.6 document the recommended improvements. For budgeting purposes, the District included a 5-year improvement prioritization plan, and which is summarized in Table ES.1. A more detailed cost summary for the 5-year plan, as well as the buildout improvements, are documented in Chapter 8. As shown on Table ES.1, the total cost over the 5-year horizon is approximately 159.1 million dollars.







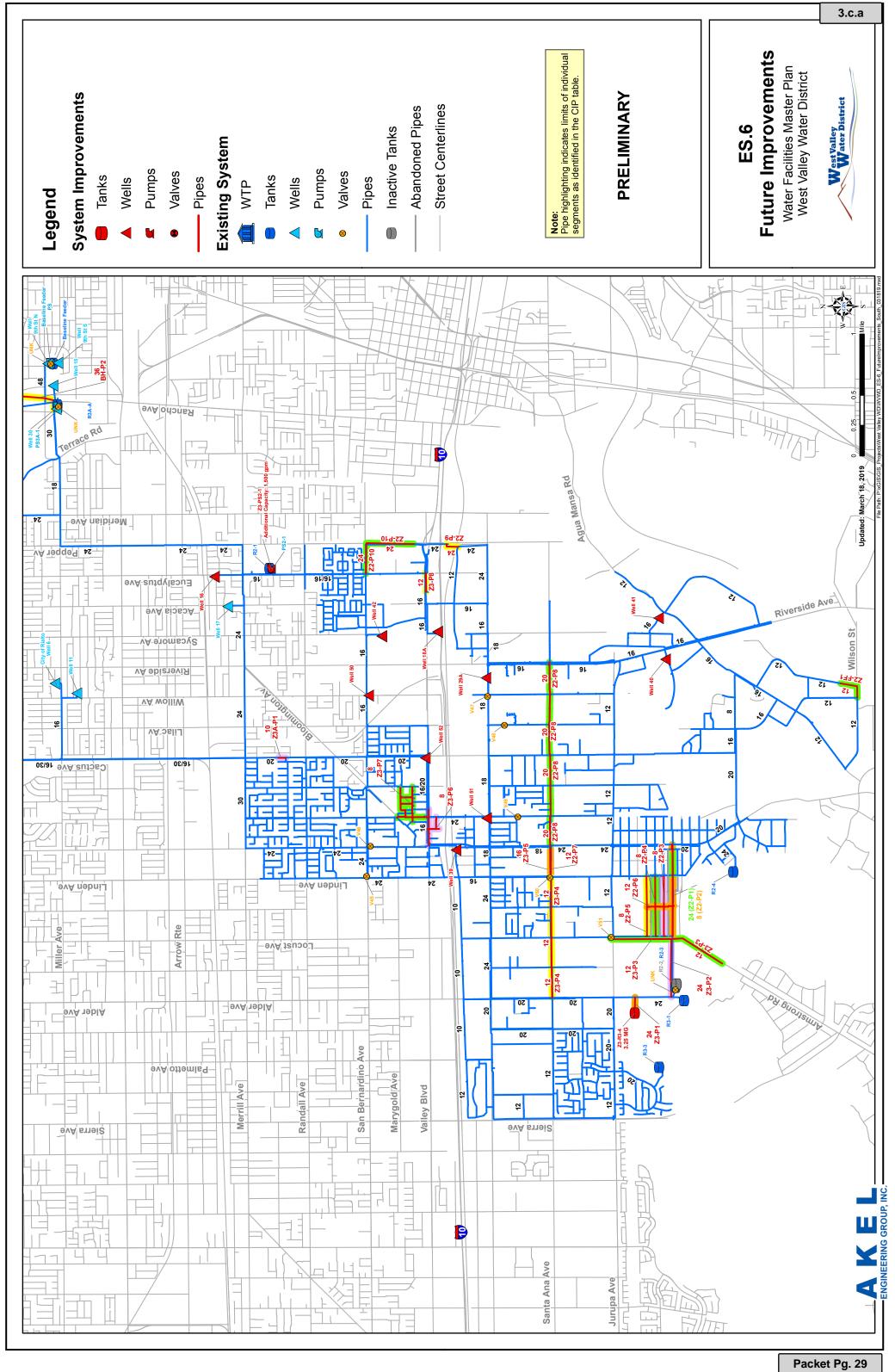


Table ES.1 5-Year CIP Summary

Water Facilities Master Plan West Valley Water District

**PRELIMINARY** 

	Existin	Existing Users	Future	Future Users	Combined P	Combined Project Costs
Fiscal Year	Fiscal Year Total (\$)	Cumulative Total (\$)	Fiscal Year Total (\$)	Cumulative Total (\$)	Fiscal Year Total (\$)	Cumulative Total (\$)
2018/19	\$2,528,000	\$2,528,000	\$6,207,000	\$6,207,000	\$8,735,000	\$8,735,000
2019/20	\$14,163,200	\$16,691,200	\$80,106,920	\$86,313,920	\$94,270,120	\$103,005,120
2020/21	\$1,766,000	\$18,457,200	\$25,858,000	\$112,171,920	\$27,624,000	\$130,629,120
2021/22	\$5,364,500	\$23,821,700	\$3,523,000	\$115,694,920	\$8,887,500	\$139,516,620
2022/23	\$6,001,000	\$29,822,700	\$7,073,000	\$122,767,920	\$13,074,000	\$152,590,620
2023/24	0\$	\$29,822,700	\$6,469,000	\$129,236,920	\$6,469,000	\$159,059,620
Total Improvement Cost		\$29,822,700		\$129,236,920		\$159,059,620
ENGINEERING GROUP, INC.						4/5/2019

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#### **CHAPTER 1 - INTRODUCTION**

This chapter provides a brief background of the District's domestic water system, the need for this master plan, and the objectives of the study. Abbreviations and definitions are also provided in this chapter.

#### 1.1 BACKGROUND

The West Valley Water District (District) provides domestic water service to customers throughout southwestern San Bernardino County and a small portion of northern Riverside County, as part of the greater San Bernardino-Riverside-Ontario metropolitan area. The service area, approximately 50 miles east of downtown Los Angeles, generally includes the cities of Fontana, Rialto, Colton, Jurupa Valley, Bloomington, and other unincorporated areas of San Bernardino County (Figure 1.1). The District provides potable water service to more than 80,000 residents, as well as a myriad of commercial, industrial, and institutional establishments. The District operates a domestic water distribution system that consists of 21 groundwater wells, 25 separate storage reservoirs across eight pressure zones, for a total storage over 72 million gallons (MG), and over 375 miles of transmission and distribution pipelines.

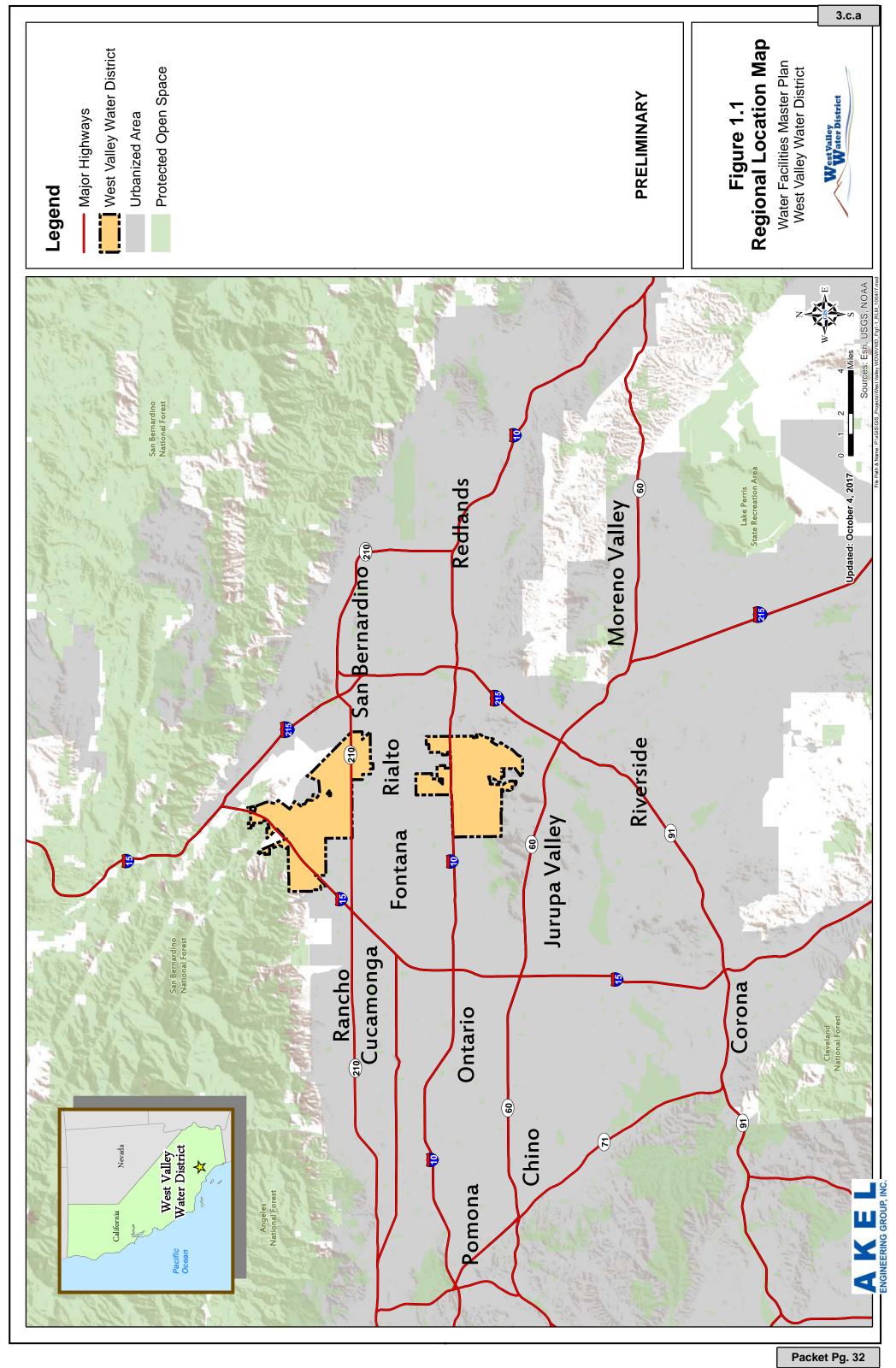
In 2012, the District developed a Water System Master Plan that identified capacity deficiencies in the existing water system and recommended improvements to alleviate existing deficiencies and serve future developments inside the District's service area. Recognizing the importance of planning, developing, and financing system facilities to provide reliable water service to existing customers and for servicing anticipated growth within the service area, the District initiated updating elements of the 2012 Water System Master Plan, to reflect current land use conditions.

#### 1.2 SCOPE OF WORK

The District approved Akel Engineering Group Inc. to prepare this 2019 Water Facilities Master Plan (WFMP) in May of 2017. This 2019 WFMP is intended to serve as a tool for planning and phasing the construction of future domestic water system infrastructure for the projected buildout of the service area. The 2019 WFMP evaluates the District's water system and recommends capacity improvements necessary to service the needs of existing users and for servicing the future growth of the District.

The service area and horizon for the master plan are reflective of the cumulative growth associated with the differing municipalities serviced by the District. Should planning conditions change, and depending on their magnitude, adjustments to the master plan recommendations might be necessary.

This master plan included the following tasks:



- Summarizing the District's existing domestic water system facilities
- Documenting growth planning assumptions and known future developments
- Updating the domestic water system performance criteria
- Projecting future domestic water demands
- Creating and calibrating a new hydraulic model using Geographic Information Systems (GIS) data
- Evaluating the domestic water facilities to meet existing and projected demand requirements and fire flows
- Evaluating the existing groundwater conditions
- Performing a capacity analysis and fire flow analysis for distribution mains
- Recommending a capital improvement program (CIP) with an opinion of probable costs for
   5-year and buildout growth
- Performing a capacity allocation analysis for cost sharing purposes

#### 1.3 PREVIOUS MASTER PLANS

The District's most recent water master plan was completed in 2012. This master plan included an evaluation of servicing growth throughout the Sphere of Influence, evaluated existing demands and projected future demands, recommended phased improvements as part of a 5 year capital improvement program, and identified pumping and storage requirements for the buildout of the Sphere of Influence.

#### 1.4 RELEVANT REPORTS

The District has completed several special studies intended to evaluate localized growth. These reports were referenced and used during the preparation of 2019 WFMP. The following lists relevant reports that were used in the completion of this master plan, as well as a brief description of each document:

- 2012 Water Master Plan, August 2012. (2012 WMP). This report documents the water demand projection and provides an update to the Capital Improvement Program, through the evaluation of the existing water system.
- 2015 San Bernardino Valley Regional Urban Water Management Plan. The District participated in the 2015 San Bernardino Valley Regional Urban Water Management Plan (RUWMP), which established a benchmark per capita water usage and targets in order to achieve higher levels of water conservation for the sustainability of water supply sources. This included adopting an updated water shortage contingency plan, defining supply sources, addressing supply reliability, and projecting sustainable supply yields and future demands.

• Draft 2017 Lytle Creek Ranch Water Facilities Feasibility Study. This report documents the preliminary water facility requirements for the buildout of the Lytle Creek Ranch Specific Plan. This report includes demand projections for the buildout of the Lytle Creek Ranch development and documents preliminary pipeline alignments as well as pump station and storage reservoir sizes and locations. Additionally, preliminary project costs are documented for the required water facility improvements.

#### 1.5 REPORT ORGANIZATION

The water system master plan report contains the following chapters:

**Chapter 1 - Introduction.** This chapter provides a brief background of the District's domestic water system, the need for this master plan, and the objectives of the study. Abbreviations and definitions are also provided in this chapter.

Chapter 2 - Planning Areas Characteristics. This chapter presents a discussion of the planning area characteristics for this master plan and defines the land use classifications. The planning area is divided into several planning sub-areas, as established by the various city and county general plans.

Chapter 3 - System Performance and Design Criteria. This chapter presents the District's performance and design criteria, which was used in this analysis for identifying current system capacity deficiencies and for sizing proposed distribution mains, storage reservoirs, pump stations and wells.

**Chapter 4 - Existing Domestic Water Facilities.** This chapter provides a description of the District's existing domestic water system facilities including the distribution mains, storage reservoir, booster pump stations and the existing wells.

**Chapter 5 - Water Demands and Supply Characteristics.** This chapter summarizes existing domestic water demands, discussed available supply characteristics, and projects the future domestic water demands.

Chapter 6 - Hydraulic Model Development. This chapter describes the development and calibration of the District's domestic water distribution system hydraulic model. The hydraulic model was used to evaluate the capacity adequacy of the existing system and to plan its expansion to service anticipated future growth.

**Chapter 7 - Evaluation and Proposed Improvements.** This section presents a summary of the domestic water system evaluation and identifies improvements needed to mitigate existing deficiencies, as well as improvements needed to expand the system and service growth.

Chapter 8 - Capital Improvement Program. This chapter provides a summary of the recommended domestic water system improvements to mitigate existing capacity deficiencies and to accommodate anticipated future growth. The chapter also presents the cost criteria and

methodologies for developing the capital improvement program. Finally, a capacity allocation analysis, usually used for cost sharing purposes, is also included.

#### 1.6 ACKNOWLEDGEMENTS

Obtaining the necessary information to successfully complete the analysis presented in this report, and developing the long term strategy for mitigating the existing system deficiencies and for accommodating future growth, was accomplished with the strong commitment and very active input from dedicated team members including:

- Ms. Linda Jadeski, Engineering Services Manager
- Ms. Joanne Chan, Operations Manager
- Mr. Joe Schaack, Production Supervisor

#### 1.7 UNIT CONVERSIONS AND ABBREVIATIONS

Engineering units were used in reporting flow rates and volumes pertaining to the design and operation of various components of the domestic water distribution system. Where it was necessary to report values in smaller or larger quantities, different sets of units were used to describe the same parameter. Values reported in one set of units can be converted to another set of units by applying a multiplication factor. A list of multiplication factors for units used in this report is shown on Table 1.1.

Various abbreviations and acronyms were also used in this report to represent relevant water system terminologies and engineering units. A list of abbreviations and acronyms is included in Table 1.2.

#### 1.8 GEOGRAPHIC INFORMATION SYSTEMS

This master planning effort made extensive use of Geographic Information Systems (GIS) technology, for completing the following tasks:

- Developing the physical characteristics of the hydraulic model (pipes and junctions, wells, and storage reservoirs)
- Allocating existing water demands, as extracted from the water billing records, and based on each user's physical address.
- Calculating and allocating future water demands, based on future developments water use.
- Extracting ground elevations along the distribution mains from available digital elevation information.
- Generating maps and exhibits used in this master plan.

**Table 1.1 Unit Conversions** 

Water Facilities Master Plan West Valley Water District

PRFLIMINARY

		PRELIMINAR
	<b>Volume Unit Calculations</b>	
To Convert From:	To:	Multiply by:
acre feet	gallons	325,851
acre feet	cubic feet	43,560
acre feet	million gallons	0.3259
cubic feet	gallons	7.481
cubic feet	acre feet	2.296 x 10 <sup>-5</sup>
cubic feet	million gallons	7.481 x 10 <sup>-6</sup>
gallons	cubic feet	0.1337
gallons	acre feet	3.069 x 10 <sup>-6</sup>
gallons	million gallons	1,000,000
million gallons	gallons	1 x 10 <sup>-6</sup>
million gallons	cubic feet	133,672
million gallons	acre feet	3.069
	Flow Rate Calculations	
To Convert From:	To:	Multiply By:
ac-ft/yr	mgd	8.93 x 10 <sup>-4</sup>
ac-ft/yr	cfs	1.381 x 10 <sup>-3</sup>
ac-ft/yr	gpm	0.621
ac-ft/yr	gpd	892.7
cfs	mgd	0.646
cfs	gpm	448.8
cfs	ac-ft/yr	724
cfs	gpd	646300
gpd	mgd	1 x 10 <sup>-6</sup>
gpd	cfs	1.547 x 10 <sup>-6</sup>
gpd	gpm	6.944 x 10 <sup>-4</sup>
gpd	ac-ft/yr	$1.12 \times 10^{-3}$
gpm	mgd	1.44 x 10 <sup>-3</sup>
gpm	cfs	$2.228 \times 10^{-3}$
gpm	ac-ft/yr	1.61
gpm	gpd	1,440
mgd	cfs	1.547
mgd	gpm	694.4
mgd	ac-ft/yr	1,120
6~		

**Table 1.2 Abbreviations and Acronyms** 

Water Facilities Master Plan West Valley Water District

PRELIMINARY

Abbreviation	Evnancian	Abbreviation	PRELIMINARY
2012 WSMP	Expansion  2012 Water System Master Plan		Expansion
2012 W3WP	Association for the Advancement of	gpm	gallons per minute
AACE International	Cost Engineering	hp	horsepower
AC	acre	HGL	hydraulic grade line
ACP	Asbestos Cement Pipe	HWL	high water level
ADD	average day demand	in	inch
AF	Acre Feet	LF	linear feet
Akel	Akel Engineering Group, Inc.	MG	million gallons
CCI	Construction Cost Index	MGD	million gallons per day
CDPH	California Department of Public Health	MMD	maximum month demand
cfs	cubic feet per second	NFPA	National Fire Protection Association
CI	cast iron pipe	PDD	peak day demand
CIB	Capital Improvement Budget	PHD	peak hour demand
CIP	Capital Improvement Program	PRV	pressure reducing valve
DIP	Ductile Iron Pipe	psi	pounds per square inch
District	West Valley Water District	ROW	Right of Way
DU	dwelling unit	SBVMWD	San Bernardino Valley Municipal Water District
EDU	equivalent dwelling unit	SCADA	Supervisory Control and Data Acquisition
ENR	Engineering News Record	SCAG	Southern California Association of Governments
EPA	Environmental Protection Agency	SHGL	Static Hydraulic Gradient Line
EPS	Extended Period Simulation	SS	Steady-State
FBR	Fluidized Bed Reactor	SOI	Sphere of Influence
ft	feet	TBD	to be determined
fps	feet per second	ULL	Urban Limit Line
FY	Fiscal Year	WFF	Oliver P. Roemer Water Filtration Facility
GIS	Geographic Information Systems	WFMP	Water Facilities Master Plan
gpd	gallons per day	WTP	Water Treatment Plant
gpdc	gallons per day per capita		
AKEL			

2/9/2018

#### CHAPTER 2 - PLANNING AREA CHARACTERISTICS

This chapter presents a discussion of the planning area characteristics for this master plan and defines the land use classifications. The planning area is divided into several planning sub-areas, as established by the various city and county general plans.

# 2.1 STUDY AREA DESCRIPTION

The West Valley Water District provides domestic water service to customers throughout southwestern San Bernardino County and a small portion of northern Riverside County, as part of the greater San Bernardino-Riverside-Ontario metropolitan area. The service area, approximately 50 miles east of downtown Los Angeles, is generally bounded by U.S. Forest Service land to the north and Riverside County to the south, with the cities of San Bernardino and Colton serving as the eastern boundaries and the City of Fontana as the western boundary (Figure 2.1). The central portion of the City of Rialto divides the District's service area into a northern system and southern system and is served by the City of Rialto. The additional water agencies serving the areas adjacent to the District service area are summarized on Figure 2.2. The District Sphere of Influence encompass 18,076 acres, serving over 80,000 residents.

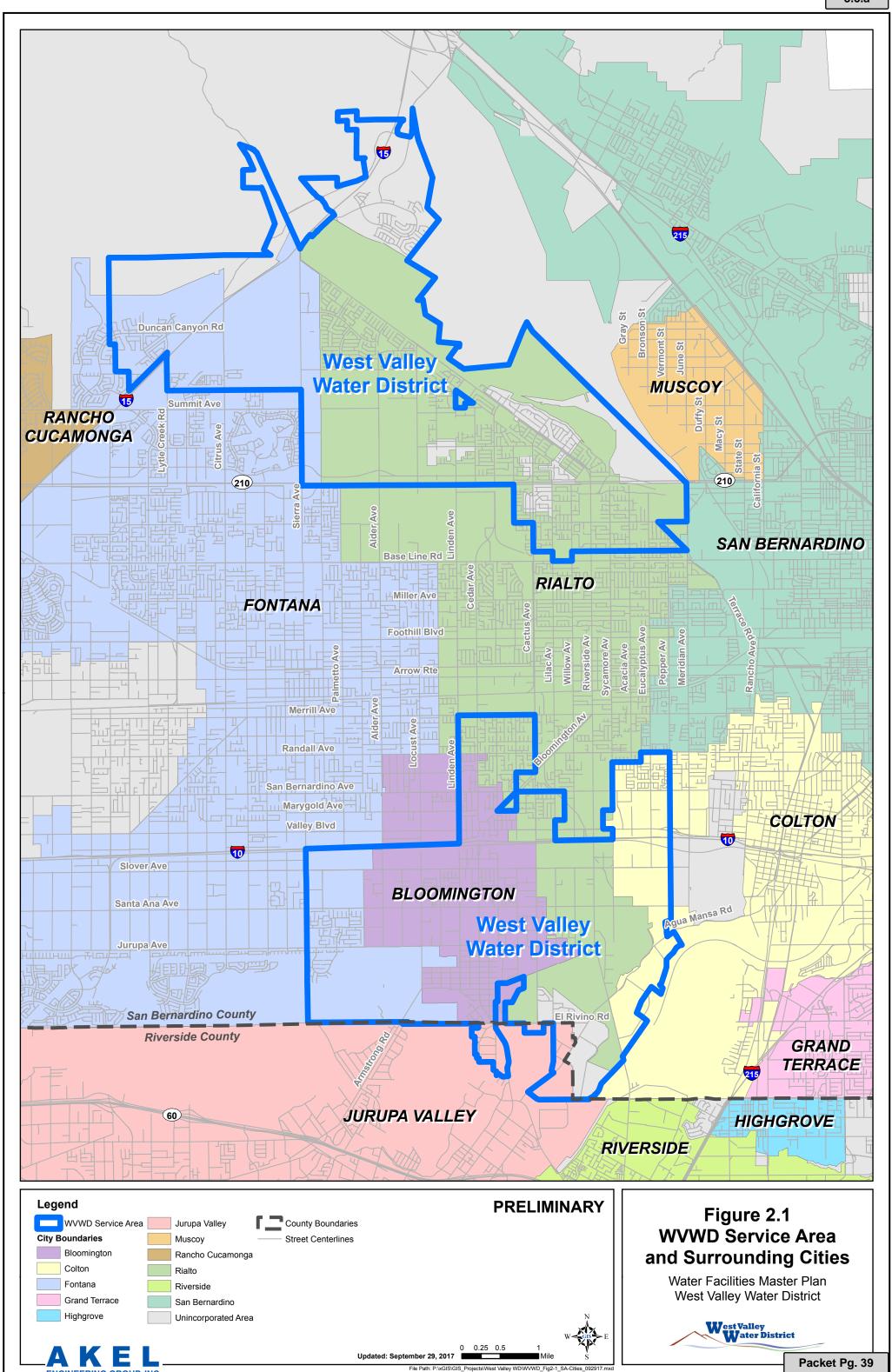
The topography of the service area generally slopes upward from south to north, with service elevations approximately ranging between 900 ft and 2,300 ft. Due to the varying terrain, the service area is divided into eight pressure zones to account for the changes in elevation. Currently, the water demands are met from a combination of groundwater wells and treated surface water. Booster stations and pressure reducing valves (PRVs) convey water from supply sources throughout the individual pressure zones.

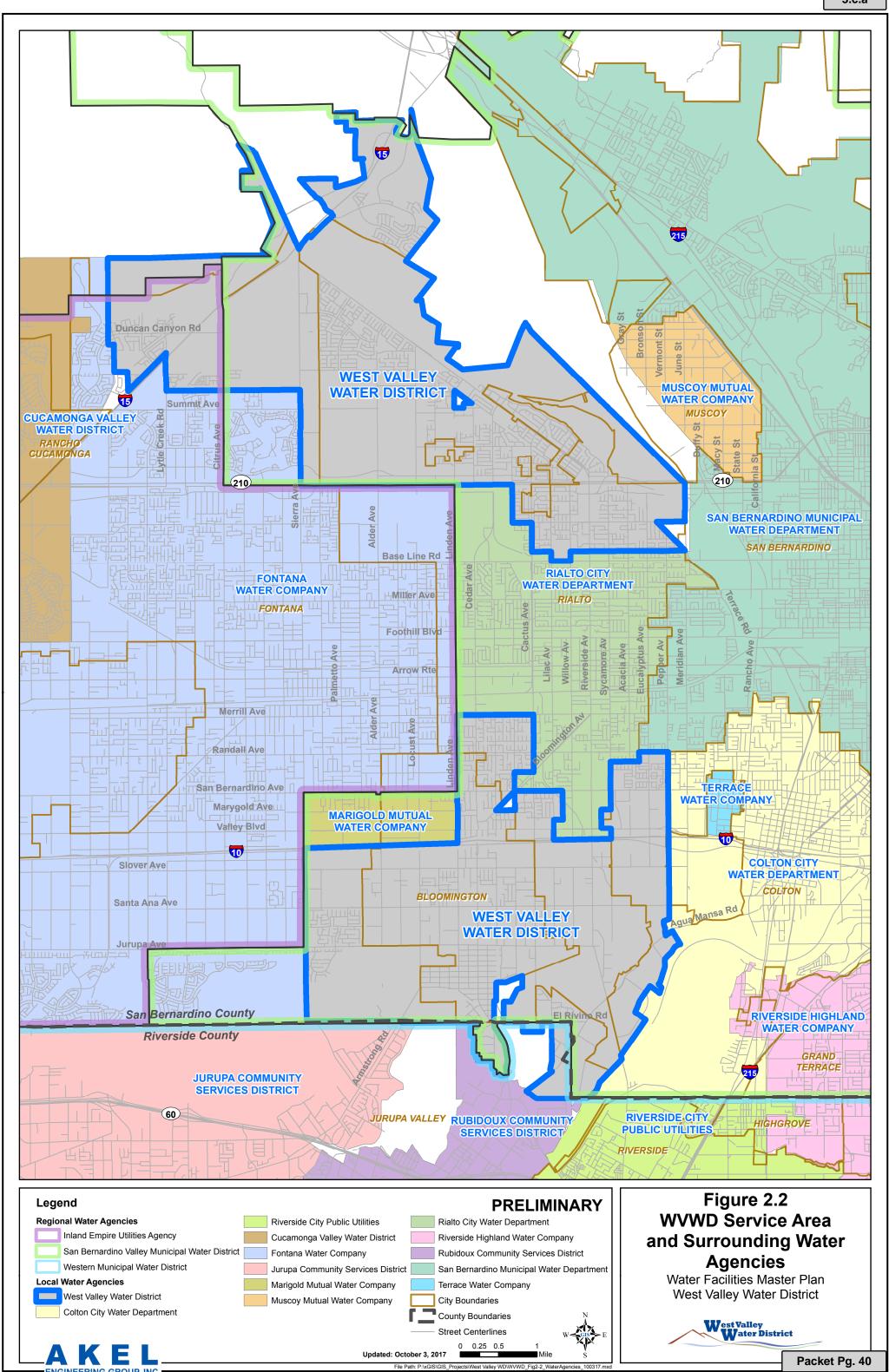
# 2.2 WATER SERVICE AREA AND LAND USE

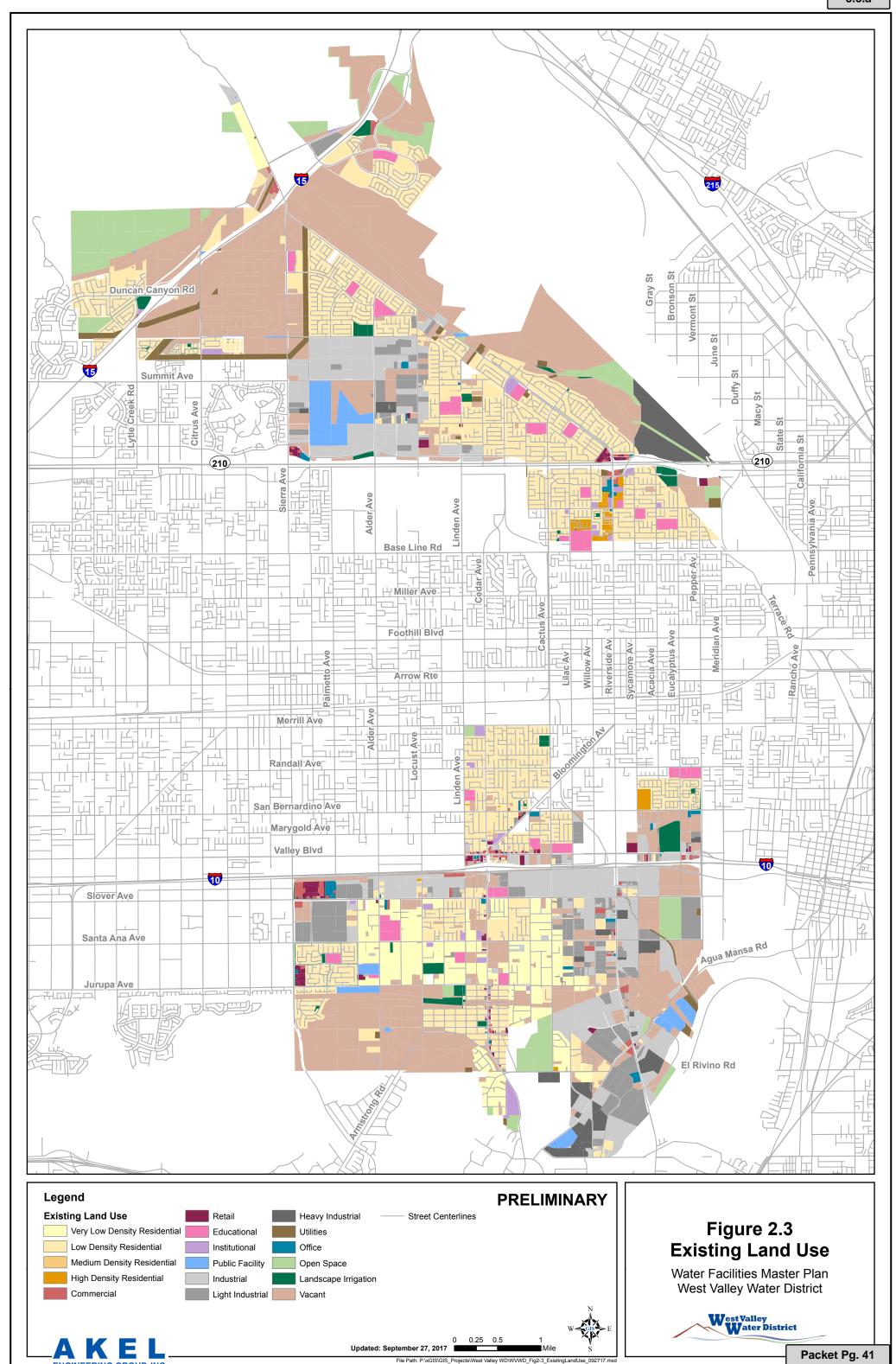
The existing service area is comprised of approximately 11,500 acres of developed lands and 6,300 acres of undeveloped land that is slated for growth. For planning purposes, this master plan evaluated the existing land use, 5-year growth projections, and buildout of the service area.

### 2.2.1 Existing Land Use

The existing land use within the District's service area is comprised of a relatively even split between residential and non-residential uses. Residential land uses comprise approximately 5,200 acres and non-residential uses totaling approximately 4,600 acres. Other land uses, including utilities, right of way, landscape irrigation, open space, and undeveloped land, make up the remainder of the service area. The existing land use is documented on Figure 2.3 and included on Table 2.1.







# **Table 2.1 Existing Service Area Land Use**

Water Facilities Master Plan West Valley Water District

PRELIMINARY

	PRELIMINARY
Land Use Designation	Existing Land Use within District's Service Area
Residential	
Residential 2	1,080
Residential 6	4,026
Residential 12	4
Residential 21	87
Subtotal- Residential	5,196
Non-Residential	
Commercial	123
Retail	121
Office	72
Educational	373
Institutional	129
Public Facility	324
Light Industrial	1,022
Heavy Industrial	510
Industrial	1,983
Subtotal-Non Residential	4,657
Other	
Utilities	293
ROW	110
Landscape Irrigation	238
Open Space	1,755
Vacant-Undeveloped	5,538
Subtotal- Other	7,934
Total	
AKEL	17,787

#### 2.2.2 Five Year Growth Projections

As part of this master plan evaluation, 5-year growth is evaluated for the purpose of identifying improvements necessary to serve development occurring in the near future. District staff have identified areas of development expected to occur within the next five years, which are summarized on Table 2.2 and shown graphically on Figure 2.4, and include the following large development projects:

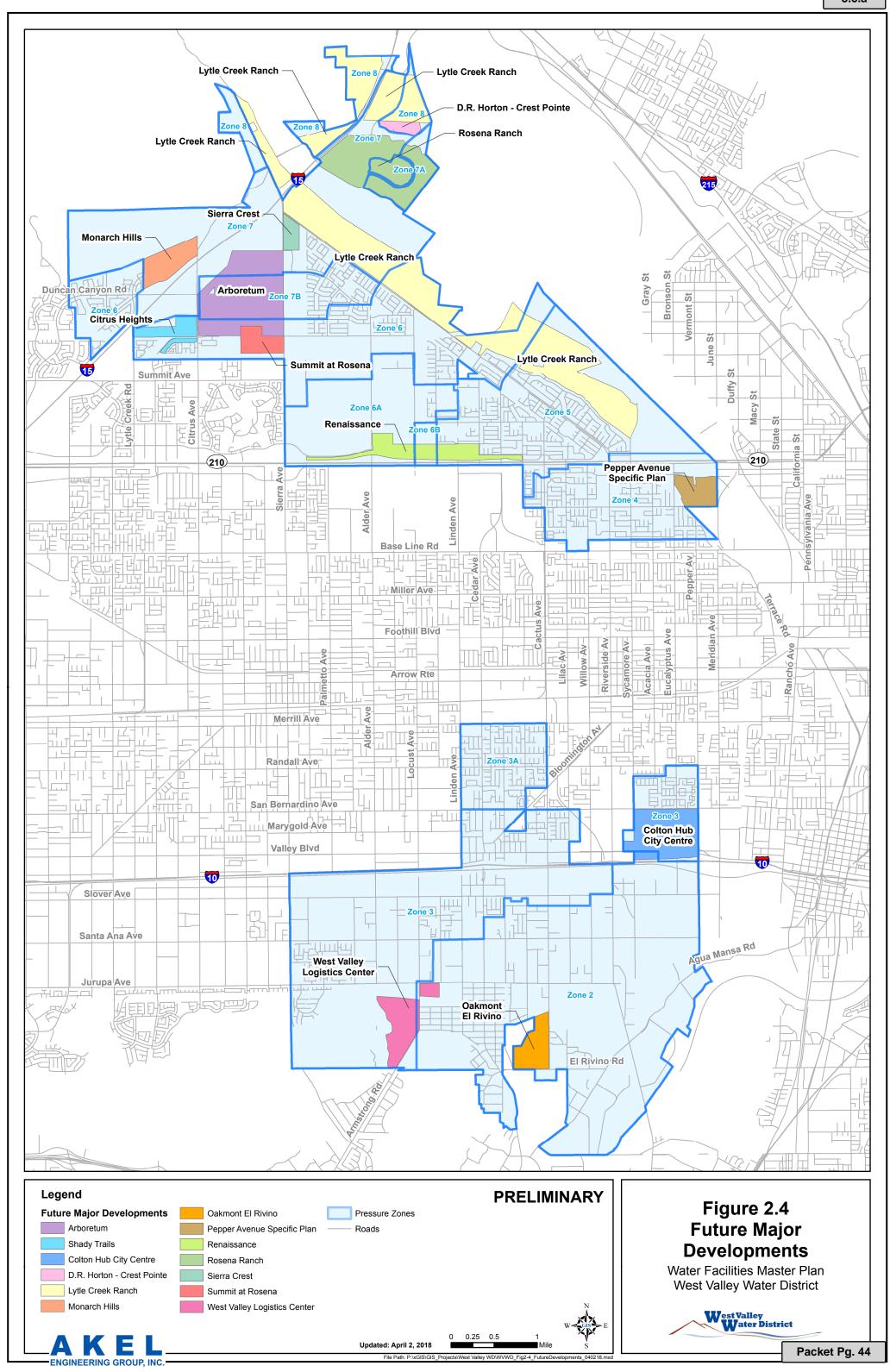
- Lytle Creek Ranch. This development is located along the northeast side of the District service area. The 5-year growth projection for Lytle Creek Ranch includes approximately 1,390 equivalent dwelling units across three pressure zones.
- Arboretum. This development is generally located north of Casa Grande Avenue between Sierra Avenue and Citrus Avenue, and south of Segovia Lane. 5-year growth estimates for Arboretum include approximately 1,990 equivalent dwelling units (EDU) in Pressure Zones 6 and 7.

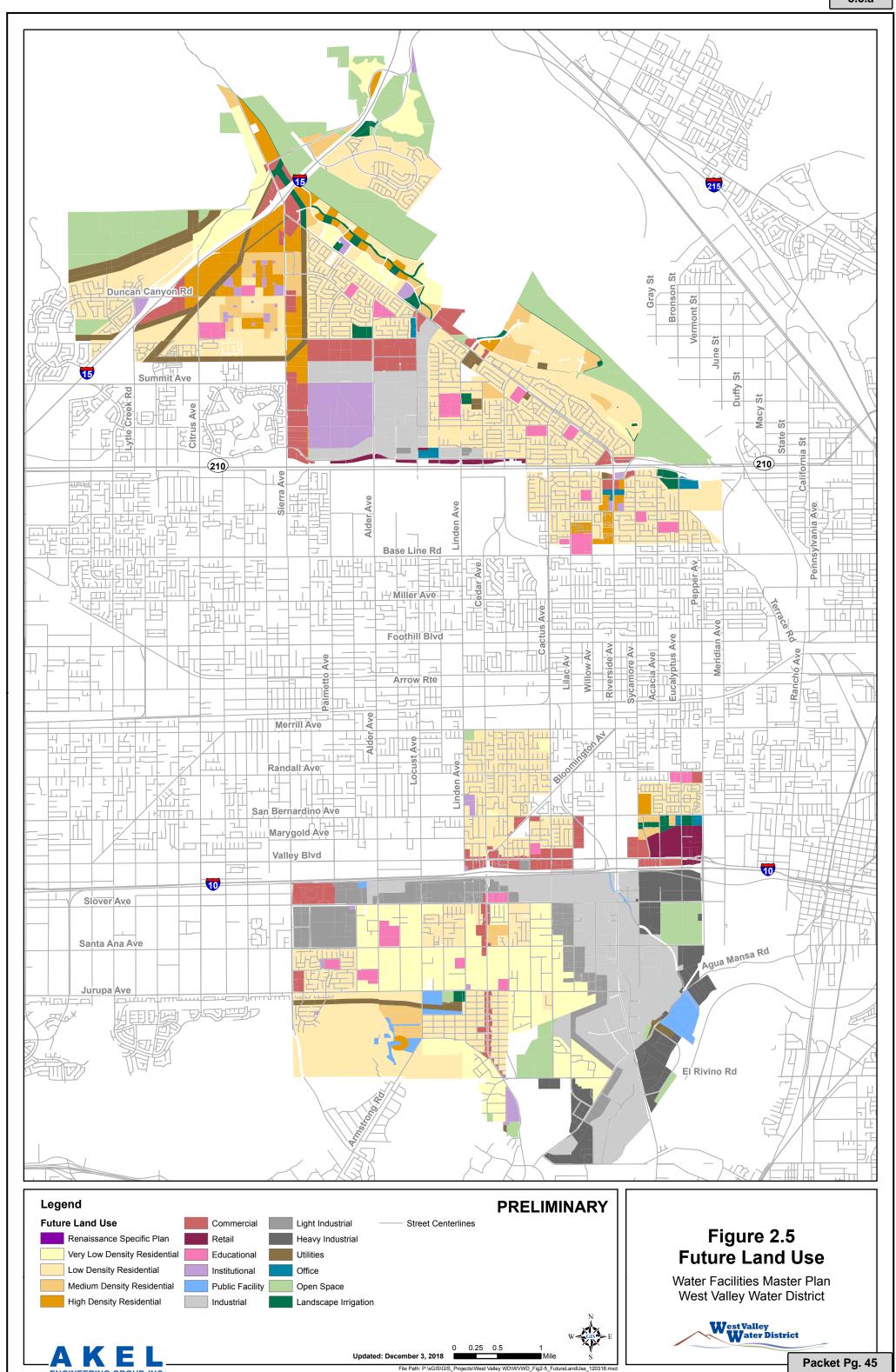
#### 2.2.3 Buildout Growth Projections

Buildout land use of the District service area is documented on Figure 2.5 and inventoried on Table 2.3. The existing and future land use acreages are broken down in toe the following categories:

- Existing Development: These acreages represent existing developed lands.
- Existing Lands Redeveloped: These acreages represent existing developed lands
  expected to redevelop into other land use types within the buildout horizon of the master
  plan.
- Existing Development Unchanged: These acreages represent the total existing acreages expected to remain within the buildout horizon of the master plan.
- **New Lands Redevelopment:** These acreages represent lands that have redeveloped from a prior use and into a new respective category.
- New Development: These acreages represent gains from the development of existing vacant lands.

This table includes existing lands, lands planned for redevelopment, and undeveloped lands planned for development. The buildout land use projections include approximately 8,800 acres of residential and 5,900 acres of non-residential uses. These acreages were extracted from shapefiles provided by District staff, which consolidated local general plan land uses. For the purposes of this master plan, land use categories with similar densities were consolidated further for ease of reference.





# **Table 2.2 5 Year Growth Assumptions**

Water Facilities Master Plan West Valley Water District

**PRELIMINARY** 

Pressure Zone ID	Development Designation	Projected EDUs
South Syste	em	
Zone 2		
	Miscellaneous Infill	200
	Subtotal	200
Zone 3		
	Wildrose Village- Phase 1	110
	Wildrose Village - Phase 2	64
	Miscellaneous Infill	230
	Subtotal	404
Zone 3A		
	Crestwood Communities	50
	Subtotal	50
North Syste	em	
Zone 4		
	Pepper Avenue Specific Plan	50
	Miscellaneous Infill	10
	Subtotal	60
Zone 5		
	Renaissance	50
	Lytle Creek Ranch	900
	Miscellaneous Infill	50
	Subtotal	1,000
Zone 6		
	Renaissance	50
	Arboretum - Meadow	200
	Arboretum - Garden	700
	Shady Trails - Phase 1	100
	Shady Trails - Phase 2	137
	Miscellaneous Infill	50
	Summit at Rosena Development	480

# **Table 2.2 5 Year Growth Assumptions**

Water Facilities Master Plan West Valley Water District

**PRELIMINARY** 

Presssure Zone ID	Development Designation	t	Projected EDUs
	Tract 18944		90
		Subtotal	1,807
Zone 7			
	Arboretum - Meadow		390
	Arboretum - Garden		700
	Sierra Crest II		180
	Monarch Hills		472
	Lytle Creek Ranch		100
	Rosena Ranch		400
	D.R. Horton		80
	Tract 18944		90
		Subtotal	2,412
Zone 8			
	Lytle Creek Ranch		390
		Subtotal	390
AKEI	G	rand Total	6,323
ENGINEERING GROUP, INC.			3/13/2018

Source: Development information provided by WVWD staff.

**Table 2.3 Existing and Future Service Area Land Use** 

Water Facilities Master Plan West Valley Water District

**PRELIMINARY** 

	E	xisting Service A	Area		Inside Sphere	of Influence	
Land Use Classification	Existing Development	Existing Lands - Redeveloped	Subtotal Existing Lands - Unchanged	New Lands - Redevelopment		elopment Outside Existing Service Area	Total
1	(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	(acres)
Residential							
Residential 2	1,080	5	1,074	200	721	6	2,002
Residential 6	4,026	412	3,614	231	1,905	5	5,756
Residential 12	4	4	0	147	409	27	583
Residential 21	87	4	83	42	503	57	685
Subtotal- Residential	5,196	425	4,772	621	3,538	95	9,025
Non-Residential							
Commercial	123	65	58	604	323	18	1,004
Retail	121	117	4	96	84	0	184
Office	72	63	9	13	42	0	64
Educational	373	75	299	35	48	0	382
Institutional	129	121	8	283	192	0	482
Public Facility	324	271	53	32	99	0	184
Light Industrial	1,022	698	324	318	104	0	746
Heavy Industrial	510	348	162	178	302	0	643
Industrial	1,983	822	1,161	702	370	0	2,233
Subtotal-Non Residential	4,657	2,579	2,077	2,260	1,565	18	5,921
Other							
Utilities	293	70	223	46	316	0	585
ROW	110	75	35	15	60	0	110
Landscape Irrigation	238	161	77	10	114	25	226
Open Space	0	0	0	327	1,688	195	2,210
Vacant-Undeveloped	5,538	0	0	0	0	0	0
Subtotal- Other	6,179	306	335	397	2,178	219	3,130
Total							
AKEL	16,032	3,310	7,184	3,278	7,281	333	2/12/20

#### 2.3 HISTORICAL AND FUTURE POPULATION

The historical population (Table 2.4) was extracted from the District's 2015 Urban Water Management Plan (UWMP), which utilized population estimates prepared by the Southern California Association of Governments (SCAG). The methodologies for calculating the projected population varied and are briefly summarized as follows:

- 2018-2022: Linearly interpolated between 2017 and 2023 based on the 5-year projected growth.
- 2023: Based on 5-year projected growth assuming 3.5 people per EDU.
- 2024-2039: Linearly interpolated between the 2022 population and the 2040 population projected in the District 2015 UWMP.
- 2040: Extracted from the District 2015 UWMP.
- 2041-2057: Calculated assuming 1.5% annual population growth, consistent with 2015 UWMP growth rate.

Though historical populations were used in understanding the domestic water consumption behaviors and trends, population forecasts are presented for informational purposes only. Estimates of future domestic water demands were not based on population, but rather on net acreage for residential and non-residential land uses. Future population and EDUs were used as a means for estimating the planning horizon of the water system and phasing improvements.

#### 2.4 CLIMATE

This section documents the existing climate for the District service area, as well as the potential effects of climate change.

#### 2.4.1 Existing Climate

The climate for the West Valley Water District is generally characterized by hot, dry summers and cool winters with intermittent rainfall. The bulk of the rainfall generally occurs in the months from November to April, with approximately 18.81 inches of rainfall typical to the area. The average high temperature in July and August ranges at approximately 95 degrees Fahrenheit, with the average low in December and February at approximately 42 degrees Fahrenheit. It should be noted that the San Gabriel Mountains border the northern extent of the service area, and form the Lytle Creek catchment. Rainfall amounts can rise significantly closer to the mountains due to orographic lifting.

#### 2.4.2 Climate Change

The 2015 San Bernardino Valley Regional Urban Water Management Plan (SBVR-UWMP) included the West Valley Water District, and documents the potential effects of climate change on the region. This document sources information from the Upper Santa Ana River Watershed Integrated Regional Water Management Plan and the Climate Change Vulnerability Assessment Checklist.

**Table 2.4 Historical and Projected Population** 

Water Facilities Master Plan West Valley Water District

**PRELIMINARY** 

					PRELIMINARY
		Annual			Per Capita
Year	Population 1,2	Growth	Average Annu	ial Demand	Consumption <sup>5</sup>
		(%)	(AF)	(mgd)	(gpdc)
Historical Popula	tion <sup>1</sup>				
2005	66,442		19,796	17.7	266
2006	67,821	2.1%	22,347	20.0	294
2007	69,228		23,167	20.0	299
2007	•	2.1% 2.1%	•	20.7	299
2008	70,665 72,131	2.1%	23,638 20,444	18.3	253
2010	•		19,556	17.5	238
2010	73,469	1.9%	•	17.5 17.4	232
	74,807	1.8%	19,479		
2012	76,145	1.8%	21,243	19.0	249
2013	77,483	1.8%	20,535	18.3	237
2014	78,821	1.7%	20,229	18.1	229
2015	80,161	1.7%	17,006	15.2	189
2016	82,013	2.3%	16,301	14.6	177
2017	83,902	2.3%	18,778	16.8	200
Projected Popula		4.40/	10.656	47.6	200
2018	87,590	4.4%	19,656	17.6	200
2019	91,279	4.2%	20,538	18.3	201
2020	94,967	4.0%	21,424	19.1	201
2021	98,656	3.9%	22,315	19.9	202
2022	102,344	3.7%	23,210	20.7	202
2023	106,033	3.6%	24,109	21.5	203
2024	106,593	0.5%	24,300	21.7	204
2025	107,154	0.5%	24,492	21.9	204
2026	107,715	0.5%	24,684	22.0	205
2027	108,276	0.5%	24,877	22.2	205
2028	108,837	0.5%	25,070	22.4	206
2029	109,398	0.5%	25,265	22.6	206
2030	109,959	0.5%	25,460	22.7	207
2031	110,520	0.5%	25,655	22.9	207
2032	111,081	0.5%	25,851	23.1	208
2033	111,642	0.5%	26,048	23.3	208
2034	112,203	0.5%	26,246	23.4	209
2035	112,763	0.5%	26,444	23.6	209
2036	113,324	0.5%	26,643	23.8	210
2037	113,885	0.5%	26,843	24.0	210
2038	114,446	0.5%	27,043	24.1	211
2039	115,007	0.5%	27,244	24.3	212
2040	115,568	0.5%	27,312	24.5	212
2041	117,302	1.5%	27,857	24.9	212
2042	119,061	1.5%	28,275	25.2	212
2043	120,847	1.5%	28,699	25.6	212
2044	122,660	1.5%	29,129	26.0	212
2045	124,500	1.5%	29,566	26.4	212

# **Table 2.4 Historical and Projected Population**

# Water Facilities Master Plan West Valley Water District

**PRELIMINARY** 

Year	Population <sup>1,2</sup>	Annual Growth	Average Annu	al Demand <sup>3,4</sup>	Per Capita Consumption <sup>5</sup>
		(%)	(AF)	(mgd)	(gpdc)
2046	126,367	1.5%	30,010	26.8	212
2047	128,263	1.5%	30,460	27.2	212
2048	130,186	1.5%	30,917	27.6	212
2049	132,139	1.5%	31,381	28.0	212
2050	134,121	1.5%	31,851	28.4	212
2051	136,133	1.5%	32,329	28.9	212
2052	138,175	1.5%	32,814	29.3	212
2053	140,248	1.5%	33,306	29.7	212
2054	142,352	1.5%	33,806	30.2	212
2055	144,487	1.5%	34,313	30.6	212
2056	146,654	1.5%	34,828	31.1	212
2057	148,854	1.5%	35,350	31.6	212
LA K E L					

ENGINEERING GROUP, INC.
Notes:
9/19/2019

- 1. Unless noted otherwise, historical population extracted from 2015 UWMP.
  - Year 2005 2009, 2015: Extracted from 2015 UWMP WVWD SBX7-7 Table 5
  - Year 2010 2014: Straight line linear interpolation between 2009 and 2015
  - Year 2016: Extracted from 2016 Year End Report received June 15, 2017
  - Year 2017: Extracted from "Population Estimates 2017" spreadsheet received June 15, 2017
- 2. Population Projection Source:
  - Years 2018 2022: Linearly interpolated between 2017 and 2023
  - Year 2023: Population growth based on 5-Year Growth Assumptions provided by District staff
  - Years 2024 2039: Linearly interpolated between 2023 and 2040
  - Years 2040: 2015 Urban Water Management Plan
  - Years 2041 2057: Calculated assuming 1.5% annual population growth, consistent with 2015 UWMP
- 3. Historical demand extracted from production statistics received from WVWD staff October 30, 2017. Historical demands exclude water produced for wholesale delivery to other agencies.
- 4. Demand Projection Source.
  - Years 2018 2022: Demand linearly interpolated between 2017 and 2023
  - Year 2023: Additional demand due to 5 year growth, assuming 670 gpd/EDU, and accounting for conservation.
  - Year 2024 2039: Demand linearly interpolated between 2023 and 2040
  - Years 2040: 2015 Urban Water Management Plan
  - Years 2041 2057: Calculated assuming per capita demand factor of 212 gpdc, consistent with 2015 UWMP demand projection methodology.
- 5. The 2015 UWMP calculated a 2020 Per Capita Water Use Target of 232 gpcd and a 2015 actual per capita water use of 190 gpcd. For demand planning purposes the UWMP used a per capita water use of 209 gpcd (10% increase over 2015). Accounting for water losses and occupancy vacancies the 2019 WFMP uses a per capita water use of 212 gpcd.

The recent climate modeling documented in the SBVR-UWMP indicates that temperatures are expected to rise. The City of Riverside is expected to experience almost double the days exceeding 95 degrees Fahrenheit by 2070 than what were historically recorded. Big Bear, which historically has had no days of 95 degree heat, is expected to have 4 days exceeding this threshold by 2070. The causal effects of the increasing climate temperatures are the reduction in alpine and sub-alpine forestation, and increasing storm intensities with decreasing frequency. The reduction in forest matter with increasing storm intensities are expected to exacerbate flooding concerns. Furthermore, the increase in temperature is expected to elevate mean snow levels, and thus reduce snowpack and yearly groundwater recharge.

The two methods for addressing the changing climate are documented as mitigation and adaptation. Mitigation efforts involve programs and policies intended to reduce carbon emissions, while adaptation efforts involve adjusting to the outcomes of climate change (risk of flooding, temperature increase, etc). It is recommended that as scientific advancements in climate change occur, and the impacts to water infrastructure are documented, that the District plan for efforts in both adaptation and mitigation.

# **CHAPTER 3 - SYSTEM PERFORMANCE AND DESIGN CRITERIA**

This chapter presents the District's performance and design criteria, which was used in this analysis for identifying current system capacity deficiencies and for sizing proposed distribution mains, storage reservoirs, pump stations and wells.

## 3.1 HISTORICAL WATER USE TRENDS

The historical domestic water consumption per capita was calculated to determine the average water use per capita per day. This was accomplished by dividing the District's historical water production by the historical population for the respective year.

The District's historical per capita consumption factors, for the period 2005-2016, are listed in **Table 3.1**. The per capita consumption has generally decreased since 2005, being reduced by approximately 20%. This trend is largely attributed to the District's effort of implementing water conservation measures. **Table 3.2** lists the last four years of monthly water production for the District from 2013 to 2016.

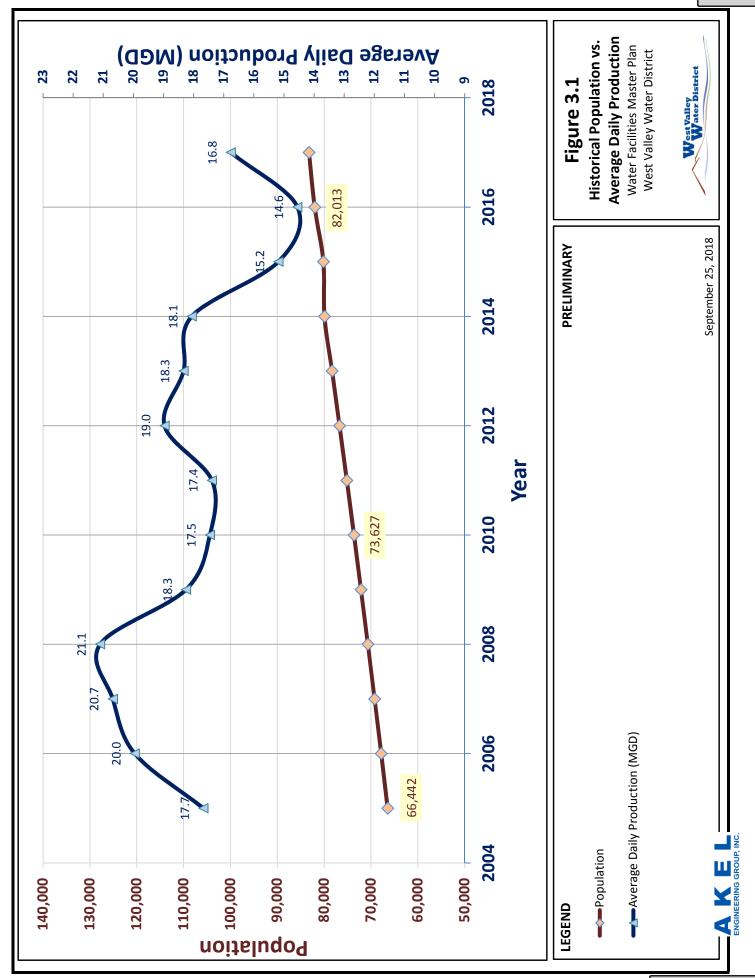
The ultimate demand forecasts included in this master plan for residential and non-residential land uses is based on net acreages. However, to generalize trends in the District's water use, per capita water use was documented. Figure 3.1 displays the historical population in relation to average daily water production. Figure 3.2 displays a comparison in the per capita water use and average daily water production. The remainder of the District's criteria are summarized in the following sections and on Table 3.3.

#### 3.2 SUPPLY CRITERIA

In determining the adequacy of the domestic water supply facilities, the source must be large enough to meet the varying water demand conditions, as well as provide sufficient water during potential emergencies such as power outages and natural or created disasters.

Ideally, a water distribution system should be operated at a constant water supply rate with consistent supply from the water source. On the day of peak day demand it is desirable to maintain a water supply rate equal to the peak day rate. Water required for peak hour demands or for fire flows would come from storage.

The District currently uses a combination of groundwater wells, State Water Project (SWP) water and treated surface water from Lytle Creek to meet the varying demand conditions of the existing customers. The minimum reliable supply to the surface water treatment facility is estimated to be approximately 4,000 afy, or 3.6 mgd. For supply planning purposes it is assumed that the total required groundwater supply shall be adequate to supply peak day demands less 4,000 afy, which is summarized on the following page.



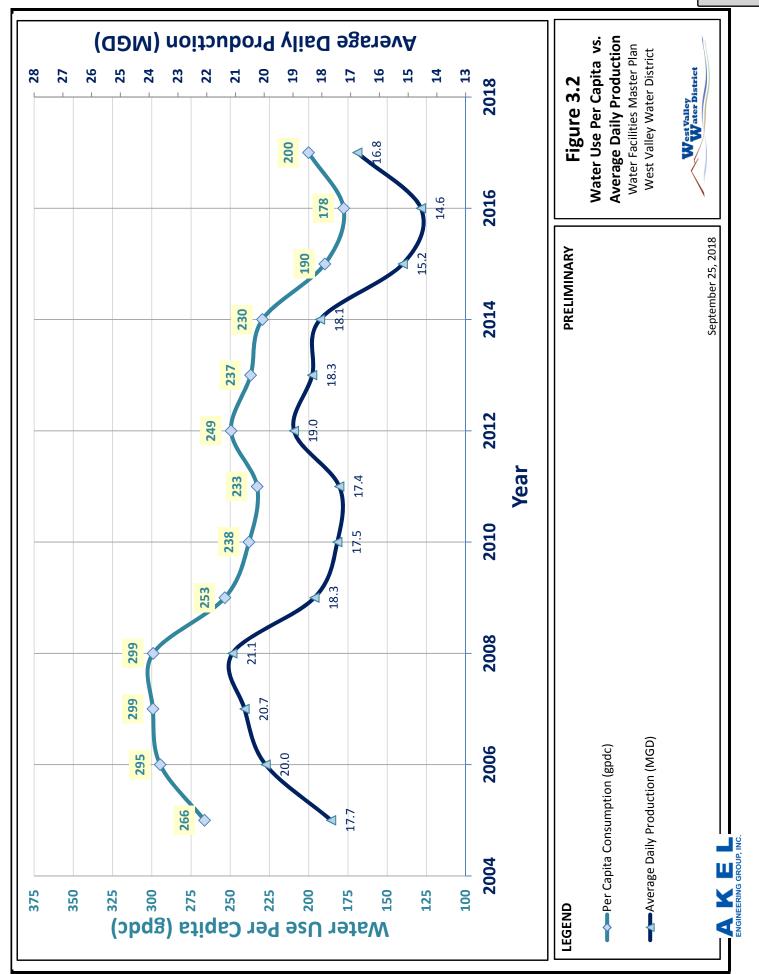


Table 3.1 Historical Annual Water Production and Peak Day Peaking Factors (2005-2017)

Water Facilities Master Plan

West Valley Water District

				_	IISTOrica	II Water	Historical Water Production	Ξ				
Year	Population <sup>1,2,3,4</sup> % Increase		Total Annual Production <sup>5</sup>	roduct	ion <sup>5</sup>		Mont	Monthly Production <sup>6</sup>	ction <sup>6</sup>	Daily Pr	Daily Production <sup>6</sup>	Average Daily Water Use per
		Consumption by WVWD	Delivered to Others	P	Total	Percent Increase	Maximum	Month of Occur.	Month of Max-to-Avg Occur. Ratio	Average	Max-to-Avg Ratio	Capita
		(AF)	(AF)	(AF)	(gpm)		(mgd)			(MGD)		(gpdc)
2002		19,796	1,355	21,151	13,114		27.49	July	1.46	17.7		592
2006	67,821 2.1%	22,347	1,970	24,317	15,078	15%	30.58	August	1.41	20.0		295
2002	69,228 2.1%	23,167	171	23,338	14,471	-4%	28.58	August	1.37	20.7		299
2008	70,665 2.1%	23,638	429	24,068	14,923	3%	28.38	August	1.32	21.1	1	299
5000	72,131 2.1%	20,444	1,137	21,581	13,381	-10%	24.97	August	1.30	18.3		253
2010	73,469 1.9%	19,556	1,210	20,766	12,876	-4%	25.19	August	1.36	17.5		238
2011	74,807 1.8%	19,479	1,146	20,624	12,788	-1%	27.25	ylut	1.48	17.4		233
2012	76,145 1.8%	21,243	1,294	22,537	13,974	%6	26.08	August	1.30	19.0		249
2013	77,483 1.8%	20,535	1,065	21,600	13,393	-4%	23.13	ylut	1.20	18.3		237
2014	78,821 1.7%	20,229	931	21,160	13,120	-5%	23.63	Иnг	1.25	18.1		230
2015	80,161 1.7%	17,006	1,191	18,197	11,283	-14%	18.62	August	1.15	15.2		190
2016	82,013 2.3%	16,301	2,070	18,371	11,391	1%	20.08	August	1.22	14.6		178
2017	83,902 2.3%	18,778	1,243	20,021	12,414	%6	22.47	July	1.26	16.8	ı	200
			Histori	sal Maxi	mum Pe	Historical Maximum Peaking Factors	ctors					
7-Year	7-Year Maximum (2011-2017)			22,537	13,974	%6	27		1.48	19.0	-	249
5-Year	5-Year Maximum (2013-2017)			21,600	13,393	%6	24		1.26	18.3	ı	237
3-Year	3-Year Maximum (2015-2017)			20,021	12,414	%6	22		1.26	16.8	ı	200
2017 N	2017 Maximum			20,021	12,414	%6	22		1.26	16.8	ı	200
			Reco	mmend	ed Peak	<b>Recommended Peaking Factors</b>	ors					
	2012 Water System Master Plan Criteria	Plan Criteria									1.70	
	2019 Water System Master Plan	Plan							1 40		1 70	

1. Historical Population from 2005 to 2014 extracted from the District's Public Water System Statistics provided by District staff September 12, 2019

<sup>2. 2015</sup> population extracted from 2015 Urban Water Management Plan

<sup>3. 2016</sup> population extracted from "2016 Year End Report", provided by District Staff on June 17, 2017

<sup>4. 2017</sup> population extracted from "2017 Year End Report", provided by District Staff on September 25, 2018

<sup>5.</sup> Annual production statistics received September 25, 2018 (including distinction between actual WVWD consumption and water delivered to others (WVWD customers versus Water Wholesale to other agencies).

<sup>6.</sup> Source : Public Water System Statistics received from District staff June 15, 2017. "Year end report" for year 2016, received June 15, 2017. Monthly and Daily Production Statistics not including water wholesale to other agencies.

Table 3.2 Historical Monthly Water Production (2015-2017)

Water Facilities Master Plan West Valley Water District

**PRELIMINARY** 

		2015			2016			2017	
Month	Mon	Monthly <sup>1</sup>	Peaking Factor	Mon	Monthly <sup>2</sup>	Peaking Factor	Mon	Monthly <sup>3</sup>	Peaking Factor
	Production (mgd)	Percent of Annual (%)	Month to Avg Factor	Production (mgd)	Percent of Annual (%)	Month to Avg Factor	Production (mgd)	Percent of Annual (%)	Month to Avg Factor
January	12.6	%/	0.83	9.0	2%	0.62	8.22	4%	0.49
February	12.4	%/	0.82	11.0	%9	0.75	8.34	4%	0.50
March	14.5	%8	96.0	11.8	%/	0.81	12.63	%9	0.76
April	17.2	%6	1.14	12.1	%/	0.83	16.39	%8	0.98
Мау	15.2	%8	1.00	14.2	%8	0.98	17.27	%6	1.03
June	18.5	10%	1.22	17.8	10%	1.22	20.41	10%	1.22
ylut	17.0	%6	1.12	20.0	11%	1.38	22.47	11%	1.34
August	18.6	10%	1.23	20.1	12%	1.38	20.72	10%	1.24
September	16.5	%6	1.09	17.5	10%	1.20	19.16	10%	1.15
October	14.1	%8	0.93	15.6	%6	1.07	19.56	10%	1.17
November	13.3	%/	0.88	14.0	%	96.0	18.08	%6	1.08
December	12.1	7%	0.80	11.4	7%	0.78	17.32	%6	1.04
Total Average Value	182.1			174.4			200.6		
Maximum Value	18.6		1.23	20.1		1.38	22.5		1.34
ENGINEERING GROUP, INC.									9/25/2018

1. PWSS Statistics received from District Staff, not including water deliveries to customers outside the District Service Area (wholesale to other agencies)

Notes:

<sup>2.</sup> Monthly Production extracted from " 2016 Year End Report", received from District Staff 06/15/2017. Does not include wholesale to other agencies. 3. Monthly Production extracted from " 2017 Year End Report", received from District Staff 09/25/2018. Does not include wholesale to other agencies.

#### **Table 3.3 Planning and Design Criteria**

Water Facilities Master Plan West Valley Water District

PRELIMINARY

Design Parameter		Criteria
Supply Requirement	Supply to meet Peak Day Demand with firm capacity	y only
	Peak day pumping shall be based on 16 hour of pum	nping/ day
Storage Requirement	Total Required Storage = Operational + Fire (For Zor	ne 2, 3, 3A, 8)
	Total Required Storage = Operational + Fire + Pump	ing (For Zone 4. 5. 6. & 7)
		100% of Peak Day Demand
	Operational storage	100% of Feak Day Demand
	Fire Storage	Low Density Residential: 0.18 MG (1,500 gpm for 2 hours)
		High Density Residential: 0.54 MG (3,000 gpm for 3 hours)
		Schools/Commercial: 0.54 MG (3,000 gpm for 3 hours)
		Office/Light Industrial: 0.54 MG (3,000 gpm for 3 hours)
		Heavy Industrial: 0.96 MG (4,000 gpm for 4 hours)
1		1.00% Average Day Demand for Supply Dependent Pumping Zones
Pump Stations <sup>1</sup>	rump Stations shall meet Peak Day Demand with re schedule).	espective firm capacity of Pressure Zone (on a 16-hour per day pur
	·	n of the total capacity of each pump station pumping into the pres
	zone, with each pump station operating without th	eir largest unit.
Pressure Reducing Valves <sup>1</sup>	PRV should be designed to meet the greater of:	
	Peak Hour Demand, or Peak Day Demand + Fire Flow	w
Pipelines	Pipelines should be designed to meet the greater of	f:
	1) Peak Hour Demand, or 2) Peak Day Demand + I	
	Criteria for existing and future pipelines inclu	
	·	oft/s during Peak Day Demand
		LO ft/s during Peak Day Demand + Fire Flow
		5 ft/1,000 ft during Peak Day Demand (assuming a C-Factor of 120)
	Dead-end pipelines shall not exceed 660 feet in leng	gth
Service Pressures	Maximum Pressure	
	In Pipelines 1	•
	At Service Connections 8	30 psi
	Minimum Pressure	
	Peak Hour Demand 4	·
Domand Dooking Factors	Peak Day Demand + Fire Flow 2	•
Demand Peaking Factors		1.40 x Average Day Demand
	· ·	L.70 x Average Day Demand
Water Demand Factors		1.70 x Peak Day Demand 212 gallons per capita per day (gpcd)
water Demailu Factors	EDU Water Use 6	
Fire Flows	Low Density Residential 1	
1110110003	High Density Residential 3	
	Schools/Commercial 3	
	Office/Light Industrial 3	
	Office/Light moustrial 5	1,000 gpm for 4 hours

Notes:

 $1. \ \ Criteria\ not\ included\ in\ District\ 2012\ Water\ Master\ Plan.\ Criteria\ shown\ recommended\ by\ Akel\ Engineering\ Group.$ 

2. Water use rate consistent with 2020 per capita water use target per District 2015 Urban Water Management Plan.

2/9/2018

Total Required Groundwater Supply = Peak Day Demands – 4,000 afy (3.6 mgd)

#### 3.3 STORAGE CRITERIA

The intent of domestic water storage is to provide supply for operational equalization, fire protection, and other emergencies, such as power outages or supply outages. Operational or equalization storage provides the difference in quantity between the customer's peak hour demands and the system's available reliable supply. The District storage criteria varies depending on what pressure zone is being served.

#### 3.3.1 Typical Storage Criteria

The District's storage criteria consists of three main elements: operational, fire flow, and pumping.

#### **Operational Storage**

Operational or equalization storage capacity is necessary to reduce the variations imposed on the supply system by daily demand fluctuations. Peak hour demands may require up to 2 times the amount of maximum day supply capacity. With storage in place, this increase in demand can be met by the operational storage rather than by increasing production from the supply sources. The District criteria for all pressure zones is to maintain an operational storage amount equal to 100 percent of peak day demand.

Operational Storage = 100% x PDD

#### **Fire Storage**

Fire storage is also needed to mitigate potential emergencies that may occur in the pressure zone, and in compliance with relevant fire codes. The recommended fire storage capacity varies by pressure zone and land use type, and is usually higher for commercial and industrial areas. Fire flow provisions for each pressure zone were calculated based on the governing (highest) land use type within a reservoir service area as follows:

- Low Density Residential: 1,500 gpm for 2 hours = 0.18 MG
- High Density Residential: 3,000 gpm for 3 hours = 0.54 MG
- Schools/Commercial: 3,000 gpm for 3 hours = 0.54 MG
- Office/Light Industrial: 3,000 gpm for 3 hours = 0.54 MG
- Heavy Industrial: 4,000 gpm for 4 hours = 0.96 MG

#### **Pumping Storage**

The majority of the District's existing and planned groundwater wells with pump stations convey through the North System. In order to ensure a sufficient volume of water is available for pumping

to meet the demands of the North System the District requires an additional amount of water to be stored in the water storage reservoirs. Therefore, Pressure Zones 4, 5, 6 and 7 carry additional pumping storage volumes for the respective higher zones, less the 4.0 mgd capacity of the WFF.

Pumping Storage = 100% x ADD of Supply Dependent Pressure Zones - 4.0 mgd

# **Total Storage Requirement**

The total storage (Qs) is the summation of operational (equalization), fire, and pumping storage requirements as follows:

For Pressure Zones 2, 3, 3A, 8:

Qs =Peak Day Demand + fire flow (varies)

For Pressure Zones 4, 5, 6, 7:

Qs =Peak Day Demand + fire flow (varies) + Pumping (varies)

#### 3.4 PRESSURE CRITERIA

Acceptable service pressures within distribution systems vary depending on District criteria and pressure zone topography. It is essential that the water pressure in a consumer's residence or place of business be maintained within an acceptable range. Low pressures below 30 psi can cause undesirable flow reductions when multiple faucets or water using appliances are used at once.

Excessively high pressures can cause faucets to leak and valve seats to wear out prematurely. Additionally, high service pressures can cause unnecessarily high flow rates, which can result in wasted water and high utility bills. The criteria for pressures in the domestic water system include the following:

- Maximum pressure, usually experienced during low demands and winter months
- Minimum pressure, usually experienced during peak hour demands and summer months
- Minimum pressure during simultaneous peak day demand and fire flow

The American Water Works Association Manual on Computer Modeling and Water Distribution System (AWWA M-32) indicates that maximum pressures are usually in the range of 90-110 pounds per square inch (psi). In some communities, the maximum pressure may be limited to 80 psi to mitigate the impact on internal plumbing. In this case, the distribution system is usually sized for the higher pressures, and individual pressure-reducing valves are installed on service lines where the pressure may be exceeded.

The minimum acceptable pressure is usually in the range of 40-50 psi, which generally provides for sufficient pressures for second story fixtures. When backflow preventers are required, they may reduce the pressures by approximately 5-15 psi. The recommended minimum pressure during fire flows is 20 psi, as established by the National Fire Protection Association (NFPA).

The District's pressure criteria are summarized as follows:

- Maximum pressure (pipelines): 130 psi
- Maximum pressure (service connections): 80 psi
- Minimum pressure (PHD): 40 psi
- Minimum pressure (PDD + Fire Flow): 20 psi

#### 3.5 UNIT FACTORS

Domestic water demand unit factors are coefficients commonly used in planning level analysis to estimate future average daily demands for areas with predetermined land uses. The unit factors are multiplied by net acreages to yield the average daily demand projections.

The total domestic water demand was extracted from consumption data maintained by the District. The demand was adjusted to balance with current production records, and to account for transmission main losses and vacancies in existing land uses. For planning purposes, the production used to develop the water demand unit factors was based on 2014 production data minus ten percent to account for current water conservation trends. The demand unit factor was then calculated using the calculated water production and total number of residential and non-residential land use acreages.

This analysis generally indicates that existing residential land uses have higher consumptive use factors than that of non-residential land uses. The existing unit factor analysis is shown on **Table 3.4**. It should be noted that extensive water conservation efforts have reduced water demands beyond the required "20x2020" target water use. The water production target of 2014 minus 10 percent is below the "20x2020" target, but is considered reasonable and conservative based on 2015 and 2016 production records. The water demand unit factors are summarized on **Table 3.5**. It should be noted that the existing industrial factors are low compared to industry standards, and were adjusted to reflect more conservative planning assumptions.

It should be noted that the water demand unit factors utilized in this WFMP are generally lower for all land use types as compared to the 2012 WMP. A comparison of the water demand unit factors is included in **Appendix A**. The water demand unit factors prepared as part of this master plan reflect changes in water use due to recent drought conditions, as well as a revised land use analysis.

Table 3.4 Water Demand Unit Factor Analysis

Water Facilities Master Plan

West Valley Water District

	: ::				Existing Average	age Daily Water Demand Unit factors	mand Unit fa	ctors			
Land Use Classification	Development within Service		Consumption <sup>1</sup>		Produ	Production <sup>2</sup>	Prodi	Production at 100% Occupancy	ccupancy	Reco	Recommended Water Unit Factor
	Area	Unadjusted Water Unit Factors	Annual Consumption	ımption	Unadjusted Water Unit Factors	Production (w/o Vacancy rate)	Vacancy Rate <sup>3,4</sup>	Projected Pro	Projected Production at 100% Occupancy	Recommende d Factor	Balance Using Recommended Unit Factor
	(net acres)	(gpd/net acres)	(pdB)	(mdg)	(gpd/net acres)	(pdB)	(%)	(gpd/net acres)	(pdB)	(gpd/net acres)	(pdg)
Residential											
Residential 2	1,080	734	792,487	550	926	1,000,047	2.9%	984	1,062,750	066	1,068,792
Residential 6	4,026	1,974	7,945,858	5,518	2,491	10,026,958	2.9%	2,647	10,655,641	2,650	10,667,777
Residential 12	4	3,414	12,569	6	4,308	15,861	2.9%	4,578	16,856	4,580	16,864
Residential 21	87	4,196	367,009	255	5,295	463,133	2.9%	5,627	492,171	5,630	492,419
Subtotal Residential	5,196		9,117,923	6,332		11,505,999			12,227,417		12,245,852
Non-Residential											
Commercial	123	1,249	154,053	107	1,576	194,401	12.1%	1,794	221,254	1,800	221,977
Retail	121	1,311	158,092	110	1,655	199,498	12.1%	1,884	227,055	1,890	227,828
Office	72	981	70,462	49	1,238	88,916	12.1%	1,409	101,198	1,410	101,302
Educational	373	1,415	528,135	367	1,786	666,459	%0.0	1,786	666,459	1,790	667,905
Institutional	129	1,112	142,911	66	1,403	180,341	%0.0	1,403	180,341	1,410	181,224
Public Facility	324	191	61,965	43	241	78,194	%0:0	241	78,194	250	81,009
Light Industrial	1,022	380	388,224	270	479	489,904	4.6%	502	513,508	200	511,143
Industrial	1,983	332	657,527	457	418	829,740	4.6%	439	869,718	1,000	1,983,076
Heavy Industrial	510	1,149	586,004	407	1,451	739,484	4.6%	1,520	775,113	1,530	780,002
Subtotal - Non-Residential	4,657		2,747,373	1,908		3,466,938			3,632,842		4,755,466
Other											
Landscape Irrigation <sup>6</sup>	450	2,125	956,577	664	2,681	1,207,114	%0:0	2,681	1,207,114	2,690	1,210,981
Marygold Mutual Water Company <sup>7</sup>			652,512			652,212			652,212		652,212
ROW	110	0	0	0	0	0	%0.0	0	0	0	0
Utilities	293	2	445	0	2	561	%0:0	2	561	10	2,931
Open Space	1,755	0	0	0	0	0	%0:0	0	0	0	0
Subtotal - Other	2,820		1,609,534	1,118		1,859,888			1,859,888		1,866,124
	12,673		13,474,831	9,358		16,832,825			17,720,146		18,867,442

1. Consumption extracted from the 2016 water meter shapefile database, provided by District Staff July 5, 2017. 2. Meters consumption was normalized to 2014 production records minus 10 percent (90% of 2014 Production Records).

3. Residential vacancy rate extracted from California Department of Finance Sheet E-5 published 2016.

4. Non-residential vacancy rates extracted from Inland Empire 2013 market report prepared by Voit Real Estate Services, downloaded September 11, 2017. Vacancy rates shown are average of rates for the cities of Fontana, Rialto, and Colton.

5. Residential Landuse categories extracted from the 2010 General Plan Landuse, published by the City of Rialto. 6. Landscape irrigation acres include estimated acres for irrigated parkways, which were assumed at 1 acre per meter.

7. Marygold Mutual Water Company demand extracted from wholesale water sale information included in water billing records received from District staff July 5, 2017. Meter located south of the intersection of Randall Avenue and Cedar Avenue.

**Table 3.5 Recommended Water Unit Factors** 

Water Facilities Master Plan West Valley Water District

**PRELIMINARY** 

Land Use Designation	Recommende	d Water Factor
	(gpd/ acre)	(gpm/acre)
Residential		
Residential 2	990	0.69
Residential 6	2,650	1.84
Residential 12	4,580	3.18
Residential 21	5,630	3.91
Non-Residential		
Commercial	1,800	1.25
Retail	1,890	1.31
Office	1,410	0.98
Educational	1,790	1.24
Institutional	1,410	0.98
Public Facility	230	0.16
Light Industrial	500	0.35
Industrial	1,000	0.69
Heavy Industrial	1,530	1.06
Other		
Landscape Irrigation	2,690	1.87
ROW	0	0
Utilities	10	0.01
ENGINEERING GROUP, INC.		1/11/2019

1/11/2019

# 3.6 SEASONAL DEMANDS AND PEAKING FACTORS

Domestic water demands within municipal water systems vary with the time of day and month of the year. It is necessary to quantify this variability in demand so that the water distribution system can be evaluated and designed to provide reliable water service under these variable demand conditions.

Water use conditions that are of particular importance to water distribution systems include the average day demand (ADD), the peak month demand (PMD), the peak day demand (PDD), the peak hour demand (PHD), and the winter demand.

The average day demand represents the annual water demand, divided by 365 days, since it is expressed in daily units. The winter demand typically represents the low month water demands and is used for simulating water quality analysis.

#### 3.6.1 Peak Month Demand

The peak month demand (PMD) is the highest demand that occurs within a calendar month during a year. The District's PMD usually occurs in the summer months, in either July or August. The PMD is used primarily in the evaluation of supply capabilities.

Historical monthly water production records, obtained for the period between 2005 and 2015 (Table 3.1), indicate the maximum month to average month ratio ranging between 1.25 and 1.52. Over the reviewed period, this ratio showed increasing or decreasing trends. Therefore, a PMD factor of 1.40 was deemed representative of trends in the District service area. The following equation is recommended for estimating the maximum month demand, given the average day demand:

Peak Month Demand = 1.40 x Average Day Demand

#### 3.6.2 Peak Day Demand

The peak day demand is the highest demand that occurs within a 24 hour day during a year. The District's PDD, which usually occurs during the summer months, is typically used for the evaluation and design of storage facilities, distribution mains, pump stations, and pressure reducing valves. The PDD, when combined with fire flows, is one of the highest demands that these facilities should be able to service while maintaining acceptable pressures within the system.

The peak day demands were obtained from the District's water production records. Production records indicate the date of occurrence and magnitude of the peak day demand for each calendar year, as listed in Table 3.1. Monthly data was provided by the District for review of water demand trends and peaking factor evaluation. For the purposes of this Master Plan, the peak day demand factor is assumed at 1.7 times the average day demand and consistent with the previous master

plan. The following equation is then used to estimate the peak day demand, given the average day demand:

Peak Day Demand = 1.70 x Average Day Demand

#### 3.6.3 Peak Hour Demand

The peak hour demand is another high demand condition that is used in the evaluation and design of water distribution systems. The peak hour demand is the highest demand that occurs within a one-hour period during a year. The peak hour demand is considered to be the largest single measure of the maximum demand placed on the distribution system. The PHD is often compared to the MDD plus fire flow to determine the largest demand imposed on the system for the purpose of evaluating distribution mains.

A peak hour to peak day ratio of 1.7 was applied to the peak day demand to yield the peak hour demand ratio of 2.9, consistent with the District design standards. The peak hour demand can then be calculated using the average day demand and the following equation:

Peak Hour Demand = 1.70 x Peak Day Demand

#### 3.7 FIRE FLOWS

Fire flows are typically based on land use, with the potential for increased fire flow based on the building type. The following are the criteria for fire flows:

- Low Density Residential. Fire flows for low density residential land use types were calculated at 1,500 gpm for two hours.
- High Density Residential. Fire flows for high density residential land use types were calculated at 3,000 gpm for three hours.
- Schools/ Commercial. Fire flows for schools and commercial land use types were calculated at 3,000 gpm for three hours.
- Office/ Light Industrial. Fire flows for office and light industrial land use types was calculated at 3,000 gpm for three hours.
- Heavy Industrial. Fire flows for heavy industrial land use types were calculated at 4,000 gpm for four hours.

#### 3.8 TRANSMISSION AND DISTRIBUTION MAIN CRITERIA

Transmission and distribution mains are usually designed to convey the maximum expected flow condition. In municipal water systems, this condition is usually the greater of either the peak hour demand or the peak day demand plus fire flow. The hydrodynamics of pipe flow create two additional parameters that are taken into consideration when evaluating or sizing water mains: head loss and velocity.

Head loss is a loss of energy within pipes that is caused by the frictional effects of the inside surface of the pipe and friction within the moving fluid itself. Head loss creates a loss in pressure which is undesirable in water distribution systems. Head loss, by itself, is not a critical factor as long as the pressure criterion has not been violated. However, high head loss may be an indicator that the pipe is nearing the limit of its carrying capacity and may not have sufficient capacity to perform under stringent conditions. The District criterion for maximum pipeline head loss is summarized as follows:

Peak Day Demand: 5 feet per 1,000 feet of pipe

Since high flow velocities can cause damage to pipes and lead to high head loss, it is desirable to keep the velocity below a predetermined limit. The District criteria for maximum pipeline velocity are summarized as follows:

- Peak Day Demand: 5 feet per second
- Peak Day Demand + Fire Flow: 10 feet per second

These velocity criteria also ensure that the head loss is kept below an acceptable limit, as the head loss in a pipe is a function of the flow velocity. Flow velocities in transmission mains 14 inches and larger are governed by the head loss criteria.

A summary of the criteria pertaining to transmission and distribution mains is included in **Table 3.3**. The pipe roughness coefficient used for calculating head loss was based on the District criterion of 120.

It should be noted that the headloss criteria in transmission mains may be relaxed, where feasible, to account for transmission main redundancy and reliability. Relaxing of the criteria requires the review and approval of the District.

#### 3.9 TIME OF USE

Southern California Edison (SCE) has defined peak use times of the year where a tiered system of energy rates are implemented to encourage decreased energy consumption. Time of use is implemented from June 1 through September 30, which coincides with the maximum day and peak hour demands in the water system. There are three stages of energy rates during summer time of use:

- Off Peak: This category is typically associated with the lowest energy costs and occurs from 9:00 PM to 4:00 PM.
- Partial Peak: This category has medium energy costs and is intended to minimize energy use when possible. It occurs from 4:00 PM to 9:00 PM on weekends and holidays.
- On Peak: This is the highest cost category, and is intended to encourage users to avoid energy consumption whenever possible. It occurs from 4:00 PM to 9:00 PM.

District staff have been implementing time of use pumping, when possible, throughout their system to reduce operational costs. It should be noted that time of use pumping may impact the sizing of pipelines within pressure zones during nighttime replenishment pumping. This high pumping period is accounted for in this master plan analysis, and modeling scenarios reflect the time of use periods.

#### CHAPTER 4 - EXISTING DOMESTIC WATER FACILITIES

This chapter provides a description of the District's existing domestic water system facilities including the distribution mains, storage reservoir, booster pump stations and the existing wells.

### 4.1 EXISTING WATER SYSTEM OVERVIEW

The District operates a domestic water distribution system that consists of 21 groundwater wells, 25 separate storage reservoirs across eight pressure zones shown in Figure 4.1, for a total storage over 72 million gallons (MG), and over 375 miles of transmission and distribution pipelines.

The District's existing domestic water distribution system is shown in Figure 4.2, which displays the existing system by pipe size. This figure provides a general color coding for the distribution mains, as well as labeling the existing wells, booster stations, pressure reducing valves, and the storage reservoirs. Additionally, Figure 4.3 summarizes the existing system with pipelines colored based on pressure zone. A hydraulic profile based on the existing operations of the District's water system is provided on Figure 4.4. The District is generally divided into two sections, commonly referred to as the North System and South System, which are briefly summarized in the following sections.

### 4.1.1 North System

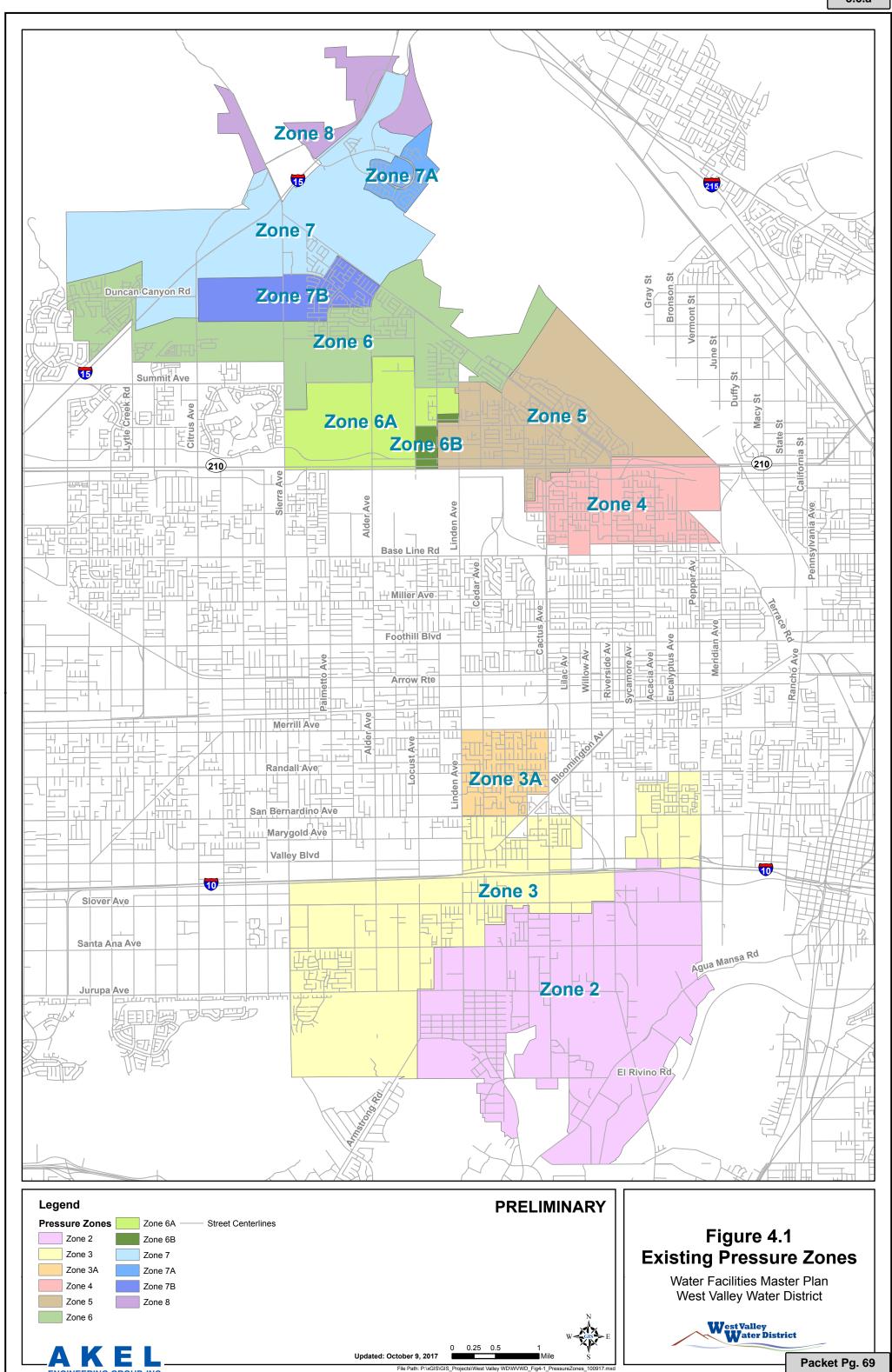
The District's North System, comprised of Pressure Zones 4, 5, 6, 7 and 8, provides domestic water service to the District's customers north of Baseline Road. Supply for this system is provided by multiple groundwater wells, the Roemer WFF in Pressure Zone 5, and water boosted from the Baseline Feeder to Pressure Zone 4 at the Lord Ranch Facility.

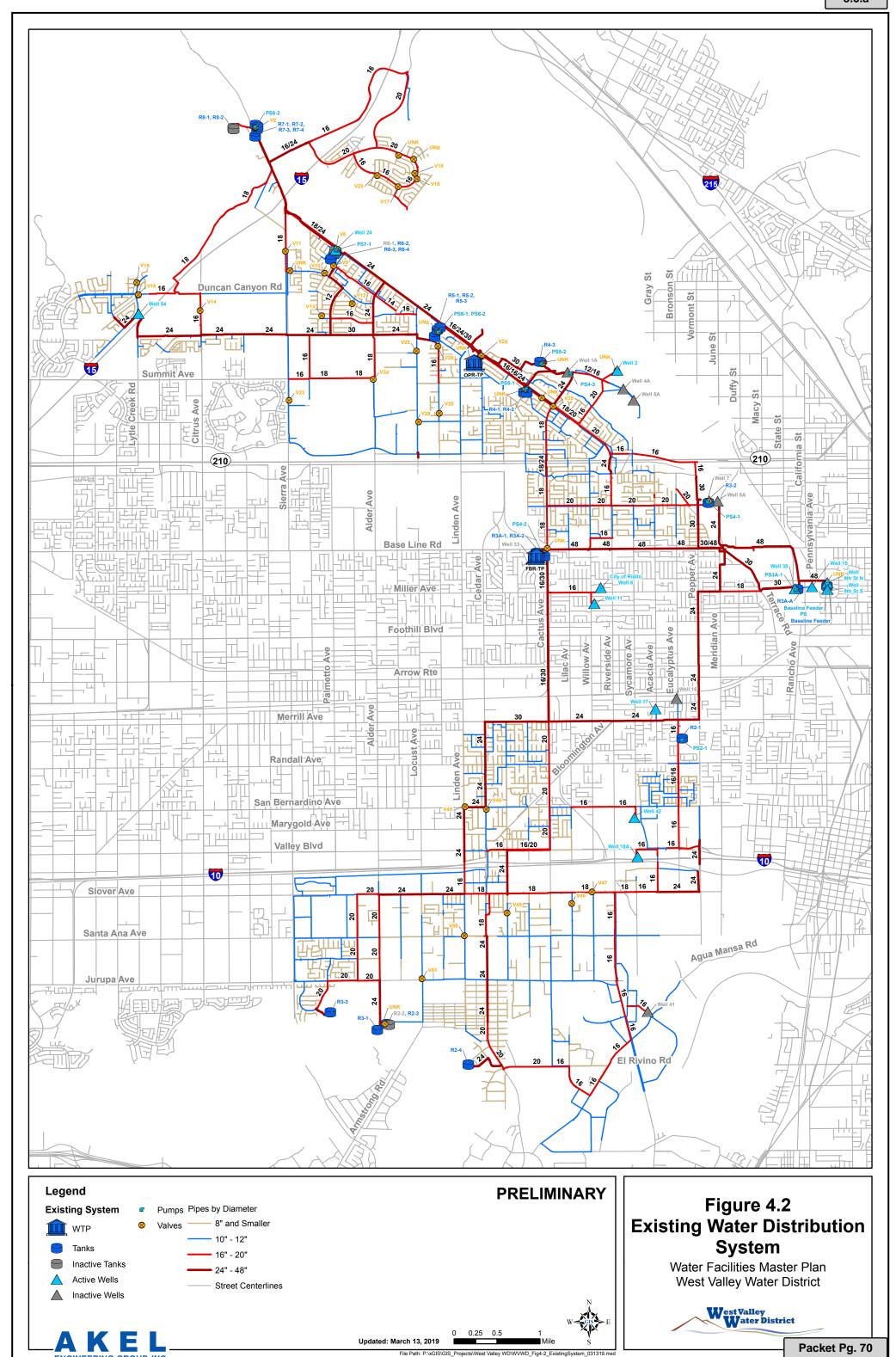
### 4.1.2 South System

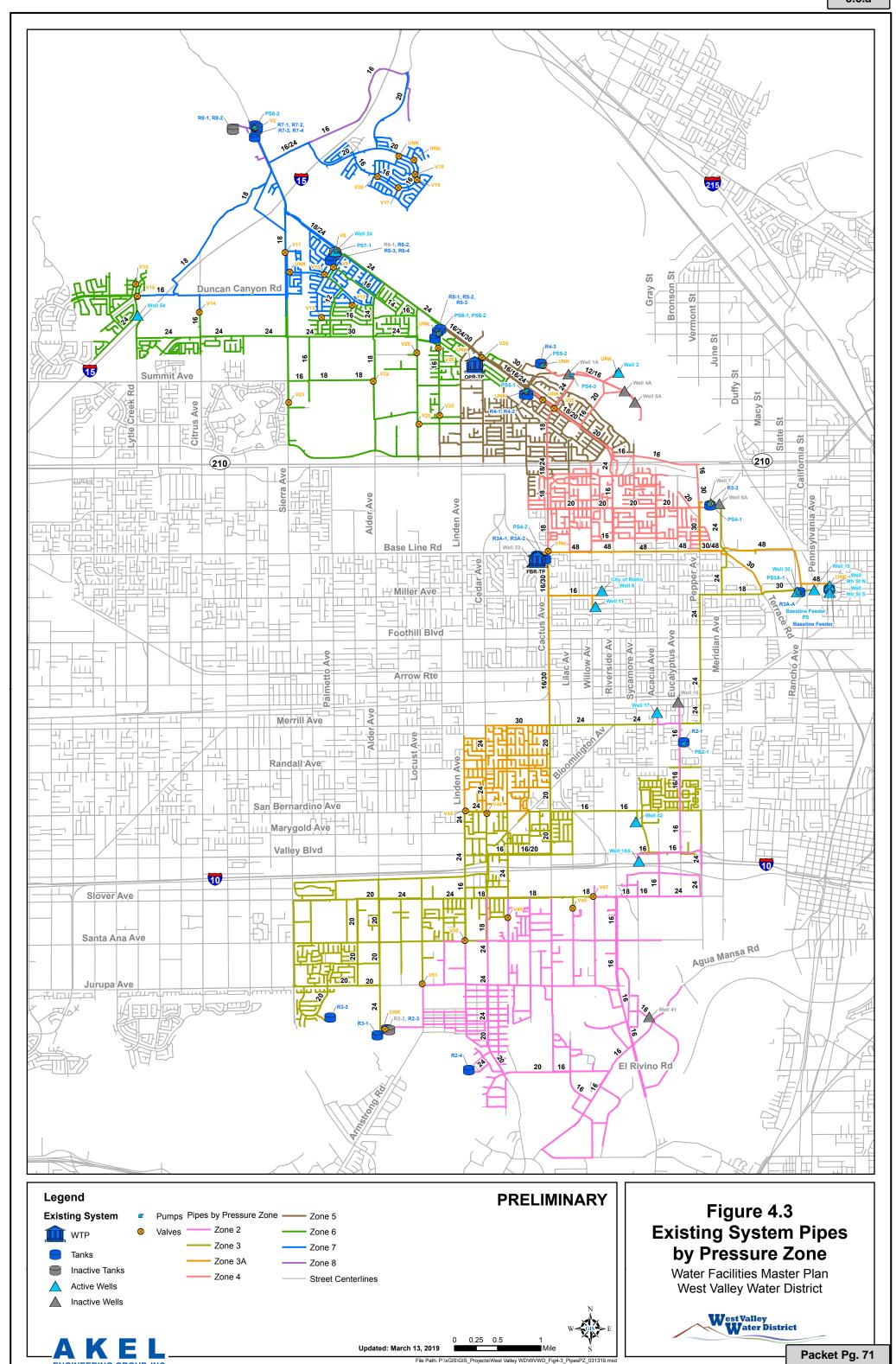
The District's South System, comprised of Pressure Zones 3A, 3, and 2, provides domestic water service to the District's customers generally located south of Merrill Avenue. Supply for this system is provided by multiple groundwater wells and the FBR treatment facility in Pressure Zone 3A.

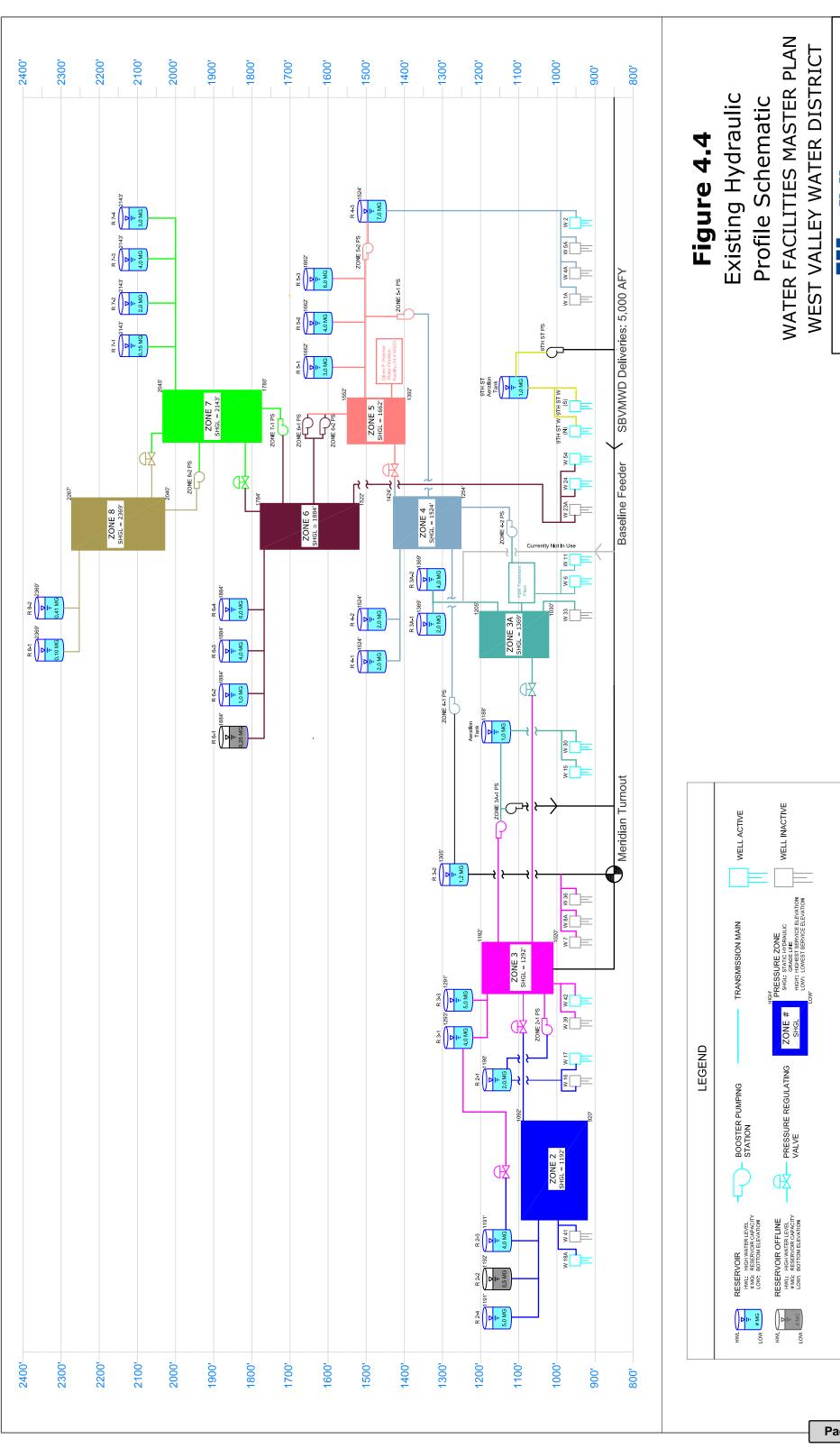
# 4.2 SOURCE OF SUPPLY

In order to meet existing domestic water demands, the District utilizes several sources of supply, including groundwater and treated surface water. The following section provides a brief summary of these sources, with a more detailed discussion provided in the Water Demands and Supply Characteristics chapter.









West Valley
PRELIMINARY

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Last Updated: 3/

Packet Pg. 72

**Table 4.1 Existing Groundwater Wells** 

PRELIMINARY

Supply	Zone	Groundwater	Location		Pump	Test Capacity <sup>1</sup>		Production		Operationa		PRELIMINARY
Well	Zone	Basin	Location	Flow	Rate	Total Dynamic	Test Year	Canacity <sup>2</sup>	Low D	emand	High D	emand
				(gpm)	(mgd)	Head (ft)		(mgd)	On (ft)	Off (ft)	On (ft)	Off (ft)
Active Gr	oundwate	er Wells		(8pm)	(64)	(10)		(mga)				
W-2	4	Lytle Creek	19973 Country Club Drive, Rialto	1,532	2.2	519	2017	1.47	18	20	18	20
W-4A	4	Lytle Creek	5914 N. Sycamore Avenue, Rialto	2,318	3.3	512	2017	2.23	9	11	12	14
W-5A	4	Lytle Creek	5914 N. Sycamore Avenue, Rialto	1,085	1.6	532	2017	1.04	8	10	10	12
W-11 <sup>4</sup>	3A	Rialto	238 W. Victoria St., Rialto	1,346	1.9	465	2017	1.29	VFD			
W-15	2,3,3A	Bunker Hill	1950 W. 9th St. San Bernardino	1,380	2.0	380	2016	1.32	24	26	24	26
W-17	2	Rialto	404 S. Acacia Avenue, Rialto	1,000	1.4		2010	0.96	10	18	10	18
W-18A	2	North Riverside	1783 S. Sycamore Avenue, Colton	2,170	3.1		2010	2.08	16	18	20	22
W-24	6	Rialto	4334 Riverside Avenue, Rialto	475	0.7	145	2017	0.46				
W-30	2,3,3A	Bunker Hill	2015 W. 9th St. San Bernardino	1,520	2.2	375	2016	1.46	22	24.5	22	24.5
W-42	3	North Riverside	295 E. San Bernardino Avenue, Rialto	1,625	2.3	578	2017	1.56	20	22	24	26
W-54	6	Rialto	Duncan Canyon Road, Fontana	920	1.3	930	2017	0.88	16	18	26	28
Rialto W-6 <sup>4</sup>	3A	Rialto	204 W. Etiwanda Ave.	1,870	2.7	451	2017	1.80	VFD			
			Total Well Capacity <sup>4</sup>	15,895	22.9			15.26				
			Firm Well Capacity <sup>4</sup> (largest unit out of service)	13,577	19.6			13.03				
Inactive (	Groundwa	ter Wells										
W-1A	4	Lytle Creek	19523 Country Club Drive, Rialto	822	1.2	367.1	2017	0.79				
W-7	3,4	Lytle Creek	6871 Martin PMP, San Bernardino	1,100	1.6		2010	1.06				
W-8A	3,4	Lytle Creek	6871 Martin Road, San Bernardino	1,700	2.4		2010	1.63				
W-41	2	North Riverside	3353 Industrial, Rialto	2,104	3.0	376.4	2016	2.02				
W-16		Rialto	296 S. Eucalyptus Avenue, Rialto	1,550	2.2		2010	1.49				
W-33	3A	Rialto	855 W Baseline Road, Rialto	2,517	3.6	425.3	2017	2.42				
W-23A	6	Rialto	4334 Riverside Avenue, Rialto	200	0.3		2010	0.19				
W-36	3,4	Lytle Creek	20600 Walnut Avenue, San Bernardino									
W-39	3	Chino	10272 Cedar Place, San Bernardino County					0.89				

Notes:

1. Source: Pump tests received from District staff August 2, 2017.

- 2. Production capacity assumes operating time of 16 hours per day.
- $3.\ Source: Operational\ control\ document\ received\ from\ District\ staff\ August\ 31,\ 2017.$
- 4. Well 11 and Rialto Well 6 both feed the District's Groundwater Wellhead Treatment System (FBR); only one well operates at any given time.

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#### 4.2.1 Groundwater Supply and Treatment Facilities

The District has 21 existing production wells, which are summarized on **Table 4.1**; this includes 12 active and nine inactive groundwater wells. As shown on **Table 4.1**; the firm capacity of the District's active groundwater wells is approximately 13,600 gpm. Rehabilitation, including water treatment, is needed to bring the remaining eight non-operational wells into production. The Kleinfelder firm was included as part of this team to evaluate the water supply and quality of the District's production wells.

Some wells are adversely impacted by contaminants, both human-caused and naturally occurring, which may limit the ability to use them as a source for consumption. The following documents the wells and their limiting water quality contaminant:

• Arsenic: Wells W-8A, W-36 and W-2

Perchlorate: Wells W-16, W-17, W-18A, W-33, W-41 and W-42

Nitrate: Wells W-16, W-18A, W-22A, W-39, and W-42

The District monitors groundwater quality and the movement of the groundwater contaminants, and in response to water quality concerns, groundwater treatment at the wellhead have been installed by the District on some wells. For example, well W-2 has Arsenic treatment and coagulation, and well W-11 has Perchlorate treatment.

A fluidized bed reactor (FBR) facility was constructed at the District's headquarters to remove perchlorate and nitrates. The FBR facility currently is used for perchlorate removal from the groundwater produced by wells W-11 and W-6. The process involves pumping groundwater from the two wells to the FBR, and additional downstream treatment facilities are utilized prior to discharge into the system, including: post-aeration tanks for treated water oxygenation, media filtration for solids removal, and a filtered water tank with a chlorination system for disinfection.

#### 4.2.2 Surface Water Supply

The Oliver P. Roemer Water Filtration Facility (Roemer WFF) treats raw water from Lytle Creek, and is supplemented with State Water Project (SWP) water from Silverwood Lake. The facility is designed to treat local Lytle Creek water, imported SWP water, and a blend of the two. Kleinfelder, included on the Master Plan team, evaluated the Roemer WFF and provided discussion and recommendations.

The current capacity of the Roemer WFF is 14.4 mgd. This treatment facility has a current maximum treatment capacity of 14.4 mgd with plans to expand to 20.4 mgd. The planned expansion assumes the construction of a 6.0 mgd membrane filtration plant. Two additional lead-lag granular activated carbon (GAC) vessel systems were installed in 2017. Appendix B documents figures from the previous master plan that include a flow schematic of the Roemer WFF and a plant site diagram of the Roemer WFF.

The current Roemer WFF consists of influent water blending ponds, rapid mixing/coagulation, flocculation, sedimentation and dual-media filtration. Filtered water is treated with GAC to remove volatile organic compounds (VOCs) and odor and taste contaminants; the filtered water ultimately is disinfected with ultraviolent (UV) light. The finished product water is chlorinated using free chlorine for further virus deactivation and to provide residual disinfectant in the distribution system.

The Roemer WFF also integrates auxiliary facilities including two filter backwash water ponds, three sludge disposal and drying ponds, multiple flow controlling/splitting structures, chemical storage building, Lytle Creek pump station, water distribution pump station, multiple intermediate pumping systems, electrical/power supply and instrumentation and control installations.

It should be noted that the City of Rialto owns 1.5 mgd of the Lytle Creek treated flows. Currently, the District delivers these flows through a connection with the City of Rialto at their Cedar Reservoir site, along Cedar Avenue south of Persimmon Avenue. The District delivers approximately 1.2 mgd, which can increase to the City of Rialto's owned capacity of 1.5 mgd depending on Lytle Creek flows.

#### 4.2.3 Baseline Feeder Pipeline

Beginning in 1998, the District began receiving water through what is known as the Baseline Feeder (BLF) pipeline. This pipeline was constructed in a joint venture with the City of Rialto and San Bernardino Valley Municipal Water District (SBVWMD). The current agreement with SBVWMD allows the District to receive up to 5,000 afy of supply through this 48-inch transmission pipeline.

In 2012, two new groundwater wells, along with an aeration tank and pump station, were constructed as part of the Baseline Feeder Well Replacement and Improvement project, which was implemented to provide adequate supply to meet the District's 5,000 afy allotment. Before this time the District received an average of 2,700 afy due to diminishing operational capacity of the original SBVWMD BLF supply wells.

Water is currently delivered to the existing system through the following two facilities.

#### 4.2.3.1 Meridian Turnout

The District receives water delivered through the BLF pipeline using a control structure at the intersection of Baseline Road and Meridian Avenue. This control structure, known as the Meridian Turnout, currently regulates the delivery of water to the District at the following locations:

- North from Baseline Road to the Lord Ranch Facility via a 24-inch pipeline
- South from Baseline Road to Pressure Zone 3 via a 24-inch pipeline

Based on current operating conditions, the Meridian Turnout prioritizes maintaining the level of the water storage reservoir 3-2, which serves as a forebay reservoir for pump station 4-1. Excess water in the BLF not required to maintain the tank level is transferred south to Pressure Zone 3.

#### 4.2.3.2 Lord Ranch Facility

The District currently relies on pump stations to transfer supply delivered via the BLF to Pressure Zone 4 and the higher North System pressure zones. Pump Station 4-1 is currently utilized as the primary pump station to convey BLF deliveries to Pressure Zone 4, and is referred to as the Lord Ranch Facility. This facility is currently comprised of a forebay water storage reservoir (Reservoir 3-2), and Pump Station 4-1. Water is delivered to the forebay reservoir via a 24-inch pipeline from the Meridian Turnout. A new pump station planned for this facility will be the primary pump station to transfer future water extracted from the Bunker Hill groundwater basin to Pressure Zone 4.

### 4.3 PRESSURE ZONES

The District's service area generally slopes upward from south to north, with service elevations ranging between 900 ft and 2,300 ft. Due to the varying terrain, the service area is divided into eight pressure zones to account for the changes in elevation.

#### 4.3.1 Zone 2 (SHGL = 1,192 feet)

Zone 2 is the southernmost zone in the District's southern system. It is generally bounded by the Santa Ana River and Riverside/San Bernardino County Line to the south, Locust, Maple and Cedar Avenues to the west, Interstate 10 to the north and Pepper Avenue to the east.

Elevations served in this pressure zone range from approximately 920 feet to 1,092 feet. This zone is supplied from one groundwater well (Well 18A) as well as PRVs from Zone 3; this zone has 3 active ground level storage reservoirs for a total storage capacity of 11.0 MG.

#### 4.3.2 Zone 3 (SHGL = 1,292 feet)

Zone 3, located within the District's southern system, is separated into two distinct areas that are divided by the City of Rialto. The first area is generally bounded by Sierra Avenue to the west and Zone 2 to the east, with San Bernardino Avenue and the Riverside/San Bernardino County Line serving as the northern and southern boundaries respectively. The second area is generally bounded by Sycamore Avenue to the west and Pepper Avenue to the east, with Randall Avenue and Interstate 10 serving as the northern and southern boundaries respectively.

Elevations served in this pressure zone range from approximately 1,020 feet to 1,192 feet. This zone can be supplied from multiple locations, which are summarized as follows:

- Baseline feeder pipeline through the Meridian Turnout
- Well 17 supply, which first enters Reservoir 2-1, before being boosted into the Pressure Zone by the 2-1 Booster Station.

- Direct supply from Well 42
- Wells 15 and 30 supply, which first enters Aeration Tank 3A-1, before being boosted into the Pressure Zone by the 3A-1 Booster Station.
- PRVs from Zone 3A

This zone has three storage reservoirs for a total storage capacity of 10.2 MG.

#### 4.3.3 Zone 3A (SHGL = 1,369 feet)

Zone 3A is the northernmost zone in the District' southern system. It is generally bound by Merrill Avenue to the north and San Bernardino Avenue to the south, with Linden Avenue and Cactus Avenue serving as the western and eastern boundaries respectively.

Elevations served in this pressure zone range from approximately 1,030 feet to 1,205 feet. This zone can be supplied from multiple locations, which are summarized as follows:

- The Fluidized Bed Reactor (FBR) treatment plant, which treats groundwater from well 11 and the City of Rialto well 6,
- Baseline feeder pipeline through the Meridian Turnout

This zone has two storage reservoirs for a total storage capacity of 6.0 MG.

#### 4.3.4 Zone 4 (SHGL = 1,524 feet)

Zone 4 is the southernmost zone of the District's northern system. It is generally bound by Highland Avenue to the north and Baseline Road to the south, with Cactus Avenue and the Southern Pacific Railroad serving as the western and eastern boundaries respectively

Elevations served in this pressure zone range from approximately 1,254 feet to 1,424 feet. This zone is currently supplied by pump station 4-1 and pump station 4-2 as well as PRVs from Zone 5. This zone has three storage reservoirs for a total storage capacity of 11.0 MG, which includes pumping storage for Zones 5, 6, 7, and 8.

#### 4.3.5 Zone 5 (SHGL = 1,662 feet)

Zone 5 is located within the District's northern system and generally bound by Summit Avenue to the north and Highland Avenue in the south. Maple Avenue and Linden Avenue serve as the western boundary while the Lytle Creek wash serves as the eastern boundary.

Elevations served in this pressure zone range from approximately 1,392 feet to 1,552 feet. This zone is supplied by the Roemer WFF as well as booster stations 5-1 and 5-2, which draw water from Zone 4. This zone has three storage reservoirs for a total storage capacity of 13.0 MG, which includes pump storage for Zones 6, 7, and 8.

#### 4.3.6 Zone 6 (SHGL = 1,884 feet)

Zone 6, located within the District's northern system, is generally bound by Duncan Canyon Road and Casa Grande Drive to the north and Highland Avenue to the south; Sierra Avenue and Brookside Avenue generally serve as the western boundaries while the Lytle Creek wash serves as the eastern boundary.

Elevations served in this pressure zone range from approximately 1,522 feet to 1,784 feet. This zone is supplied from booster stations 6-1 and 6-2, which draw water from Zone 5, as well as PRVs from Zone 6; this zone has 3 active storage reservoirs for a total storage capacity of 11.0 MG, which includes pumping storage for Zones 7 and 8.

Zone 6 includes two subzones: Zone 6A, and Zone 6B. Zone 6A includes the developed area bound to the north by Summit Avenue and Lowell Street, Locust Avenue to the east, Foothill Freeway to the south and Sierra Avenue to the west. Zone 6B is bound to the north and west by Zone 6A, with Maple Avenue and Highland Avenue generally serving as the eastern and southern boundaries respectively.

#### 4.3.7 Zone 7 (SHGL = 2,143 feet)

Zone 7, located within the District's northern system, is bounded to the south by pressure zone 6, and bounded north by the San Bernardino National Forest, then along the Interstate 15 to Glen Helen Regional Park. Elevations served in this pressure zone range from approximately 1,780 feet to 2,045 feet. This zone is supplied from booster station 7-1, which draws water from Zone 6, as well as PRVs from Zone 8; this zone has 4 storage reservoirs for a total storage capacity of 9.2 MG, which includes pumping storage for Zone 8.

Pressure Zone 7 includes two subzones: Zone 7A, and Zone 7B. Zone 7A serves the residential development along Sycamore Creek Loop. Zone 7B is generally south of Terra Vista Drive, between Riverside Avenue and Citrus Avenue.

#### 4.3.8 Zone 8 (SHGL = 2,369 feet)

Pressure Zone 8 is the northernmost zone in the District's northern system and is generally north of Glen Helen Parkway, with Sierra Avenue and Clearwater Parkway serving as the western and eastern boundaries respectively.

Elevations in this pressure zone range from approximately 2,040 feet to 2,267 feet. This zone is supplied from booster stations 8-1 and 8-2, which draw water from Zone 7; this zone has two storage reservoirs for a total storage capacity of 0.51 MG.

#### 4.4 TRANSMISSION AND DISTRIBUTION PIPELINES

Supply is pumped directly into the District's distribution system via 375 miles of pipeline, with diameters ranging from pipelines less than 6-inches in diameter to 48-inch pipelines. The District

maintains a robust transmission system, with approximately 60 miles of pipeline greater than or equal to 18-inches in diameter. The existing system pipelines are documented on Figure 4.2, and color-coded by pipe size. Similarly, Figure 4.3 documents the existing system, and color-coded by pressure zone serviced.

An inventory of existing pipes, extracted from the GIS-based hydraulic model and used in this analysis, is included in Table 4.2. For each pipe diameter, the inventory lists the length in feet, as well as the total length in units of miles. Additionally, standard pipe roughness coefficients used for various materials are included for reference on Table 4.3.

#### 4.5 STORAGE RESERVOIR

Storage reservoirs are typically incorporated in the water system to provide water supply for operation during periods of high demand, for meeting fire flow requirements, and for other emergencies, as defined in the District's planning criteria.

The District's existing storage reservoirs are summarized on Table 4.4, along with their capacity, high water level, tank height, and construction type. These reservoirs are also shown on the hydraulic profile schematic (Figure 4.4), the high water level and bottom tank elevations. The District maintains a robust system storage capacity, in excess of 71 million gallons.

#### 4.6 **BOOSTER STATIONS**

Water is conveyed from the lower pressure zones to the higher pressure zones via a series of booster pump stations (Table 4.5). Water is extracted from various sources, including surface water from Lytle Creek and purchased State Water Project water treated at the Oliver P. Roemer Water Filtration Facility, the Bunker Hill Basin water delivered through the Baseline Feeder, and groundwater wells. This water is then boosted throughout the water system by an interconnected transmission network.

Table 4.5 lists the location, design capacity, and individual pump information at each pump station. Operational controls for the booster pumps are controlled to turn "on" or "off" depending on their assigned storage reservoirs, as listed in this table.

#### 4.7 PRESSURE REDUCING VALVES

There are several sub-pressure zones that are pressure reducing valve (PRV) dependent within the existing system. Other PRVs act as emergency connections between pressure zones in case of a catastrophic failure. An inventory of the PRVs, their size, location, pressure zone serviced and settings are included on Table 4.6.

4-12

Table 4.2 Existing Modeled Pipe Inventory

**PRELIMINARY** 

10/3/2017 388.9 127.0 11.1 62.4 24.1 77.0 24.9 11.2 13.0 26.7 3. 8. 9.6 0.7 0.2 0.5 0.0 0.5 Total 2,053,440 406,370 140,956 329,700 670,350 127,390 131,383 20,021 58,489 58,981 68,684 50,951 3,948 2,709 2,685 96/ 47 Unknown 59,547 10,721 1,543 25,357 4,315 1,732 5,421 5,953 3,393 286 117 464 154 287 43 0 47 HDPE 279 279 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Pipe Length By Material 444,064 300,829 104,318 26,841 9,752 2,163 81 0 16 13 0 20 31 0 0 0 0 **Ductile Iron** 50,629 10,040 19,812 16,787 1,059 1,858 136 453 431 23 0 0 30 0 0 0 0 Cast Iron 22,991 6,186 8,011 9/0/9 2,174 167 255 122 0 0 0 0 0 0 0 0 0 **Asbestos Cement** 727,534 155,210 293,451 115,728 12,833 79,143 11,983 12,562 24,214 14,545 7,864 0 0 0 0 0 0 **Existing Distribution System** 748,396 19,735 133,232 160,537 93,109 46,114 94,076 33,615 3,186 33,969 57,416 36,799 2,709 50,480 2,568 Steel **Baseline Feeder Pipeline** 286 0 A K E ENGINEERING GROUP, Note: Diameter Total Total 36 10 12 14 16 18 20 30 22 24 7 3 4 9  $\infty$ 

1. Pipeline length and material based on GIS data provided by District Staff, as included in the 2017 Water System Hydraulic Model.

Table 4.3 Pipe Roughness Coefficients

						<b>PRELIMINARY</b>
			Age (1	Age (years)		
гіре масепаі	0	10	20	30	40	50
Asbestos Cement	125	125	125	125	125	125
Cast Iron	120	110	100	06	85	80
Ductile Iron	130	125	120	115	110	105
Plastic (PVC)	145	145	140	140	135	135
Steel	130	120	110	100	06	80
Note:						9/29/2017

1. At age=0, the roughness coefficients are commonly used values for new pipes. Roughness coefficients decrease with age at a rate that depends on pipe material.

2. Pipes with an unknown material or age were assigned a roughness coefficient of 110.

**Table 4.4 Existing Storage Facilities** 

**PRELIMINARY** 

Designation	Capacity	High Water Level	Tank Height	Type of Construction
	(MG)	(ft)	(ft)	
Zone 2				
R2-1	2.00	1,190	29.0	Reinforced Concrete
R2-2 (Inactive)	0.50	1,192	30.0 <sup>2</sup>	Welded Steel
R2-3	4.00	1,191	31.0	Welded Steel
R2-4	5.00	1,191	31.0	Welded Steel
Subtotal (Active Facilities)	11.00			
Zone 3A				
R3A-1	2.00	1,369	18.0	Reinforced Concrete
R3A-2	4.00	1,369	23.0	Welded Steel
Subtotal	6.00			
Zone 3				
R3-1	4.00	1,293	33.0	Welded Steel
R3-2	1.20	1,305	32.0	Welded Steel
R3-3	5.00	1,292	31.0	Welded Steel
Subtotal	10.20			
Zone 4				
R4-1	2.00	1,524	24.0	Reinforced Concrete
R4-2	2.00	1,524	19.0	Reinforced Concrete
R4-3	7.00	1,524	24.0	Welded Steel
Subtotal	11.00			
Zone 5				
R5-1	3.00	1,662	24.0	Reinforced Concrete
R5-2	4.00	1,662	23.5	Welded Steel
R5-3	6.00	1,662	24.0	Reinforced Concrete
Subtotal	13.00			

**Table 4.4 Existing Storage Facilities** 

**PRELIMINARY** 

Designation	Capacity	High Water Level	Tank Height	Type of Construction
	(MG)	(ft)	(ft)	
Zone 6				
R6-1 (Inactive)	0.25	1,885	24.0	Welded Steel
R6-2	1.00	1,884	24.0	Welded Steel
R6-3	4.00	1,884	31.0	Welded Steel
R6-4	6.00	1,884	31.0	Welded Steel
Subtotal (Active Facilities)	11.00			
Zone 7				
R7-1	0.15	2,143	23.5	Welded Steel
R7-2	2.00	2,143	23.0	Welded Steel
R7-3	4.00	2,143	23.5	Welded Steel
R7-4	3.00	2,143	23.5	Welded Steel
Subtotal	9.15			
Zone 8				
R8-1	0.10	2,369	24.0	Welded Steel
R8-2	0.41	2,363	18.0	Welded Steel
Subtotal	0.51			
Total Storage Cap	acity			
AKEI	71.86			
ENGINEERING GROUP, INC.				5/19/2017

Note

<sup>1.</sup> Unless noted otherwise, storage facility information extracted from West Valley Water District 2012 Water System Master Plan

<sup>2.</sup> Source: Tank information received from district staff October 30, 2017.

**Table 4.5 Existing Booster Pump Stations** Water Facilities Master Plan West Valley Water District

										PRE	PRELIMINARY
		Source	Destination		ō	Operational Capacity <sup>2</sup>	,2	J	Operational Controls <sup>4</sup>	Controls <sup>4</sup>	
Designation No.	Location	Pressure Zone	Pressure Zone	Design Capacity¹	Total (mgd)	Hours or operation	Firm³	Low Demand On Of	mand	High Demand On Off	off Off (ft)
Zone 2 to Zone 3 Transfer PS	Zone 2-1 Reservoir	7	м	1,500 gpm (1 pump)	1.4	16.0	0.0	19.0	21.0	23.0	25.0
FBR Treatment Facility		٠	3A	2,000 gpm	2.9	24.0	2.9				
Zone 3A-1 PS <sup>3</sup>	2015 9th St	3, 3A	3, 3A	3,500 gpm @ 210' (2 pumps, Z3A) 3,400 gpm @ 150' (2 pumps, Z3)	20.0	16.0	16.6	18.0	20.0	22.0	24.0
Zone 4-1 PS	6871 Martin Rd	ε	4	2,000 gpm @ 240' (2 pumps) 1,100 gpm @ 240' (1 pump)	4.9	16.0	3.0	10.0	12.0	13.0	15.0
Zone 4-2 PS	855 Baseline Rd	3A	4	2,400 gpm @ 170' (3 pumps)	6:9	16.0	4.6	7.0	9.0	9.0	11.0
Zone 4 Transfer PS	Zone 4-3 Reservoir	4	4	5,000 gpm (1 pump)		As Needed					
Zone 5-1 PS <sup>4</sup>	5700 Riverside Ave	4	S	3,000 gpm @ 170' (4 pumps)	11.5	16.0	8.6	0.6	11.0	13.0	15.0
Zone 5-2 PS	At Reservoir R4-3	4	2	3,200 gpm @ 181' (6 pumps)	18.4	16.0	15.4	10.0	12.0	14.0	16.0
Oliver P. Roemer WFF Effluent Pumps	3010 Cedar Ave		Ŋ	1,800 gpm @ 130' (4 pumps)	10.4	24.0	7.8				
Zone 6-1 PS <sup>4</sup>	5210 Riverside Ave	rv	9	2,200 gpm @ 230' (3 pumps) 1,850 gpm @ 235' (1 pump) 850 gpm @ 220' (1 pump)	6.8	16.0	8.9	14.0	16.0	24.0	26.0
Zone 6-2 PS	5210 Riverside Ave	S	9	2,590 gpm @ 265' (6 pumps)	14.9	16.0	12.4	15.0	17.0	25.0	27.0
Zone 7-1 PS	4334 Riverside Ave	9	7	2,200 gpm @ 280' (3 pumps) 1,300 gpm @ 280' (1 pump)	7.6	16.0	5.5	16.0	18.0	20.0	22.0
Zone 8-1 PS	3434 Lytle Creek Rd	7	∞	280 gpm @ 225' (1 pump) 175 gpm @ 225' (1 pump)		As Needed					
Zone 8-2 PS	3296 Lytle Creek Rd	7	8	1,630 gpm @ 252' (4 pumps)	6.3	16.0	4.7	10.0	16.5	10.0	16.5
AKEL ENGINEERING GROUP, INC. Notes:										0,	9/25/2017

Source: West Valley Water District 2012 Water Master Plan
 Excluding the Roemer WFF and FBR Treatment plant, production capacity assumes operating time of 16 hours per day.
 Firm capacity defined as total pump capacity excluding largest pump.

**Table 4.6 Existing Pressure Reducing Valves** 

PRELIMINARY

Valve ID	Location	Size	Pressu	ire Zone	Set	tings
			Upstream	Downstream	Upstream	Downstrea
Zone 8						
V2	8-2 Pump Station	10	8	7	111	N/A
Zone 7						
V8	Riverside (By Zone 7-1 PS)	12	7	7B	120	80
V9	Live Oak & Via Bello	8	7	7B	-	-
V10	Dove Tree & Terra Vista	8	7	7B	-	-
V11	North Sierra, across from school	8	7	7B	Not in Use	
V12	Terra Vista & Tamarind	8	7	6	95	60
V13	Goldenrod & Sunrise	8	7	6	-	-
V14	Citrus 1/4 mile south of Duncan Canyon	8	7	6	-	-
V15	Six M Ranch Ln & Cloudcrest Way	8	7	6	Not in Use	
V16	Duncan Canyon & Coyote Canyon South side	8	7	6	190	80
V17	Sweet bay and Sycamore Creek	8	7	7A	140	73
V18	Kimberlite & Sycamore Creek	8	7	7A	140	80
V19	Black Cottonwood & Sycamore Creek	8	7	7A	140	92
V20	Eve Primrose Ln & Sycamore Creek	8	7	7A	140	80
Zone 6						
V23	South Sierra, Sierra & Summit	8	6	6A	-	-
V24	End of Alder (by Target warehouse)	12	6	6A	105	75
V25	Locust (by fireworks factory)	12	6	6A	115	75
V26	Maple (top near bend)	8	6	6A	114	70
V27	Linden South of Riverside	8	6	6A	-	-
V28	Riverside and Cedar	6	6	6A	140	75
V29	Locust and Bohnert	8	6A	6B	112	82
V30	Maple and Banyon	6	6A	6B	120	70
Zone 5						
V35	Riverside and Cactus	8	5	4	-	-
Zone 3						
V44	San Bernardino and Linden	16	3A	3	-	-
V45	San Bernardino and Linden	12	3A	3	-	-
V46	San Bernardino and Cedar	12	3A	3	-	-
V47	Slover near Willow	12	3	2	-	-
V48	Lilac below Slover	8	3	2	-	-
V49	Larch and Buckskin	8	3	2	-	-
V50	Santa Ana and Linden	10	3	2	-	-
V51	Locust and Jurupa	12	3	2	-	-

1. Source: Control valve inventory received from District staff August 3, 2017.

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# CHAPTER 5 – WATER DEMANDS AND SUPPLY CHARACTERISTICS

This chapter summarizes existing domestic water demands, discussed available supply characteristics, and projects the future domestic water demands.

#### 5.1 EXISTING DOMESTIC WATER DEMANDS

The existing water demands used for this master plan were based on the District's 2016 water billing consumption records as well as total annual production. The existing water demands in this analysis are adjusted to match the annual production records and account for system losses.

The existing demand distribution, by pressure zone, was obtained from the water billing records. Using GIS, each customer account was geocoded to its physical location within its existing pressure zone. The accounts were then sorted by pressure zone and the total demand in each zone was calculated.

The District's existing average day domestic water demands, as extracted from the water billing records, were lower than the total demands listed in the annual production records due to system losses that occurred between the groundwater wells and customer service connections. In 2016 this water loss volume was approximately 6% of the total water produced by the District. For evaluation purposes the total domestic water demands were adjusted to reflect the 2014 production volume less 10%. This adjustment accounts for continuing changes in customer water use in response to State-mandated drought measures. The existing domestic water demands used in the evaluation, for each pressure zone, are summarized by pressure zone on Table 5.1.

#### 5.2 FUTURE DOMESTIC WATER DEMANDS

Future demands were projected using the unit factors for residential and non-residential land uses and included the developments within the District service area. Table 5.2 organizes the future land use categories and their corresponding domestic water demands. It should be noted that the existing domestic water demands in Table 5.2 were calculated using the recommended water unit factors, which take into account future water conservation practices, and are intended to represent the water use practices of customers at the buildout of the master plan horizon. The total average day domestic water demands from existing and future developments is calculated at 31.6 mgd.

These demands were used in sizing the future infrastructure facilities, including distribution mains, storage reservoirs, and booster stations. Demands were also used for allocating and reserving capacities in the existing or proposed facilities. **Table 5.1** summarizes the buildout water demand for each pressure zone.

**Demands by Pressure Zone** Table 5.1

**PRELIMINARY** 

**Total Peak Day** Demands<sup>4</sup> 32.8 11.3 11.6 11.1 53.7 1.9 3.9 4.8 1.5 7.7 Increase from Existing 114% 165% 276% 72% 72% **93%** 16% 43% 97% 82% % **Buildout**<sup>3</sup> Total Average **Day Demands** 31.6 12.3 19.3 4.6 9.9 2.8 8.9 6.5 6.0 1.1 2.3 **Demands by Pressure Zone New Demand** 10.1 (mgd) 2.5 2.5 5.8 1.8 0.0 0.3 0.2 2.4 0.4 Subtotal 21.5 (mgd) 13.5 2.8 8.0 2.0 5.6 4.4 4.0 0.5 4.1 1.1 5-Year Growth<sup>2</sup> **New Demand** (mgd) 1.6 0.3 0.0 4.2 0.1 0.0 0.7 1.2 0.3 3.7 South System Pressure Zones **North System Pressure Zones** Existing<sup>1</sup> 17.4 3.9 1.0 7.6 2.0 2.5 9.8 2.0 3.2 0.2 **System-Wide Demands** A K E L ENGINEERING GROUP, INC **Pressure Zone** Subtotal Subtotal 3A ന \_  $\infty$ 4 Ŋ 9

9/13/2019

1. Average day demands based on 2014 production less 10%, where the demand distribution by pressure zone is based on 2016 water billing records

2. Demands due to 5-Year growth based on development information provided by District Staff.

Notes:

3. Future demands based on additional growth due to buildout of General Plan Land Use.

4. Peak Day Demand = 1.7 x Average Day Demand

5. The demands shown in this table include system losses.

Table 5.2 Buildout Average Daily Water Demands
Water Facilities Master Plan
West Valley Water District

						Buildout Wa	Buildout Water Demands					
Land Use	ш	Existing Development	ıt			Future D	evelopment to bo	e Serviced withir	Future Development to be Serviced within Planned Area Boundary	undary		
Classifications		Within Service Area	6	5	Within Service Area	8		Sphere of Influence	Influence		OT To	Total
	Existing Development	Water Unit Factor	Average Daily Demand	New Development	Future Water Unit Factor	Average Daily Demand	Existing Development	New Development	Future Water Unit Factor	Average Daily Demand	Total Development within SOI	Average Daily Demand
Residential	(190 901)	(2) (2) (2) (2) (3) (3) (4) (4) (4) (4) (4) (4) (4) (4) (4) (4	(p.dg)	(liet dele)	(8) of 11cc act c)	(p./9)	(160 8016)	(100 2016)	(Sporting age)	(2013)	(וופר פרוב)	(pd9)
Residential 2	1,074	066	1,063,695	921	066	912,078	0	9	066	5,842	2,002	1,981,614
Residential 6	3,614	2,650	9,577,035	2,136	2,650	5,660,863	0	ī	2,650	14,234	5,756	15,252,132
Residential 12	0	4,580	0	556	4,580	2,544,483	0	27	4,580	124,527	583	2,669,010
Residential 21	83	5,630	468,282	545	5,630	3,069,456	0	57	5,630	319,248	685	3,856,986
Subtotal Residential	4,772		11,109,011	4,158		12,186,880	0	95		463,851	9,025	23,759,741
Non-Residential												
Commercial	28	1,800	105,083	927	1,800	1,668,923	0	18	1,800	32,621	1,004	1,806,627
Retail	4	1,890	7,317	180	1,890	339,845	0	0	1,890	0	184	347,162
Office	6	1,410	12,207	55	1,410	77,652	0	0	1,410	0	64	89,859
Educational	299	1,790	534,407	84	1,790	149,565	0	0	1,790	0	382	683,972
Institutional	8	1,410	10,866	475	1,410	669,137	0	0	1,410	0	482	680,003
Public Facility	53	250	13,324	131	250	32,761	0	0	250	0	184	46,085
Light Industrial	324	200	161,978	422	200	210,874	0	0	200	0	746	372,852
Heavy Industrial	162	1,530	248,184	480	1,530	735,142	0	0	1,530	0	643	983,325
Industrial	1,161	1,000	1,160,728	1,072	1,000	1,071,836	0	0	1,000	0	2,233	2,232,564
Subtotal Non-Residential	2,077		2,254,094	3,825		4,955,735	0	18		32,621	5,921	7,242,450
Other												
Utilities	223	10	2,230	362	10	3,618	0	0	10	0	285	5,849
ROW	35	0	0	75	0	0	0	0	0	0	110	0
Landscape Irrigation	77	2,690	207,367	124	2,690	333,334	0	25	2,690	66,291	226	606,992
Open Space	0	0	0	2,015	0	0	0	195	0	0	2,210	0
Subtotal Other	335		209,598	2,576		336,952	0	219		66,291	3,130	612,841
Totals	7,184		13,572,703	10,559		17,479,567	0	333		562,763	18,076	31,615,032

#### 5.3 REGULATIONS IMPACTING DEMAND

The State of California recently enacted Senate Bill 606 and Assembly Bill 1668, which regulate water demands based on user categories and establish planning targets for indoor and outdoor water use. These laws establish a target of maximum indoor residential water use of 55 gpdc by the year 2025, and a target of 50 gpdc by 2030. The State Water Resources Control Board is also expected to provide guidance on the calculation of indoor and outdoor water use from commercial, industrial and institutional uses, and similar targets, which are expected by 2022. These regulations are likely to establish long term water use reductions, which will impact supply and infrastructure planning.

## 5.4 DIURNAL DEMAND PATTERNS

Water demands vary with the time of day and by account type according to the land use designation. These fluctuations were accounted for in the modeling effort and evaluation of the water distribution system. The diurnal demand patterns affect the water levels in storage reservoirs and amount of flow through distribution mains.

Using available SCADA data provided by District staff, unique diurnal curves were developed for the Pressure Zones 3, 3A, 4, 5, 6, and 7. These patterns were developed using a mass balance method for each pressure zone, using the pump station flow in, pump station flow out, and the change in storage volume to estimate the fluctuation in zone demand. As shown on **Figure 5.1** and **Figure 5.2**, the hourly demand multipliers by pressure zone range from a maximum of 2.3 in Pressure Zone 6 to a minimum of 0.3 in Pressure Zone 5. The diurnal patterns were confirmed during the calibration effort of the District's hydraulic model and corresponding SCADA information.

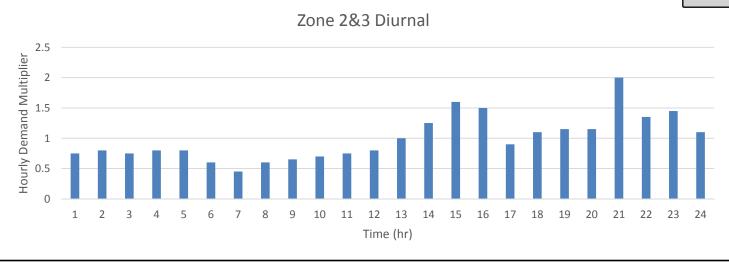
#### 5.5 WATER SUPPLY CHARACTERISTICS

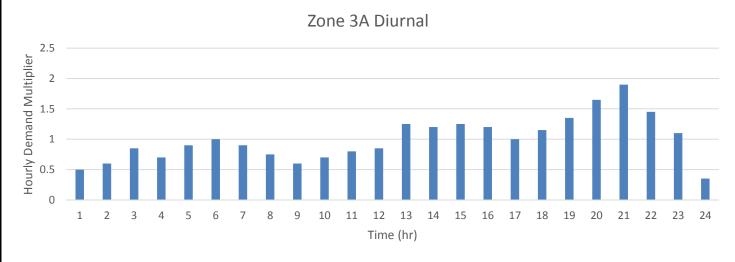
In order to meet the existing domestic water demands, the District utilizes several sources of supply, including groundwater and treated surface water. Some supply sources are subject to constraints that can impact the availability and reliability. The following sections summarize the supply sources and the related constraints, as well as documents the assumptions utilized in planning the supply-related improvements intended to meet future demands at the buildout.

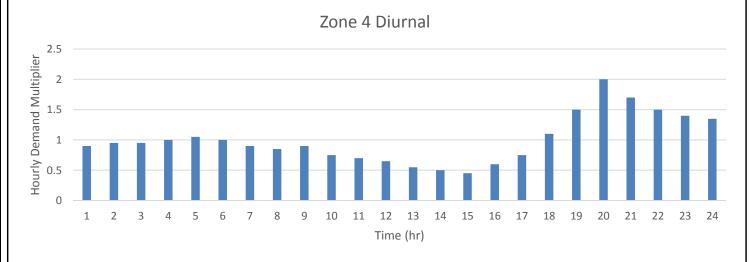
#### 5.5.1 Groundwater Supply Sources and Constraints

As discussed in a previous chapter, the District currently utilizes multiple wells to extract groundwater for delivery to existing water system customers. These groundwater wells extract water from five separate groundwater basins, which are shown graphically on Figure 5.3 and briefly summarized on the following pages.









Hourly Demand Multiplier

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February 5, 2018

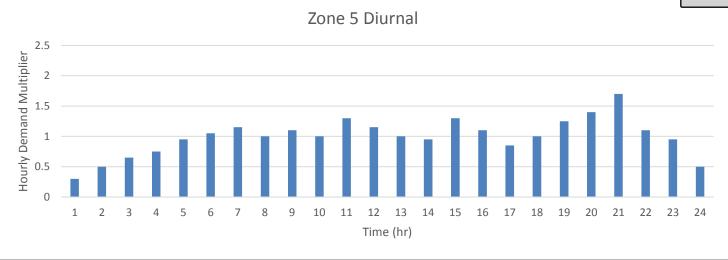
# Figure 5.1 Pressure Zone Demand Diurnals

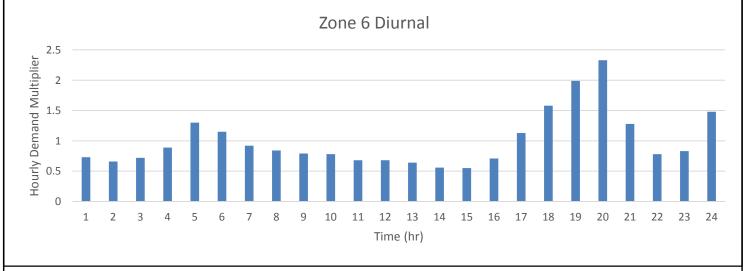
Water Facilities Master Plan West Valley Water District

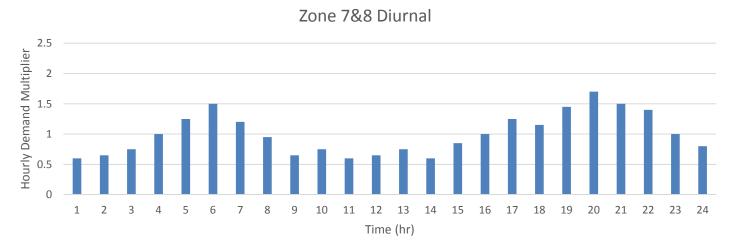












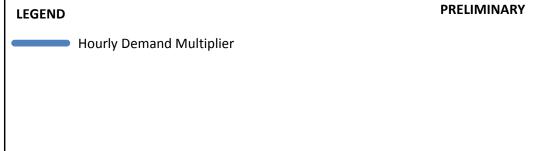
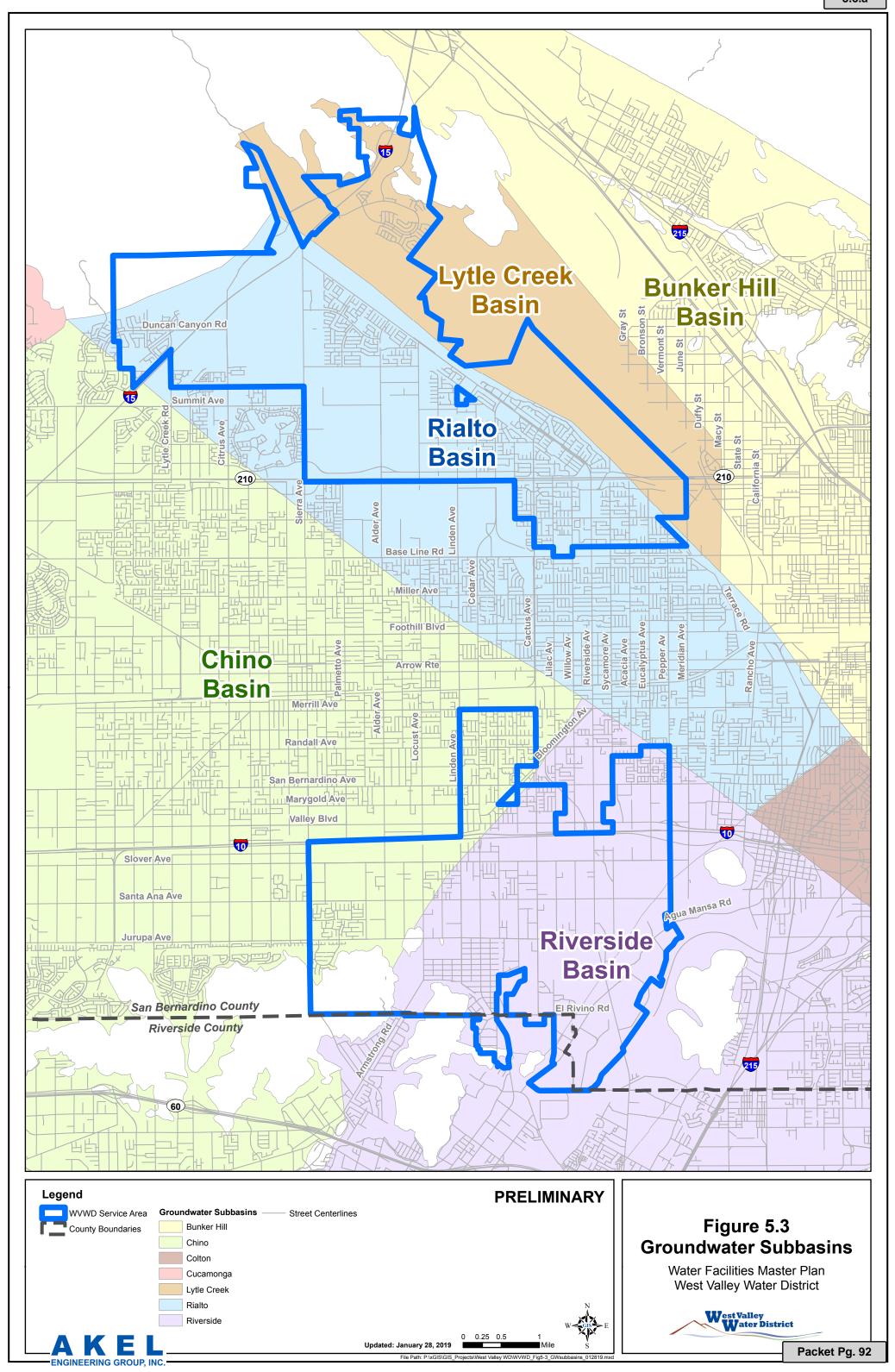


Figure 5.2
Pressure Zone
Demand Diurnals

Water Facilities Master Plan West Valley Water District



February 5, 2018



#### 5.5.1.1 Lytle Creek Basin

The Lytle Creek groundwater basin is a subbasin of the Bunker Hill groundwater basin, and underlies the northern extent of the District's North System. The subbasin is part of the Upper Santa Ana Valley Groundwater Basin and is generally adjoined to the west by the Rialto-Colton basin along the Lytle Creek fault and along the east and southeast by the remaining portions of the Bunker Hill basin. The San Gabriel Mountains form the northwestern border. It should be noted that DWR Bulletin 118 includes the Lytle Creek subbasin as part of the Bunker Hill basin and does not address it separately. However, the Santa Ana Region Basin Plan identifies this area as a separate management zone, and the District currently refers to it separately in discussions of groundwater quality and quantity from the remaining Bunker Hill basin.

The District's water rights in the Lytle Creek Basin are limited to 12,105 gallons per minute (gpm) if they are diverting their full allotment (2,290 gpm) of surface flow from Lytle Creek. If flows from the Creek are low and the District is receiving a portion of their allotment, they can pump the difference from the wells to a combined maximum of 14,395 gpm from the basin, depending on how much water is available to pump and how much water is available to divert from Lytle Creek. The District has no restrictions on how much is can pump and serve within the Lytle Creek Region.

The basin is an adjudicated groundwater basin and is managed by the Lytle Creek Water Conservation Association. The basin is highly porous and easily replenished during heavy precipitation years. Well production in the basin varies as the basin levels change from year to year.

The quality of groundwater in the Lytle Creek basin is characterized with arsenic contamination, in particular Well No 36 (not currently in use). Currently, only well W-2 has coagulation-based wellhead treatment to remove arsenic before its water is used for water supply.

#### 5.5.1.2 Bunker Hill Basin

The Bunker Hill groundwater basin adjoins the eastern boundary of the District's North System. The basin is part of the San Bernardino Basin Area and is generally adjoined to the west by the Lytle Creek basin and the Rialto-Colton basin

The extractions in the Bunker Hill basin are governed by the Western Judgement. The Western Judgment defined and adjudicated the San Bernardino Basin Area in 1969, and allocates percentages of the safe yield volume to the various agencies capable of extracting water from the basin.

The District has unrestricted water rights in the Bunker Hill basin, but has restrictions on pumping and exporting from certain areas of the basin as is defined in the 1924 Judgment for Lytle Creek Region and as defined in a City of San Bernardino Municipal Water Department's Basin Management Ordinance.

Plumes of various chemical pollutants have been detected in the Bunker Hill groundwater basin requiring installation of treatment to protect basin water quality. Currently, the District has two operational wells producing high quality water for water supply without any regulated contaminants requiring treatment. The Bunker Hill Basin is expected to be a reliable long-term water supply source able to make up shortfalls in water supply that could be caused by long-term droughts.

The District has two existing wells in the Bunker Hill Basin (Wells W-15 and W-30) within the defined area of the 1924 Judgment for the Lytle Creek Region.

In addition to the two existing wells, the District and the City of Rialto by agreement with the SBVMWD, have renewed a contract for a project to pump groundwater from the Bunker Hill Basin through a 48-inch diameter pipeline known as the BLF. The agreement requires that SBVMWD provide a supply up to 5,000 afy to the District (5.76 mgd).

The District owns one third of the BLF from Meridian Avenue to the Cactus Reservoir. This can provide up to 14,000 gpm of capacity in the pipeline. The additional capacity in the pipeline may be utilized for pumping water from the Bunker Hill Basin into the Baseline Reservoirs (R3A-1 and R3A-2). Additional agreements in the future may provide for more purchased water from SBVMWD or the City of San Bernardino or the District could drill additional wells to meet ultimate water demand.

#### 5.5.1.3 Rialto-Colton Basin

The Rialto-Colton basin underlies a majority of the District's North System. The basin is generally bounded to the northwest by the San Gabriel Mountains, the San Jacinto fault to the northeast, and the Rialto-Colton fault to the southwest, with the Santa Ana River traversing the southeastern portion of the basin.

Extractions in the Rialto-Colton basin are governed by the 1961 Rialto Basin Decree. Based on the groundwater elevations for three specific index wells verified between March and May of each year, the extraction entitlement for the District may be limited. Water levels in the Basin have declined in recent years, reducing the amount of groundwater extractions. Steps are being taken to formulate a long term strategy to manage the basin.

When the basin is not subject to restrictions by the adjudication, the District has unlimited extraction rights. During drought conditions, and when the adjudication is in effect, the extraction right ranges from 6,134 afy during drought periods to 3,067 afy in the most severe drought periods.

Since 2002, the Santa Ana River Water Quality Control Board (SARWQCB) has been conducting an investigation of groundwater contamination in the area of the City of Rialto. This site has also been designated as a Superfund site by the US EPA. Water quality of the Rialto Basin is characterized with elevated concentrations of perchlorate and nitrate, thus requiring treatment and

reducing its ability to be a reliable water supply. Currently installed wellhead treatment systems utilize ion exchange (IX) and fluidized bed reactor (FBR) treatment to mitigate perchlorate and nitrate contamination.

During years when the average elevation of the spring-high water levels in the three index wells is below 967.7 feet above mean sea level, the amount of water which the stipulated parties are entitled to pump from the Basin is reduced one percent (1%) for each foot. The average elevations of the spring-high water levels for the October 1, 2017 through the September 30, 2018 water year is 931.3 feet above mean sea level, or 38.4 feet below 969.7 feet mean sea level, thus reducing the District's extractions from the Basin by thirty-eight percent (38%).

#### 5.5.1.4 Chino Basin

The Chino basin underlies a portion of the District's South System. The basin is generally bounded to the east by the Rialto-Colton fault, the San Gabriel Mountains to the north, and the Jurupa Mountains and Puente Hills to the south. The Chino Basin consists of about 235 square miles of the Upper Santa Ana River Watershed, and is an alluvial valley that is mainly flat from east to west, and slopes from the north to the south at a one to two percent grade. This basin is among the largest groundwater basins in southern California, with about 5,000,000 acre-feet of water and an unused storage capacity of about 1,000,000 acre-feet.

The Chino basin is an adjudicated groundwater basin and is managed by the Chino Basin Watermaster, which manages the basin through the Chino Optimum Basin Management Plan. Without incurring replenishment costs, the District is entitled to approximately 1,000 afy of groundwater extraction from this subbasin. The District has two wells (W-37 and W-39) in the Chino Basin which can produce 1.4 mgd and 3.8 mgd, but are not currently in service due to high levels of perchlorate and nitrate. The District will have to install wellhead treatment on these wells to take advantage of their pumping ability and the District's rights in the basin.

#### 5.5.1.5 Riverside-Arlington Basin (North Riverside Groundwater Basin)

The Riverside-Arlington basin underlies a majority of the District's South System. The basin is generally bounded to the north by the Jurupa Mountains, to the northeast by the Rialto-Colton fault, and the Box Springs Mountains and Arlington Mountain to the south, with the Santa Ana River traversing the northern portion of the basin. This groundwater basin is a large alluvial fill basin that is bound by major faults and topographic barriers. Recharge to the basin occurs by the underflow from basins to the north, from the Santa Ana River, and from percolation of surface water runoff from the surrounding uplands.

The extractions in a portion of the North Riverside basin upstream of the Riverside Narrows are governed by the Western Judgement. However, there is no extraction limit for the District's wells in this basin. Water quality of the basin is characterized with elevated concentration of perchlorate and emerging increase of nitrate concentration. The currently installed wellhead treatment system utilize IX to remove perchlorate. The District has identified that some wells located in the basin

present possible contamination with Methyl tert-butyl ether (MTBE). Wells Number 40 and 41 are monitored monthly, however no MTBE has been detected in these wells or any other District wells.

#### 5.5.2 Surface Water Supply

The following sections document the District's existing sources of surface water supply, current water supply constraints, and existing surface water quality.

#### 5.5.2.1 Surface Water Supply Sources

As discussed in a previous chapter, the District currently treats two sources of surface water at the Roemer WFF for delivery to existing water system customers: State Water Project water and flow from Lytle Creek. These sources and the related reliability are briefly summarized in the following sections and shown on Table 5.3.

Creek surface water and has entered into an agreement with the City of San Bernardino to purchase the City of San Bernardino's 3.00 cfs (1,350 gpm) water rights for a total of 8.09 cfs (3,640 gpm or 5.2 mgd) of Lytle Creek surface water. The City of San Bernardino, due to infrastructure limitations, is unable to utilize its rights and divert water from the Creek. The District also has a court settlement agreement with Fontana Union Water Company for approximately one percent (1%) of Fontana Union Water Company's annual water production to be taken at the District's WFF. This is approximately 320 acre feet per year, or 200 gpm. The City of Rialto has 2.3 cfs water rights. The District, the City of Rialto, and the City of San Bernardino, have a combined capacity of 10.39 cfs (6.7 mgd) of Lytle Creek surface water rights.

In 1993, the District and the City of Rialto jointly constructed the Oliver P. Roemer WFF, a 7.2 mgd water treatment plant, in Pressure Zone 5, to treat 6.7 mgd of Lytle Creek surface water. The facility produced approximately 5.2 mgd annual average daily flow of supply to the District and approximately 1.5 mgd for the City of Rialto from Lytle Creek.

Lytle Creek surface water flows fluctuate seasonally and the District and City of Rialto's water right could be prorated whenever the Lytle Creek water flow is below 800 miner inches (16 cfs). When the Lytle Creek surface water flow drops below 16 cfs, the water right of both the District and the City of Rialto are subject to proration. In addition to the flow fluctuation, the turbidity of Lytle Creek surface water flow also varies seasonally.

State Water Project. The District currently imports SWP water from SBVMWD through the
Lytle Turnout off of the San Gabriel Feeder Pipeline. This SWP water is delivered to the
Roemer WFF and treated in addition to the Lytle Creek flows. Recently constructed

Table 5.3 Water Supply Portfolio

PRFI IMINARY

Springs	Maximum Water When	Imported			Historical Water Use <sup>2</sup>	Vater Use <sup>2</sup>			
	Available <sup>1</sup>	Water <sup>1</sup>	2011	2012	2013	2014	2015	2016	2017
	(AFY)	(AFY)	(AFY)	(AFY)	(AFY)	(AFY)	(AFY)	(AFY)	(AFY)
Surface Water									
Imported SWP <sup>3</sup>		No Limit	400	849	1,194	1,643	2,244	2,839	2,653
Lytle Creek	5,8704		4,203	4,700	3,110	2,363	2,271	2,026	4,540
Other Surface Water									
Groundwater Basins <sup>5</sup>									
Lytle Creek Basin	19,500 <sup>6</sup>		2,983	4,002	3,776	3,262	2,159	1,850	2,365
Bunker Hill Basin	No Restrictions		1,335	1,682	1,885	1,478	1,520	1,351	2,300
Chino Basin	10007		0	0	0	0	0	0	0
Rialto-Colton	No Restrictions <sup>8</sup>		4,883	4,093	4,005	3,916	2,505	2,123	3,923
Riverside-Arlington	No Restrictions		3,144	3,932	3,389	2,992	2,065	2,745	1,089
Total Groundwater Use			12,345	13,709	13,055	11,648	8,249	8,069	9,677
Other Water Sources									
Purchased GW through Baseline Feeder Pipeline		2,000	3,020	1,990	3,350	4,819	4,367	3,380	3,151
Total Historical Water Use	se								
Total A K F I			19,968	21,248	20,709	20,473	17,131	16,314	20,022

1. Source: WVWD 2012 Water System Master Plan.

2. Unless noted otherwise, historical water use extracted from Water System Statistics provided by WVWD Staff on September 25. 2018.

1/28/2019

- 3. Water imported from the SWP is purchased from San Bernardino Valley Municipal Water District.
- 4. The District has a 3,700 AFY water right to Lytle Creek and has entered into an agreement with the City of San Bernardino to purchase the City's 2,170 AFY water right for a total of 5,870 AFY water right to Lytle Creek
- 5. Historical water use by groundwater basin extracted from the following:
- Years 2011-2015: WVWD 2015 Urban Water Management Plan, Table 11-10. Year 2016: Basin data provided by WVWD staff on January 28, 2019.
- 6. During extended periods of drought well production in Lytle Creek Basin is projected to be reduced. However, there is no maximum amount of water that Year 2017: Groundwater basin production report provided by WVWD staff on September 24, 2018. that can be pumped and served within the Lytle Creek Basin region.
  - 7. The District's water rights are limited to approximately 1000 AFY without incurring replenishment costs.
- 8. When the basin adjudication is in effect the extractions rights range from 6,134 AFY to 3,067 AFY depending on the severity of the drought.

metering and transmission facilities will enable the District to import and treat up to 20 mgd upon the completion of the Roemer WFF capacity expansion. It should be noted that the SWP water is considered an interruptible water supply, and while historically reliable, the potential disruption of SWP water deliveries are accounted for when planning future water infrastructure facilities.

#### 5.5.3 Water Supply Planning

In order to meet the growing demand requirements of the District service area and provide additional water supply reliability, the existing water supply capacity will require expansion. This expansion will include the rehabilitation of existing groundwater wells, the construction of new groundwater wells, and the expansion of the Roemer WFF treatment plant, which are generally described in the following sections.

#### 5.5.3.1 Rehabilitate Existing Wells

The District currently has multiple groundwater wells that are inactive due to water quality constraints or other operational issues. The rehabilitation of these existing wells will increase the District's supply capacity and multiple sites have infrastructure in place to facilitate the delivery of water to the existing water distribution system. The rehabilitation of these existing wells is considered the first priority for planning water supply improvements, which is reflected in the supply capacity analysis and recommended improvements discussed in a later chapter.

#### 5.5.3.2 Construct New Wells

New groundwater wells are required to meet the expanded needs of the planning area boundary. The well locations shown in this WFMP are preliminary and are intended as placeholders for planning purposes. The location of future groundwater wells will be determined based on site feasibility studies completed as part of the design process. The general assumptions for the recommendation of new wells are documented as follows:

- Due to the availability of water supply in the Bunker Hill groundwater basin the
  development of future wells is recommended. However, as an alternative to constructing
  new groundwater wells the District could also enter into contract to receive deliveries of
  Bunker Hill water through the Baseline Feeder pipeline.
- As discussed in a previous section, Pressure Zone 2 receives a majority of its supply by PRV from Pressure Zone 3. To limit this supply dependency, new wells are recommended to meet the buildout development demand requirements within Pressure Zone 2.

#### 5.5.3.3 Roemer WFF Treatment Expansion

The Roemer WFF has a current treatment capacity of 14.4 mgd. The District has plans to expand the capacity by an additional 6.0 mgd, which will increase the total treatment capacity to 20.4 mgd. Based on the 4,000 afy (3.6 mgd) of projected Lytle Creek flows, it is estimated that

approximately 16.8 mgd total of SWP water could be purchased to utilize the full treatment capacity of the Roemer WFF.

#### 5.5.4 Surface Water Quality

Lytle Creek and State Water Project are the two sources of surface water currently used for the

District's surface water supply. Lytle Creek, which is a perennial stream in the upper watershed, is a local surface water that is treated for domestic water use. During the summer for short periods, Lytle Creek surface water flow will drop below 16 cfs, which causes the District's water rights to be subject to proration. Turbidity, microbiological contaminants and other surface water-typical constituents characterize the quality of the water from Lytle Creek.

The District has been utilizing water from the State Water Project since 1999. The current metering and transmission facilities allow the District to import 20 mgd (23,000 afy) of the SWP water. Quality of the SWP water is characterized with elevated concentration of total organic carbon (TOC). Traditionally, the District imports and treats the SWP water for potable water supply at the Roemer WFF.

#### 5.5.5 Other Water Sources

This section documents other sources of water supply, both existing and potential, that are available to the District. This section was completed by Kleinfelder.

#### 5.5.5.1 Baseline Feeder

The water supply of the Baseline feeder comes from SBVMWD-owned wells in the Bunker Hill Basin. The current agreement with SBVMWD allows the District to receive up to 5,000 afy of supply. The District could investigate additional supply through the BLF.

#### 5.5.5.2 Alternative Water Sources

No other water source is currently being utilized by the District. However, due to climate change and severe droughts, the District is considering the feasibility of developing alternative source of water supplies including but not limited to water banking, storm water run-off collection and recyclable water. Capacity and water quality of these alternative sources are not defined at this point in time. Further study of potential yields and treatment methodologies will need to be completed prior to implementing new water sources. Treatments may include removal of turbidity, oil, heavy metals, microbiological contaminants, and other regulated water quality constituents may be necessary. As opportunities arise and technology advances, it is recommended that the District continue to explore the possibility of expanding its water supply portfolio and developing new sources of water supply.

#### 5.5.6 Current and Future Regulations

The US EPA has set mandatory water quality standards in the National Primary Drinking Water Regulations (NPDWRs) for inorganics, organic chemicals, disinfectant and disinfection by-products, and microbiological contaminants. The US EPA recommends secondary non-enforceable National Secondary Drinking Water Standards (NSDWSs) for 15 contaminants that may cause aesthetic effects on potable water. The quality of the District's potable water is in full compliance with local, state and federal regulatory requirements.

The pending regulations that may be of importance for the District and its water supply system include:

- California DDW's recommendations to establish a lower perchlorate detection limit for purposes of reporting. If proved technically and economically feasible and beneficial to the public health, the current perchlorate MCL of 6 parts per billion (PBB) may be revised.
- The Lead and Copper Rule will be updated in 2018 to incorporate EPA changes and lessons learned from the water crisis in Flint, Michigan.
- Development of a new unregulated contaminant monitoring regulation. DDW is in the process of gathering information on the presence and concentration of contaminants of concern in potable water systems. If deemed necessary, the DDW may choose to regulate, or increase regulation, of some of these contaminants in the future.

Although not currently utilized by the District, the pending new regulation for water reuse, including recycled water and water for potable reuse, may be important for the District's future water

supply.

#### CHAPTER 6 - HYDRAULIC MODEL DEVELOPMENT

This chapter describes the development and calibration of the District's domestic water distribution system hydraulic model. The hydraulic model was used to evaluate the capacity adequacy of the existing system and to plan its expansion to service anticipated future growth.

#### 6.1 OVERVIEW

Hydraulic network analysis has become an effectively powerful tool in many aspects of water distribution planning, design, operation, management, emergency response planning, system reliability analysis, fire flow analysis, and water quality evaluations. The District's hydraulic model was used to evaluate the capacity adequacy of the existing system and to plan its expansion to service anticipated future growth.

#### 6.2 MODEL SELECTION

The District's hydraulic model combines information on the physical characteristics of the water system (pipelines, groundwater wells, and storage reservoir) and operational characteristics (how they operate). The hydraulic model then performs calculations and solves a series of equations to simulate flows in pipes and calculate pressures at nodes or junctions.

There are several network analysis software products that are released by different manufacturers, which can equally perform the hydraulic analysis satisfactorily. The selection of a particular software depends on user preferences, the distribution system's unique requirements, and the costs for purchasing and maintaining the software.

The District's previous model was developed using the Innovyze (formerly known as MWHSoft) H2ONet, which allows for steady-state and extended period simulations within an AutoCAD user interface. As part of this master plan, the hydraulic model was redeveloped into the GIS-based hydraulic model InfoWater by Innovyze. The model has an intuitive graphical interface and is directly integrated with ESRI's ArcGIS (GIS), providing a useful modeling tool linked to the newly developed District GIS.

# 6.3 HYDRAULIC MODEL DEVELOPMENT

Developing the hydraulic model included skeletonization, digitizing and quality control, developing pipe and node databases, and water demand allocation.

#### 6.3.1 Skeletonization

Skeletonizing the model refers to the process where pipes not essential to the hydraulic analysis of the system are stripped from the model. Skeletonizing the model is useful in creating a system that accurately reflects the hydraulics of the pipes within the system, while reducing complexities

of large systems, which will reduce the time of analysis while maintaining accuracy, but will also comply with limitations imposed by the computer program. For the purposes of this master plan, skeletonizing was kept to a minimum due to the integrity of the GIS.

#### 6.3.2 Pipes and Nodes

Computer modeling requires the compilation of large numerical databases that enable data input into the model. Detailed physical aspects, such as pipe size, pipe elevation, and pipe lengths, contribute to the accuracy of the model.

Pipes and nodes represent the physical aspect of the system within the model. A node is a computer representation of a place where demand may be allocated into the hydraulic system, while a pipe represents the distribution and transmission aspect of the water demand. In addition, reservoir dimensions and capacities, and groundwater well capacity and design head, were also included in the hydraulic model.

#### **6.3.3 Digitizing and Quality Control**

The District's existing domestic water distribution system was digitized in GIS using several sources of data and various levels of quality control. The data sources included the District's existing system as maintained by staff in GIS, as well as conversation with District staff and record drawings.

After reviewing the available data sources, the hydraulic model was updated and verified by District staff. Resolving discrepancies in data sources was accomplished by graphically identifying each discrepancy and submitting it to engineering and GIS staff for review and comments. District comments were incorporated in the verified model.

#### 6.3.4 Demand Allocation

Demand allocation consists of assigning water demand values to the appropriate nodes in the model. The goal is to distribute the demands throughout the model to best represent actual system response.

Allocating demands to nodes within the hydraulic model required multiple steps, incorporating the efficiency and capabilities of GIS and hydraulic modeling software. Existing land use water demand factors were used in conjunction with the existing land use map. Each demand factor was applied to the appropriate land use and then multiplied by the acreage. In the absence of complete water billing records, this methodology was considered the best approach for accurately allocating the existing water demands.

Domestic water demands from each anticipated future development, as presented in a previous chapter, were also allocated to the model for the purpose of sizing the required future facilities. The demands from the greater Planning Area were allocated based on proposed land use and the land use acreages. As many of the areas were very large in size, demands were allocated evenly

to the demand nodes within each area. Infill areas, redevelopment areas, and vacant lands were also included in the future demand allocation.

#### 6.4 MODEL CALIBRATION

Calibration is intended to instill a level of confidence in the pressures and flows that are simulated. Calibration generally consists of comparing model predictions to field measured results and making necessary adjustments.

#### 6.4.1 Calibration Plan and SCADA

The District relies on multiple sources of supply, including groundwater wells, treated water supply, and water deliveries through the Baseline Feeder. The District maintains SCADA at its tank sites, booster stations, and the Oliver P Roemer Water Filtration Facility. As such, this SCADA information was considered adequate for calibrating the hydraulic model. Figure 6.1 documents each point used in the calibration of the hydraulic model.

District staff provided hourly flow data for each well and booster station, as well as tank levels for each pressure zone for July 2017. This data was further consolidated and compared with daily demand data to best calibrate to peak day conditions.

#### 6.4.2 Steady State Calibration

As part of this master plan, a steady-state calibration was performed on the existing system. Steady-state model runs consist of "snapshot" model run where the system is evaluated for a single specified hour. Typically, steady-state model runs are calibrated to fire flow tests, where a static pressure and residual pressure are provided. The model is then simulated for that specific hour and fire flow, and a pressure comparison is completed. The modeled Hazen Williams C-Factor and connectivity are adjusted based on the calibration results.

The steady-state calibration results are documented on Table 6.1. The results generally indicate that the system is in good health. There are robust looped-pipe networks and transmission main connectivity within the existing system, which help to mitigate the negative effects of fire flows.

#### 6.4.3 EPS Calibration

The model was also calibrated for extended period simulation (EPS), which typically involved comparing the hydraulic model to field conditions over at least 24 hours. EPS calibration consists of comparing model predictions to diurnal operational changes in the water system. The intent of an extended period simulation

The calibration process was iterative and resulted in satisfactory comparisons between the field measurements and the hydraulic model predictions at each well site. It should be noted that some of the SCADA information at the well sites and the booster station sites were found to be

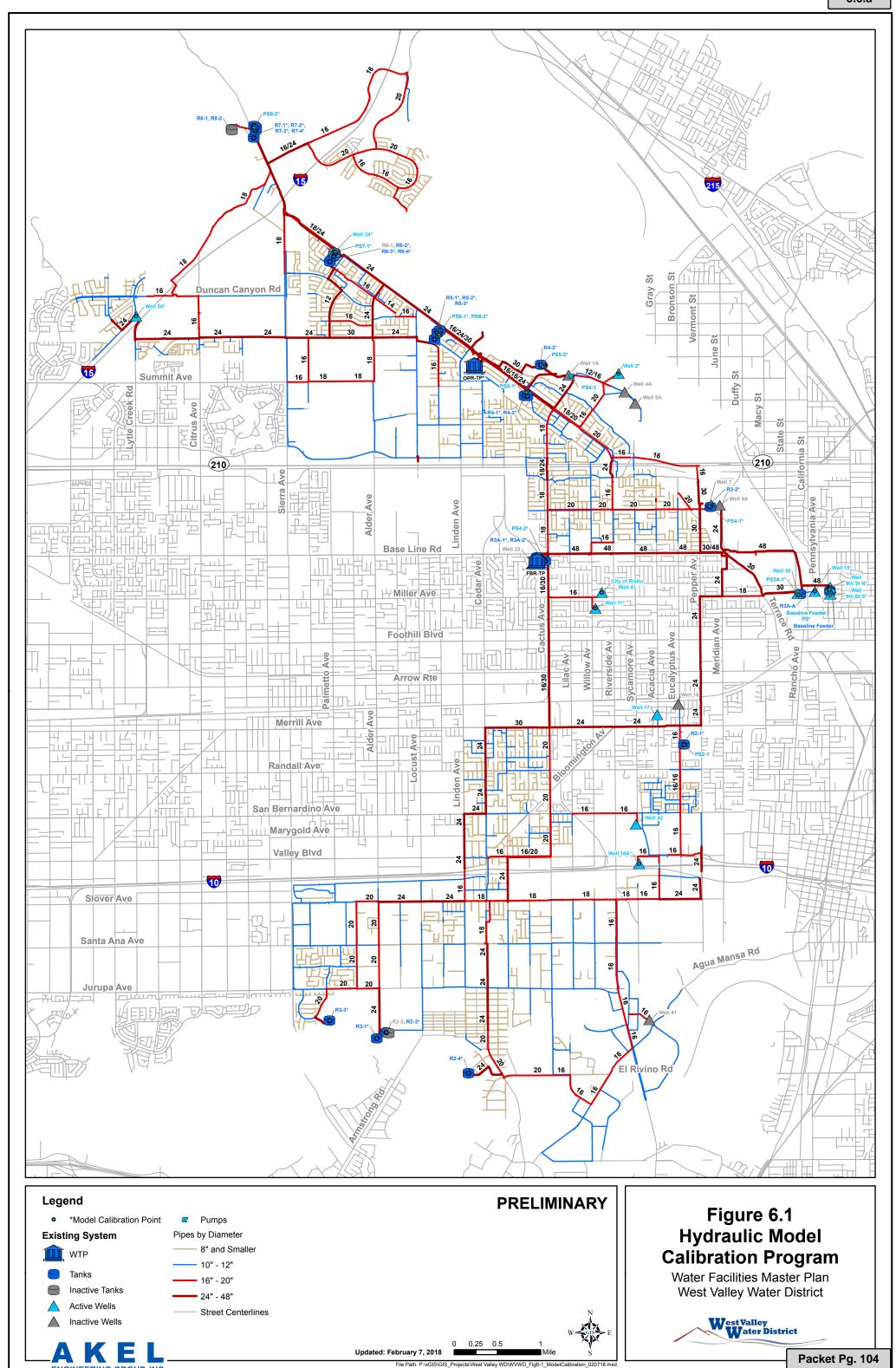


Table 6.1 Steady State Calibration Results

**PRELIMINARY** Difference 9/11/2017 **Percent** 10.0% -1.0% -7.4% 2.5% -3.8% 8.6% -1.2% 7.2% 6.3% Residual Pressure Simulated 109.63 65.44 65.32 82.48 80.35 79.05 70.35 77.17 78.64 (isd) Observed 107 89 99 75 74 80 72 74 9/ Difference Percent 1.3% -1.9% -1.3% 6.5% %0.9 1.5% -1.9% 11.5% -2.0% Static Pressure Simulated 114 (isd) 70 72 85 98 78 87 83 84 Observed 113 (isd) 85 75 71 73 82 80 85 80 2755 S Willow Avenue, Bloomington, CA **Address of Gauging Hydrant** 1571 N Sycamore Avenue, Rialto, CA 5891 N Sycamore Avenue, Rialto, CA 17132 Slover Avenue, Fontana, CA 2010 W Stonehurst Dr., Rialto, CA 654 S. Cactus Avenue, Rialto, CA 1350 Brown Ave., Riverside, CA 2092 Spruce Avenue, Rialto, CA 884 S Church Street, Rialto, CA Time 7/25/16 3/30/16 11/2/16 11/2/16 8/8/16 1/10/17 3/16/17 3/16/17 8/8/16 Date Pressure Zone 3**A** 3A 7 2 9 2 m Location Number 580 260 569 568 570 573 576 578

Notes:

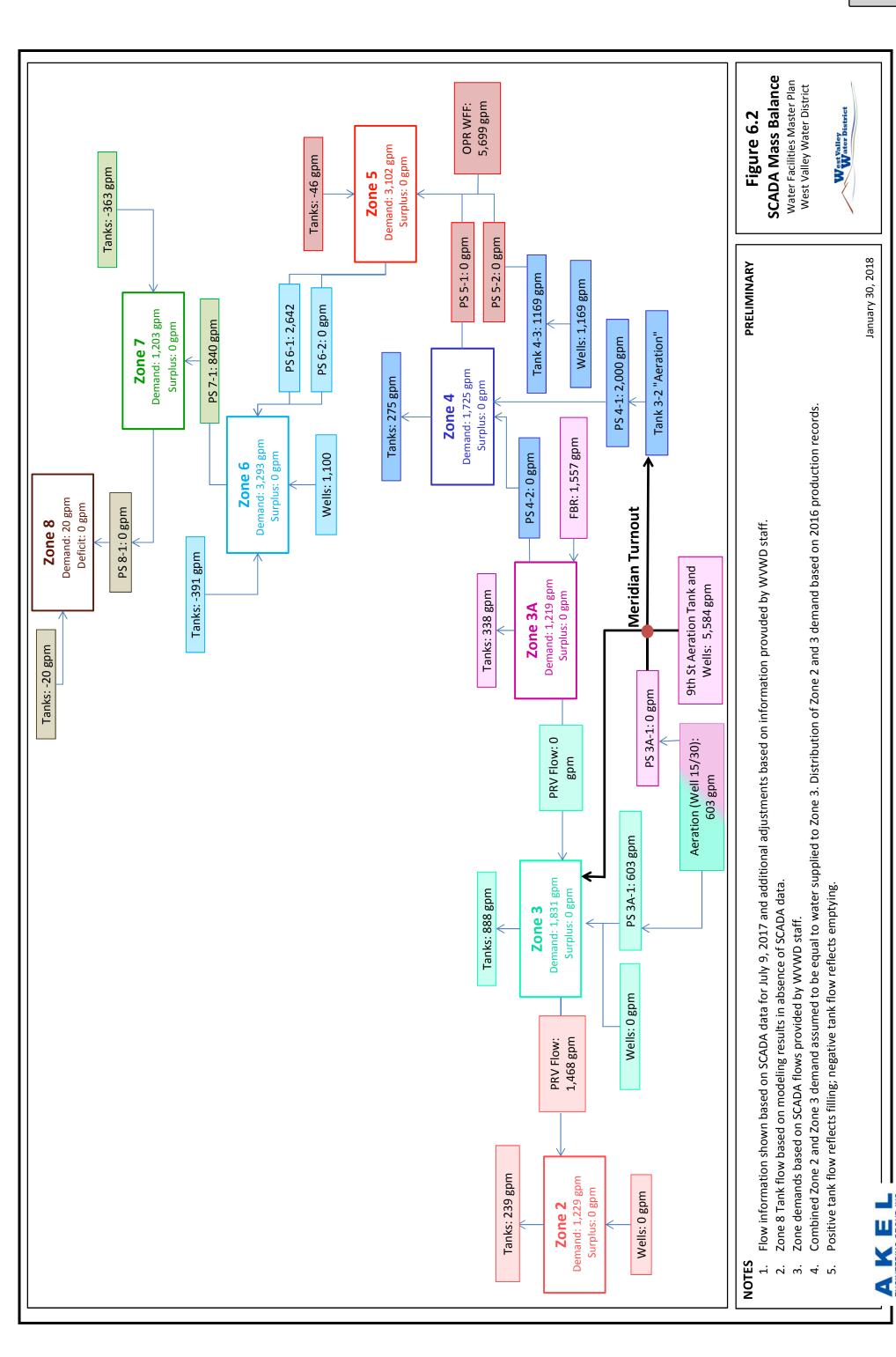
1. Fire flow locations and results based of historical fire flow tests received from District staff.

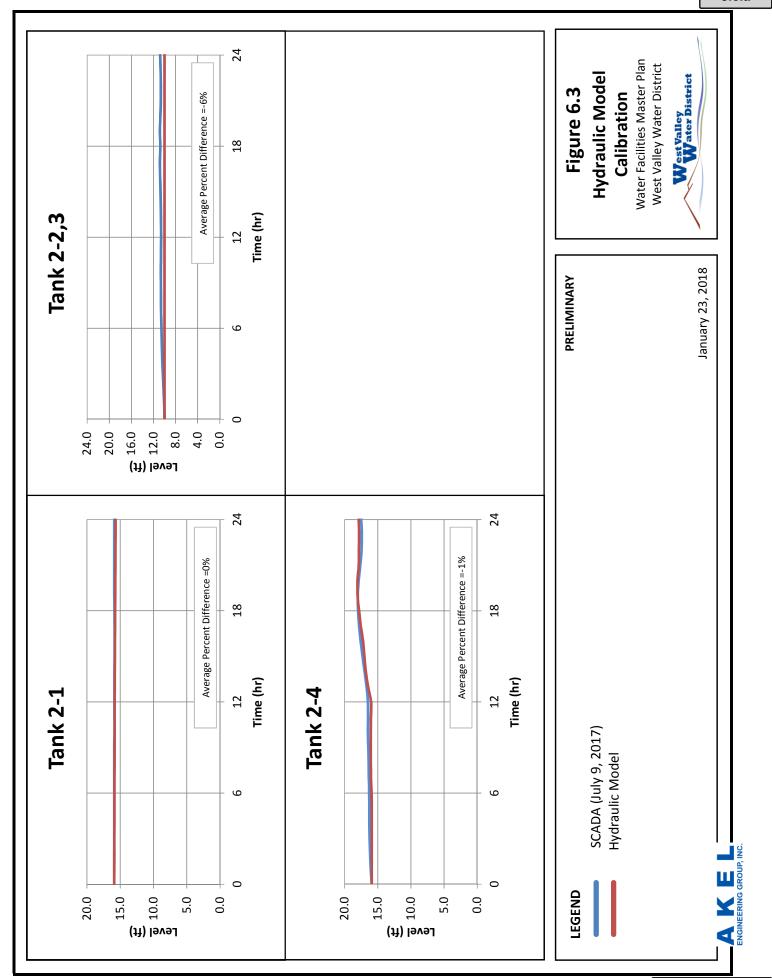
erroneous. As such, a mass balance of the existing water system by pressure zone was completed and submitted to District staff for review (Figure 6.2). Calibration information for the wells and the booster stations relied heavily on District staff knowledge of the system, and interpretation of trendlines observed in the SCADA. The calibration results were graphically summarized for each site and included in Appendix C.

Representative extracts from Appendix C are shown on Figure 6.3 for calibration points at the Zone 5, 6, and 7 storage reservoirs.

#### 6.4.4 Use of the Calibrated Model

The calibrated hydraulic model was used as an established benchmark in the capacity evaluation of the existing water distribution system. The model was also used to identify improvements necessary for mitigating existing system deficiencies and for accommodating future growth. This valuable investment will continue to prove its value to the District as future planning issues or other operational conditions surface. It is recommended that the model be maintained and updated with recent construction to preserve its integrity.





### **CHAPTER 7 - EVALUATION AND PROPOSED IMPROVEMENTS**

This section presents a summary of the domestic water system evaluation and identifies improvements needed to mitigate existing deficiencies, as well as improvements needed to expand the system and service growth.

### 7.1 OVERVIEW

The calibrated hydraulic model was used for evaluating the distribution system for capacity deficiencies during peak hour demand and during peak day demands in conjunction with fire flows. Since the hydraulic model was calibrated for extended period simulations, the analysis duration was established at 24 hours for analysis.

The criteria used for evaluating the capacity adequacy of the domestic water distribution system summarized in the System Performance and Design Criteria chapter.

### 7.2 FIRE FLOW ANALYSIS

The fire flow analysis consisted of using the peak day demand in the hydraulic model and applying hypothetical fire flows. The magnitude and duration of each fire flow was based on the governing land use type within proximity to the fire location. The criterion for fire flows was also summarized in the System Performance and Design Criteria chapter.

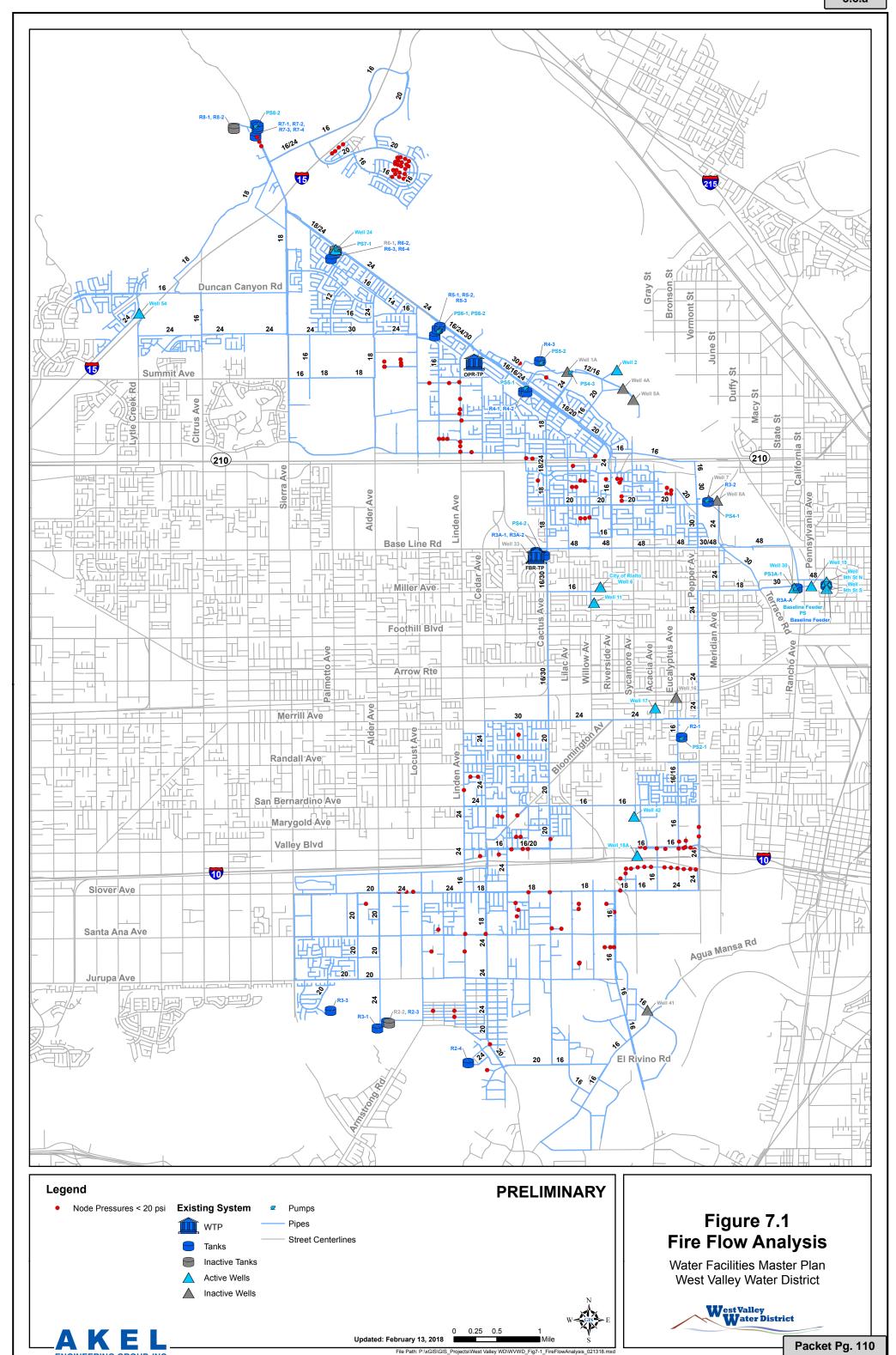
The hydraulic model indicates that the District's existing distribution system performed adequately during the fire flow analysis. Figure 7.1 documents the hydraulically simulated pressure deficiencies within the existing distribution system. As discussed in the system performance and design criteria chapter, pressures within the water main must be above 20 psi to provide adequate pressure for firefighting purposes. Figure 7.2 documents the fire flow availability based on the nearby infrastructure and hydraulically available head pressure.

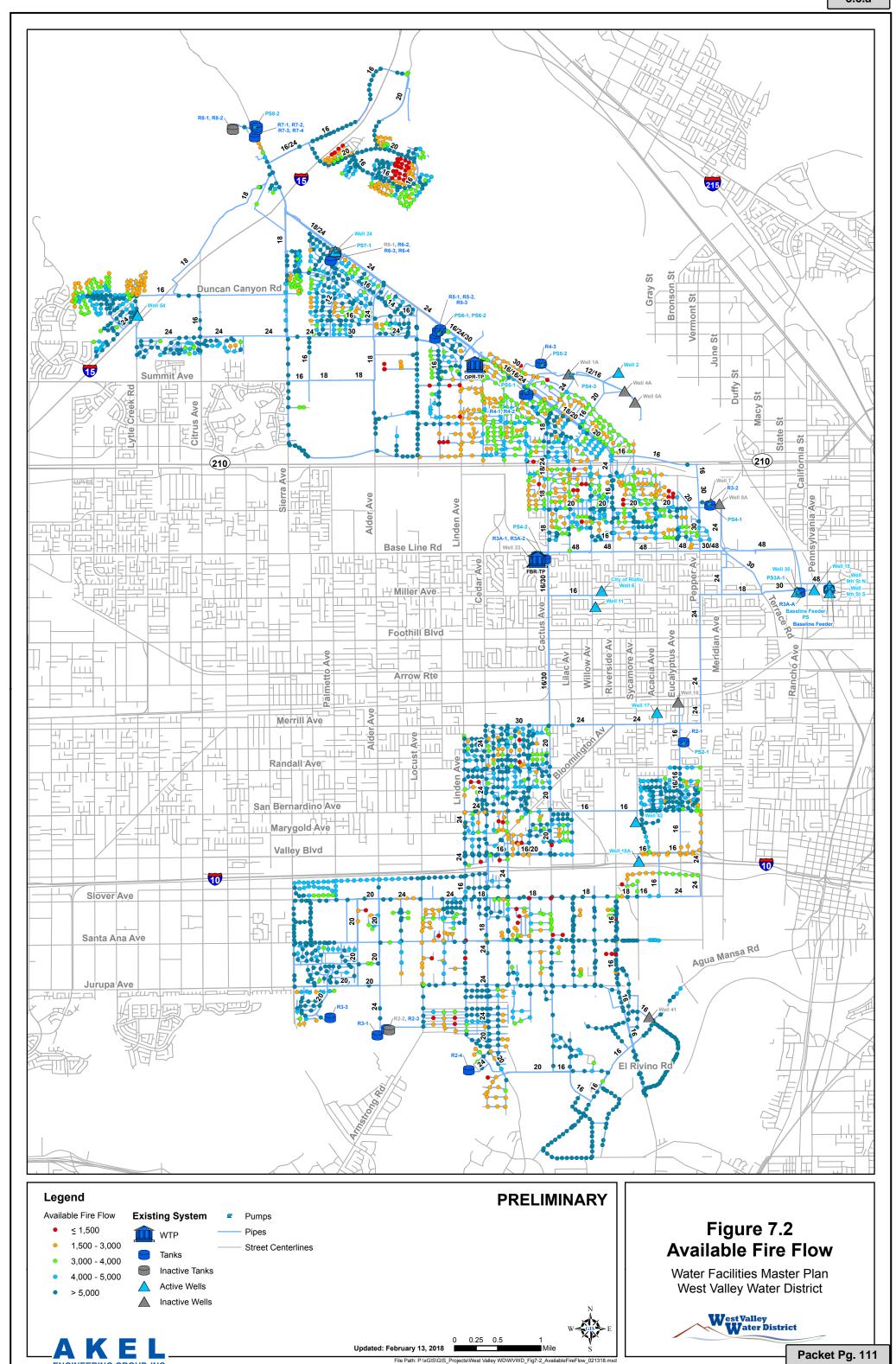
### 7.2.1 Fire Flow Improvements

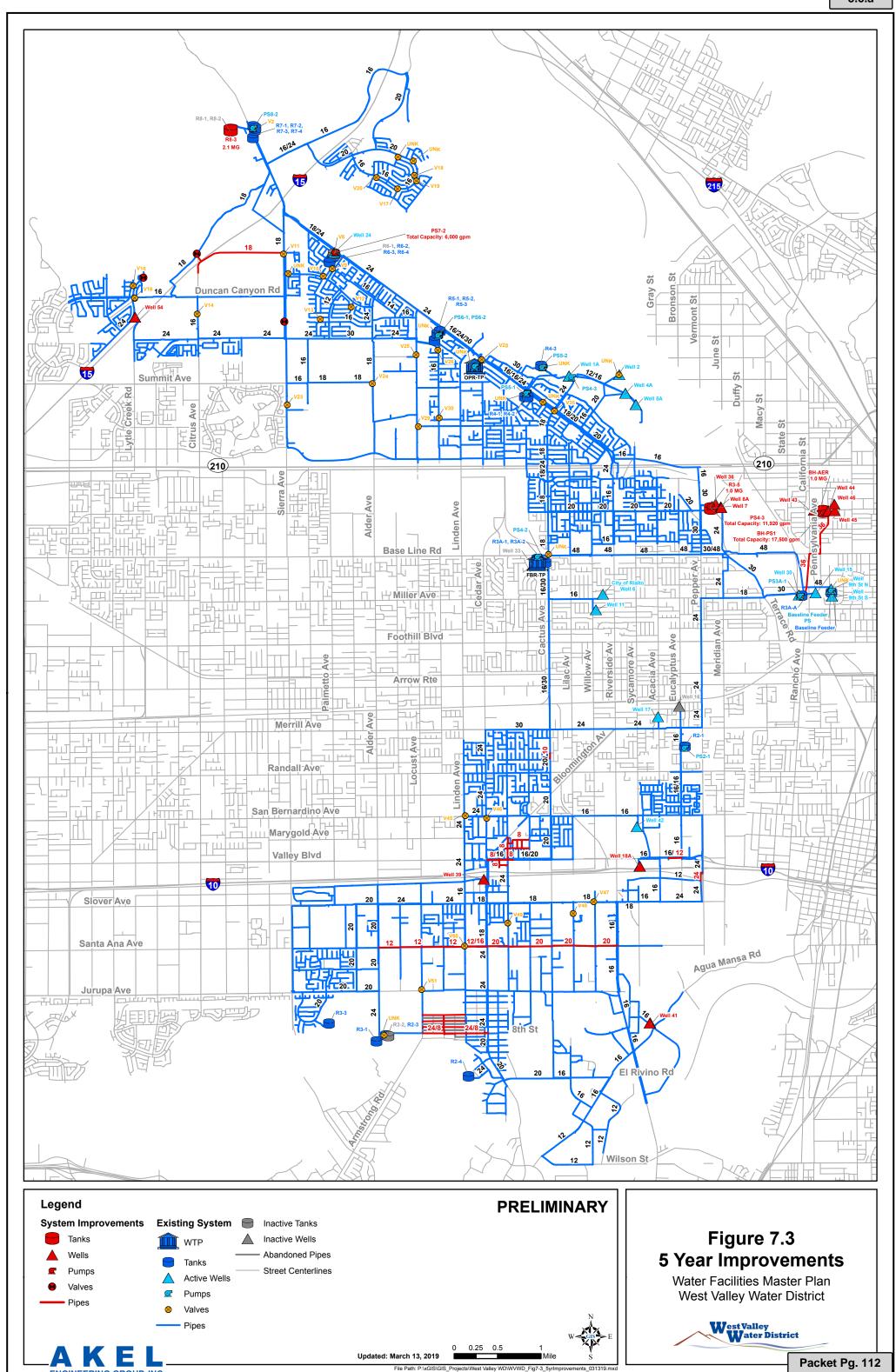
Improvements recommended to support fire flow delivery are shown with the 5-year improvements on Figure 7.3.

### 7.2.2 Other Potential Improvements

It should be noted that there are areas of the system that have vulnerabilities when assessed against the Master Plan fire flow criteria. However, it was determined that some of these areas may have reduced fire flow requirements, per the California Fire Code, or other potential fire fighting capabilities, and thus, improvements are not included in this Master Plan. As future development occurs, it is recommended that a development specific fire flow analysis be completed to document any potential deficiencies and appropriate mitigation be completed.







### 7.3 LOW PRESSURES ANALYSIS

The existing domestic water distribution system was evaluated to determine the minimum pressure adequacy during peak day demand conditions. During peak day demands, the minimum pressure requirement is 40 psi, while during the peak hour demand, the minimum pressure requirement is 35 psi. The hydraulic analysis indicated the existing system is able to provide minimum pressures reasonably well. Minimum pressures during peak day demand conditions are summarized graphically on Figure 7.4. Areas of low pressure are briefly described as follows:

- Zone 4, approaching Highway 210
- Zone 5, approaching Roemer WFF

### 7.4 HIGH PRESSURES ANALYSIS

The hydraulic model was also used to determine if the existing domestic water distribution system meets the District's System Performance and Design Criteria for maximum pressures. Under typical operating conditions the maximum allowable pressure in a pipeline is 130 psi, while the maximum service connection pressure is 80 psi. It is recommended that any new service connections made in areas identified as experiencing high pressure implement a pressure reducing device as part of the service connection. The hydraulic analysis indicated the existing system is able to provide minimum pressures reasonably well. Maximum pressures during peak day demand conditions are summarized graphically on Figure 7.5. Areas of maximum pressure are briefly summarized as follows:

- Zone 2, southeast of Agua Mansa Road
- Zone 8, Glen Helen Parkway
- Zone 6, southwest of I15 and Duncan Canyon Rd
- Zone 6, north of the existing Zone 5 tanks

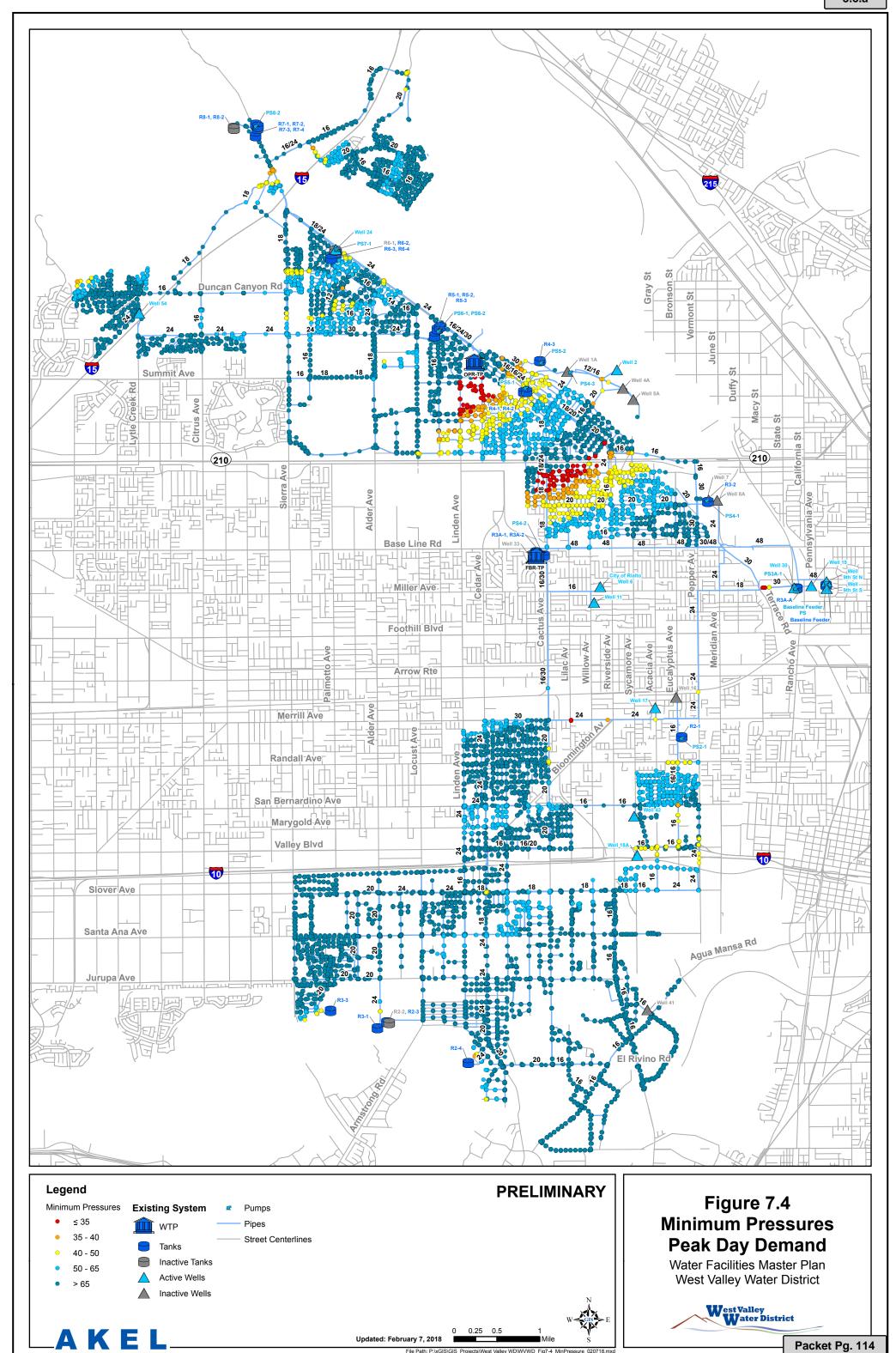
### 7.5 WATER SUPPLY REQUIREMENTS

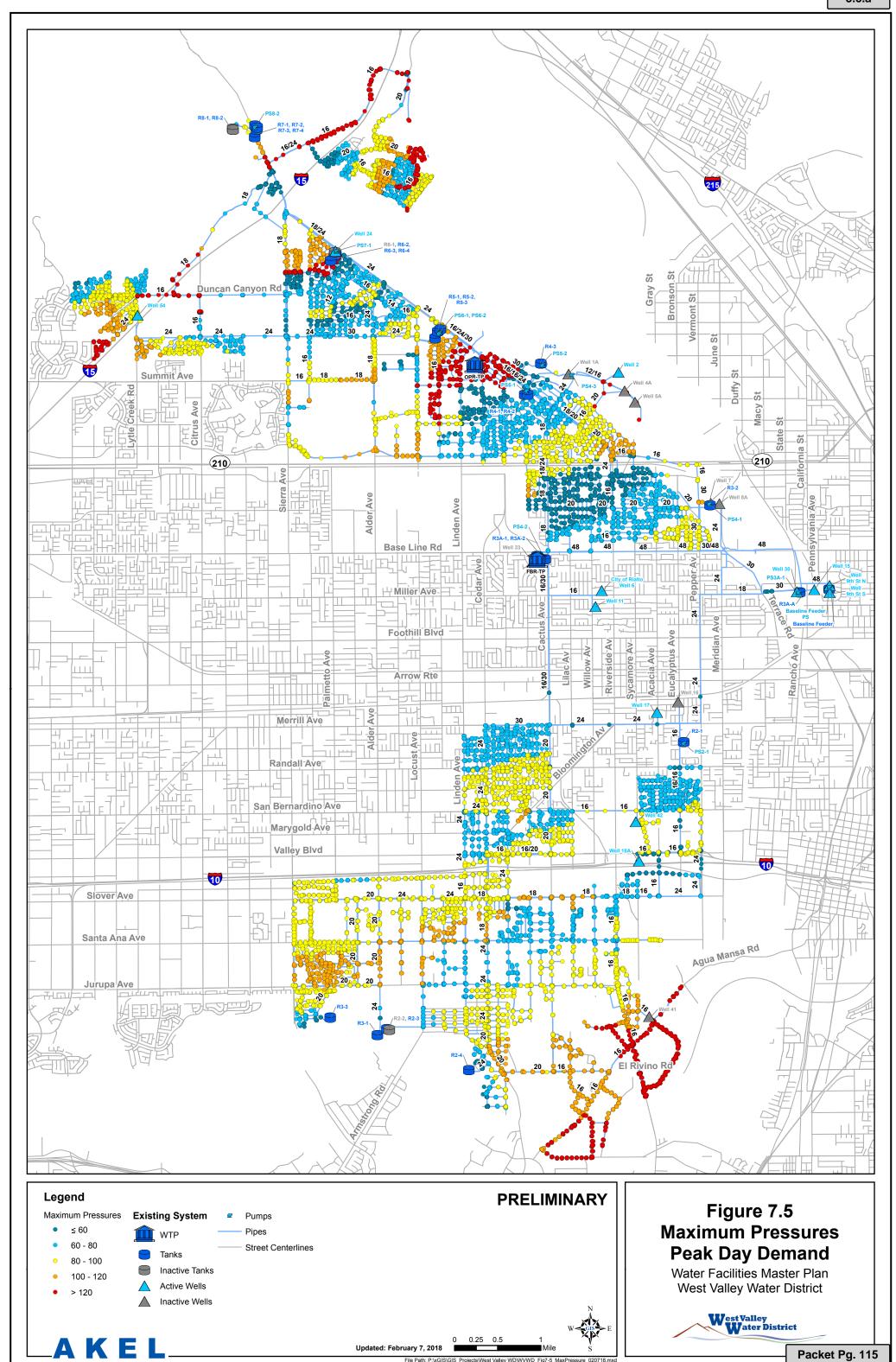
The District's existing water supply capacity is identified in this section. Additionally, this section documents the additional supply capacity recommended to meet the requirements of the 5-year and buildout development horizons.

### 7.5.1 Water Supply Scenarios

As discussed in previous chapters the District's existing supply capacity is comprised of both groundwater and treated surface water. For planning purposes, the supply capacity analysis considered two supply alternatives, which are summarized as follows:

Supply Scenario 1: This supply scenario assumes Roemer WFF is operating at maximum





treatment capacity, with groundwater wells providing the remaining supply requirements.

• **Supply Scenario 2:** This supply scenario assumes an interruption in SWP water availability and Roemer WFF is assumed to be treating Lytle Creek flows, which are estimated at 4,000 afy (3.6 mgd).

Thus, supply recommendations are based on the ability of the water facilities meeting each of the aforementioned supply scenarios.

### 7.5.2 System-Wide Water Supply Analysis

The system-wide water supply capacity analysis for existing and buildout conditions is summarized on Table 7.1, which includes the supply requirements and available supply volumes under both Supply Scenario 1 and Supply Scenario 2. Table 7.1 also documents the phased supply improvements, which includes the rehabilitation of existing wells and the construction of new wells. In addition to a system-wide supply capacity analysis.

As documented on Table 7.1, the District's supply facilities are capable of meeting the existing supply requirements. Under the conservative Supply Scenario 2, the District has a supply deficiency of approximately the District has an existing supply capacity surplus the District

### 7.5.3 Pressure Zone Supply Analysis

In addition to a system-wide water supply capacity analysis, the existing pressure zones were evaluated to determine the feasibility of reducing the interzonal supply dependencies with the construction and rehabilitation of new wells. Pressure Zones 2, 3, and 3A were evaluated independently to identify supply improvements to mitigate existing supply dependencies while Pressure Zones 4-8 were evaluated together, with future pump stations planned to convey the existing and future supplies to the higher zones. The pressure zone supply analyses are summarized in the following sections.

### **7.5.3.1 Pressure Zone 2**

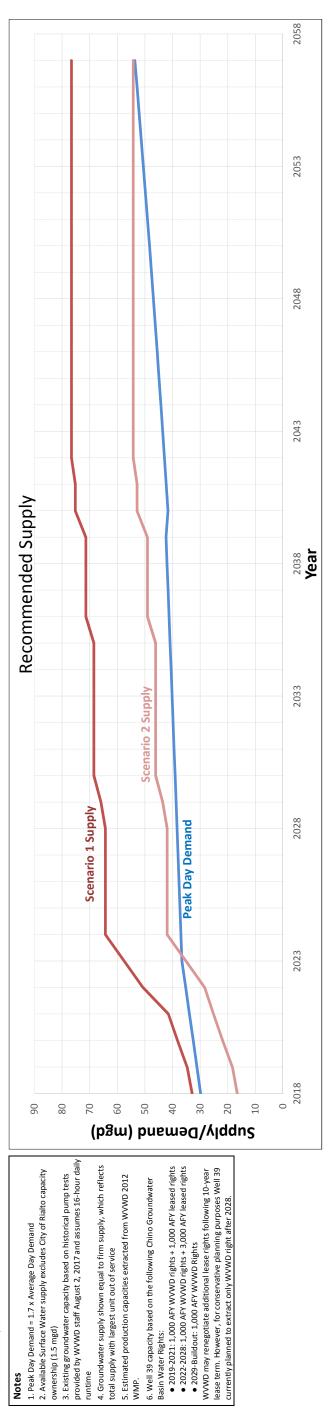
Under existing conditions Pressure Zone 2 relies on groundwater wells and PRVs from Pressure Zone 3 to meet existing supply requirements. As documented on Table 7.2, three new wells are recommended for equipping and construction to mitigate this existing supply dependency. Additionally, one new well will be required within the buildout development horizon to meet additional demands.

### 7.5.3.2 Pressure Zone 3

Under existing conditions Pressure Zone 3 utilizes groundwater wells and water delivered through the Meridian Turnout to meet existing supply requirements. As documented on **Table 7.3**, three wells are recommended for rehabilitation and construction to mitigate a portion of this supply dependency. It should be noted that the potential future wells in this pressure zone are located

Table 7.1 Phased Supply Planning
Water Facilities Master Plan
West Valley Water District

	2018 20	2019 2020	0 2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034 2	2035 20	2036 20	2037 203	2038 2039	707	10 2041	11 2042	12 2043	13 2044	1 2045	2046	2047	2048	2049	2050	2051	2022	2053	2054	2055	2056 2057
Population Forecasting																																					
Projected Annual Growth Rate	4.4% 4.	4.2% 4.0%	3.9%	3.7%	3.6%	0.7%	0.7%	0.7%	0.7%	0.7%	0.7%	0.7%	0.7%	0.7%	0.7%	0 %9:0	0.6% 0.	0.6% 0.	9.0 %9.0	%9.0 %9.0	-1	.9% 1.5%	% 1.5%	% 1.5%	% 1.5%	4.5%	, 1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5% 1.5%
Projected Population 8	87,590 91,	91,279 94,967	67 98,656 10	5 102,34	12,344 106,033	3 106,767	7 107,502	108,236	108,971	108,236 108,971 109,706 110,440	110,440	111,175	111,910 112,644		113,379 114,114		114,848 115	115,583 116	116,318 117,052	,052 117,787	115,	568 117,3	302 119,0	120,8	117,302 119,061 120,847 122,660	60 124,500		7 128,26	3 130,186	126,367 128,263 130,186 132,139	134,121	136,133	138,175	136,133 138,175 140,248 142,352		144,487 14	146,654 148,854
Projected Demands																																					
Average Day Demands, mgd	17.6	18.3 19.1	1 19.9	20.7	21.5	21.7	21.9	22.1	22.4	22.6	22.8	23.0	23.2	23.4	23.6	23.8 2	24.0 2	24.3 24.	ιč	24.7 24.9	24	.5 24.9	.9 25.2	25.	6 26.0	26.4	26.8	27.2	27.6	28.0	28.4	28.9	29.3	29.7	30.2	30.6	31.1 31.6
Peak Day Demands <sup>1</sup> , mgd	29.8	31.2 32.5	5 33.9	35.2	36.6	36.9	37.3	37.6	38.0	38.4	38.7	39.1	39.4	39.8	40.2	40.5	40.9	41.2 41.	9	42.0 42.4	.4 41.7	7 42.3	42	9 43	.6 44.2	44.9	45.6	46.2	46.9	47.6	48.3	49.1	49.8	9.05	51.3	52.1	52.9 53.7
Buildout Supply Analysis																																					
Required Supply (PDD)	29.8	31.2 32.5	5 33.9	35.2	36.6	36.9	37.3	37.6	38.0	38.4	38.7	39.1	39.4	39.8	40.2	40.5	40.9	41.2 41.	9	42.0 42.4	41	.7 42.3	.3 42.9	9 43.6	6 44.2	44.9	45.6	46.2	46.9	47.6	48.3	49.1	49.8	9.09	51.3	52.1	52.9
Available Supply			OPR E	OPR Expansion Online	n Online																																
Supply Scenario 1 (Maximum Surface Water Treatment)	rface Water	Treatmer		<b>→</b>																																	
						18.9	18.9	18.9	18.9	18.9	18.9	18.9	18.9	18.9	18.9						0								18.9	18.9	18.9	18.9	18.9	18.9	18.9		
Groundwater <sup>3,4</sup>				20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0 20	20.0 20.0	.0 20.0	0 20.0	.0 20.0	0 20.0	0 20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0 20.0
	32.9 3.	32.9 32.9	9 32.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9 38	38.9 38	38.9 38.9	38	6.88 6.	6.88 6.	9 38.9	6 38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9	38.9 38.9
Supply Scenario 2 (Minimum Surface Water Treatment)	rface Water	Treatmen	٩										_																	-							
Surface Water <sup>2</sup>	2.1 2	2.1 2.1	1 2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1 2	2.1 2	2.1 2.	2.1 2.1	1 2.1	1 2.1	1 2.1	1 2.1	1 2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1
	14.5 1	14.5 14.5	5 14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5 14	14.5 14	14.5 14.5	14	.5 14.5	.5 14.5	5 14.5	5 14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5	14.5 14.5
Total Available Supply	16.6	16.6 16.6	6 16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6 16	16.6 16	16.6 16.	6 16.	.6 16.6	.6 16.6	6 16.6	6 16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6	16.6 16.6
Recommended New Wells																																					
WellID	*	39 41, 50	50 16, 52	2 29A	43, 44	45, 46					7,8A	36						51			34B, 3	35C	22A	ď													
Estimated Production Capacity <sup>5,6</sup>		2.1, 1.4	1.4 1.4, 1.9	9 1.4	3.4, 3.4	1 3.4, 3.4					2, 2.3	2.6					. 4	2.9			1.9, 1	1.9	1.4														
Supplied Zone		2, 3	3,3	2	3A, 3A	3A, 3A					4,4	4						2			4,4		4														
New Firm Well Capacity	20.0	21.8 25.3	3 28.6	31.8	38.6	45.4	45.4	45.4	45.4	45.4	47.0	49.6	49.6	49.6	49.6	49.6	49.6 5.	52.5 52.	2.5 52.	2.5 52.	.5 56.3	3 56.3	.3 57.7	7 57.7	7 57.7	57.7	57.7	57.7	57.7	57.7	57.7	57.7	57.7	57.7	57.7	27.7	57.7 57.7
Supply Capacity Analysis																																					
Scenario 1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -	- Treat maximum Lytle Creek and State Water Project water at OPR WFF, with 1,680 afy delivery to City of Rialto - Includes additional 4,000 AFY SWP water delivery for 15 years (2022-2036) - Construct new wells to offset surface water treatment - Onstruct new wells to offset surface water treatment - OPR WFF treatment capacity expansion online 2022	num Lytle fy delivery ditional 4, existing c ew wells to	Creek and Stocity of R 100 AFY SW filine wells offset surfiped pacity expe	state Wai ialto 'P water i 'ace wate	ter Project delivery fo er treatmer	water at rr 15 years	OPR WFF,																														
	3.1 3	3.5 5.7	7.6	15.5	20.9	27.3	27.0	26.6	26.3	25.9	27.2	29.4	29.1	28.7	28.4	28.0 2	27.6 3	30.2 29.	9.8 29.	9.4 29.1	33.	.6 32.9	.9 33.7	7 33.0	0 32.4	1 31.7	31.1	30.4	29.7	29.0	28.3	27.5	26.8	26.1	25.3	24.5	23.7 22.9
Scenario 2 de de (Conservative Surface ex Water Supplies)	- Treat 4,000 AFY of Lytle Creek water at OPR WFF, with 1,680 afy delivery to City of Rialto. Existing Lytle Creek wells considered non-reliable and not include existing Lytle Creek wells. Rehabilitate existing offline wells Construct new wells as needed	ity of Rialt ity of Rialt le Creek w ily totals existing o ew wells a:	le Creek wa :Ils conside :fline wells	ster at OF	t OPR WFF, with 1,680 afy non-reliable and not included in	ith 1,680 and not incl	afy uded in																														
	13.3	-178 -107	7 -8.7	9 9	7.	0.5	4.6	4.3	3.0	3.6	4.8	7.1	6.7	6.4	6.0	5.6	7 2 7	7 8 7	7.4 7.1	1 67	-	2 10.6	6 113	2 10 7	7 10.0	0.4	8.7	8.0	7.3	6.6	5.9	5.2	77	3.7	2.0	2.2	1.4



## **Table 7.2 Pressure Zone 2 Supply Analysis**Water Facilities Master Plan

West Valley Water District

**PRELIMINARY** 

			3.4.5			9 -		11:19:47
Year	ADD <sup>1</sup>	PDD <sup>2</sup>	Supply Sources","		Groundwater Supply	er Supply	surpius	surplus/Deficit
			Well	Source	Total	Firm	Total	Firm
	(pgm)	(pgm)		(mgd)	(mgd)	(mgd)	(mgd)	(mgd)
2018/19	2.65	4.51	Existing W-17	96.0				
			Existing W-18A	2.08	3.04	0.96	-1.47	-3.55
2019/20	2.68	4.56	Equip W-41 (Treatment)	2.10	5.14	3.04	0.59	-1.51
2020/21	2.71	4.60	Equip W-16 (Pump Shaft)	1.40	6.54	4.44	1.94	-0.16
2021/22	2.73	4.65	Construct W-29A	1.40	7.94	5.84	3.30	1.20
2022/23	2.76	4.69			7.94	5.84	3.25	1.15
2023/24	2.79	4.74			7.94	5.84	3.21	1.11
Buildout	4.55	7.74	Construct W-51	2.90	10.84	7.94	3.10	0.20
ENGINEERING GROUP, INC.	UP, INC.							2/4/2019

;:

1. Demands are based on the following:

- 2018/19: Estimated existing demand

- 2019/19-2022/23: Linear interpolation between 2018/19 and 2023/24

- 2023/24: Additional demand based on projected 5-year growth.

2. PDD =  $1.7 \times ADD$ 

3. Existing well capacities based on pump tests received from District staff August 2, 2017 and assume 16-hour daily operations.

4. Future well capacities based on 2012 Water Master Plan and assume 16-hour daily operations.

5. Firm capacity excludes largest groundwater supply.

# Table 7.3 Pressure Zone 3 Supply Analysis Water Facilities Master Plan West Valley Water District

**PRELIMINARY** 

;			Supply Sources <sup>3,4,5</sup>		Groundwater Supply <sup>6</sup>	er Supply <sup>6</sup>	Surplus	Surplus/Deficit
Year	ADD-	PDD-	Well	Source	Total	Firm	Total	Firm
	(mgd)	(mgd)		(pgm)	(pgm)	(pgm)	(mgd)	(pgm)
2018/19	3.87	6.57	Existing W-15	1.32				
			Existing W-30	1.46				
			Existing W-42	1.56	4.34	2.78	-2.23	-3.79
2019/20	3.92	99.9	Construct W-50	1.40				
			Rehabilitate W-39	3.80	9.54	5.74	2.88	-0.92
2020/21	3.97	6.75	Construct W-52	1.90	11.44	7.64	4.69	0.89
2021/22	4.02	6.84			11.44	7.64	4.60	08.0
2022/23	4.08	6.93			11.44	7.64	4.51	0.71
2023/24	4.13	7.02			11.44	7.64	4.42	0.62
Buildout	6.63	11.28	Meridian Turnout Delivery	3.63	15.07	11.27	3.80	0.00
ENGINEERING GROUP, INC								2/4/2019

es:

- 1. Demands are based on the following:
- 2018/19: Estimated existing demand
- 2019/19-2022/23: Linear interpolation between 2018/19 and 2023/24
- 2023/24: Additional demand based on projected 5-year growth.
- $2. PDD = 1.7 \times ADD$
- 3. Existing well capacities based on pump tests received from District staff August 2, 2017 and assume 16-hour daily operations.
  - 4. Future well capacities based on 2012 Water Master Plan and assume 16-hour daily operations.
- 5. Under buildout development PDD conditions Pressure Zone 3 will require approximately 2,500 gpm supply deliveries through the District's Meridian Turnout facility.
- Firm capacity excludes largest groundwater supply.

within the Chino Groundwater Basin. Based on the existing water rights limitations within the Chino Groundwater Basin, the District currently plans to extract its allowed amount utilizing Well 39 and no additional wells are planned for construction. Therefore, under the buildout development horizon Pressure Zone 3 will require continued supply deliveries through the Meridian Turnout.

### 7.5.3.3 Pressure Zone 3A

Under existing conditions Pressure Zone 3A utilizes the FBR treatment facility to meet existing supply requirements. As documented on **Table 7.4**, under existing and buildout conditions, this facility is anticipated to be sufficient to meet the zone's supply requirements. However, it should be noted that in the event the FBR treatment facility supply is interrupted this pressure zone can receive deliveries through both the Baseline Feeder Pipeline and Pump Station 3A.

### 7.5.3.4 Pressure Zone 4-8 (North System Pressure Zones)

Under existing conditions Pressure Zones 4, 5, 6, 7, and 8 are supplied by both groundwater wells and the OPR treatment facility. As summarized on **Table 7.5**, under Supply Scenario 1 the existing water supply facilities are capable of meeting the supply requirements of the pressure zones. However, under the conservative Supply Scenario 2, the available groundwater supply capacity is unable to offset the reduction in surface water available for treatment. In order to mitigate this deficiency the new wells are recommended for construction and equipping; this includes the development of the Bunker Hill well field, comprised of future wells 43, 44, 45, and 46, which is recommended for immediate design and construction. Additionally, to continue to maximize the treatment of surface water supplies, the OPR WFF 6.0 mgd expansion is planned for immediate design and construction. This capacity expansion will enable the District to take advantage of available surface water supplies and minimize groundwater pumping when possible.

### 7.5.4 Recommended Supply Improvements

The following sections summarize the recommended supply improvements intended to mitigate existing supply deficiencies and accommodate future growth under the five-year and buildout development horizon.

### 7.5.4.1 Five-Year Supply Improvements

The following section summarizes the supply improvements recommended for implementation within the five-year development horizon, which are briefly on the following pages.

• **Well 16:** This well has a design capacity of 1,500 gpm and discharges into water storage reservoir 2-1. This well has existing treatment for perchlorate and additional treatment is required for nitrate before being activated.

# Table 7.4 Pressure Zone 3A Supply Analysis

Water Facilities Master Plan West Valley Water District **PRELIMINARY** 

Year	ADD <sup>1</sup>	PDD <sup>2</sup>	Supply Sources <sup>3,4</sup>	rces <sup>3,4</sup>	Groundwat	Groundwater Supply <sup>5</sup>	Surplus	Surplus/Deficit
			Well	Source	Total	Firm	Total	Firm
	(mgd)	(mgd)		(mgd)	(pgm)	(pgm)	(mgd)	(pgw)
2018/19	1.04	1.77	FBR	2.88	2.88	2.88	1.11	1.11
2019/20	1.05	1.78			2.88	2.88	1.10	1.10
2020/21	1.05	1.79			2.88	2.88	1.09	1.09
2021/22	1.06	1.80			2.88	2.88	1.08	1.08
2022/23	1.07	1.81			2.88	2.88	1.07	1.07
2023/24	1.07	1.82			2.88	2.88	1.06	1.06
Buildout	1.11	1.89			2.88	2.88	0.99	0.99
ENGINEERING GROUP, INC.	ROUP, INC.							2/4/2019

Notes:

- 1. Demands are based on the following:
- 2018/19: Estimated existing demand
- 2019/19-2022/23: Linear interpolation between 2018/19 and 2023/24
- 2023/24: Additional demand based on projected 5-year growth.
- 2. PDD =  $1.7 \times ADD$
- 3. Existing well capacities based on pump tests received from District staff August 2, 2017 and assume 16-hour daily operations.
- 4. Future well capacities based on 2012 Water Master Plan and assume 16-hour daily operations.
- 5. The FBR treatment facility is planned to provide supplies to Pressure Zone 3A under existing and buildout conditions. However, the District can provide supplemental supplies to this zone through the Baseline Feeder Pipeline as well as Pump Station 3A.

### Table 7.5 North System Pressure Zone Supply Analysis

Water Facilities Master Plan

West Valley Water District

Year Al (n (n 2018/19 9			Groundwater Supply	er Supply			Surface Water	Water		Surplus/ Deficit	Deficit	
	ADD <sup>1</sup> P	PDD <sup>2</sup>	Supply Source <sup>3,4,5</sup>		Supply Capacity <sup>6</sup>	apacity <sup>6</sup>	Scenario 1 <sup>7,8</sup> (Maximum	Scenario 2 <sup>9</sup> (Minimum	Scenario 1 (Maximum Surface Water)	rio 1 rface Water)	Scenario 2 (Minimum Surface Water)	rio 2 rface Water)
			Well	Capacity	Total	Firm	Surface Water)	Surface Water)	Total	Firm	Total	Firm
	ı) (pgm)	(mgd)		(pgm)	(mgd)	(pgm)	(mgd)	(mgd)	(mgd)	(mgd)	(pgw)	(mgd)
	9.81 1	16.67	Existing W-24	0.46								
			Existing W-54	0.88								
			Existing W-9th St (North)	2.88								
			Existing W-9th St (South)	3.36	7.58	4.22	12.90	2.10	3.81	0.45	-8.65	-10.35
2019/20 10	10.55 1	17.94			7.58	4.22	12.90	2.10	2.54	-0.82	-8.26	-11.62
2020/21 11	11.30 1	19.21			7.58	4.22	12.90	2.10	1.27	-2.09	-9.53	-12.89
2021/22 13	12.05	20.48			7.58	4.22	12.90	2.10	0.00	-3.36	-10.80	-14.16
2022/23 12	12.79 2	21.75	Construct W-43	3.40			OPR WFF Expansion Online	nsion Online				
			Construct W-44	3.40	14.38	10.98	18.90	2.10	11.53	8.13	-5.27	-8.67
2023/24 13	13.54 2	23.02	Construct W-45	3.40								
			Construct W-46	3.40	21.18	17.78	18.90	2.10	17.06	13.66	0.26	-3.14
Buildout 19	19.32 3	32.84	Equip W-7 (Blind Flanged)	2.00								
			Equip W-8A (Treatment)	2.30								
			Equip W-22A (Treatment)	1.40								
			Construct W-34B	1.90								
			Construct W-35C	1.90								
AKFI			Equip W-36 (Treatment)	2.60	33.28	29.88	18.90	2.10	19.34	15.94	2.54	-0.86

1. Demands are based on the following:

- 2018/19: Estimated existing demand

- 2019/19-2022/23: Linear interpolation between 2018/19 and 2023/24

- 2023/24: Additional demand based on projected 5-year growth.

2. PDD =  $1.7 \times ADD$ 

3. Existing well capacities based on pump tests received from District staff August 2, 2017 and assume 16-hour daily operations.

4. Future well capacities based on 2012 Water Master Plan and assume 16-hour daily operations.

5. For conservative supply planning purposes existing Lytle Creek groundwater basin wells (W-1, W-2, W-4A, W-5A) are considered non-reliable and excluded from the supply analysis.

6. Firm capacity excludes largest groundwater supply.

7. Scenario 1 assumes OPR WFF operating at maximum treatment capacity, with 1.5 mgd of treated water delivered to the City of Rialto.

8. The OPR WFF treatment capacity expansion is assumed to come online in the year 2022/23.

9. Scenario 2 assumes OPR WFF treating minimum reliable Lytle Creek supply of 4,000 AFY, with 1.5 mgd of treated water delivered to the City of Rialto.

- Well 29A: This well has a design capacity of 1,500 gpm and is planned to discharge directly into Pressure Zone 2. Treatment for perchlorate and nitrate is required before being activated.
- Well 39: This well has a capacity of up to 4,000 gpm and is planned to discharge directly into Pressure Zone 3. Once drilled, water quality sampling indicated nitrate exceeding regulatory limits. As such, the well was never equipped, and requires treatment and equipping prior to production.
- Well 41: This well has a design capacity of 2,000 gpm and directly discharges into Pressure Zone 2. Currently, this well experiences levels of perchlorate above the regulated maximum contaminant levels and wellhead treatment is required to bring online. Existing treatment vessels located at the reservoir 2-1 site are currently unused and may potentially be relocated to this well site. Feasibility of the relocation of these vessels is dependent on the site constraints. Additional land purchase may be required, should the site not accommodate the vessels. It should be noted that the rehabilitation of this well is expected to reduce the required PRV flow from Pressure Zone 3.

### 7.5.5 Recommended Supply Improvements

- Wells 43, 44, 45, and 46: These wells each have a planned design capacity of 3,400 gpm and are planned as part of the Bunker Hill wellfield development. These wells are planned to discharge into a new aeration tank, which will act as a forebay to a new pump station discharging into a transmission pipeline that will ultimately connect to an existing 30-inch transmission main near the Pump Station 3A site before being conveyed to the Lord Ranch Facility.
- Well 50: This well has a design capacity of 1,500 gpm and is planned to discharge directly
  into Pressure Zone 3. Once drilled, water quality sampling indicated perchlorate exceeding
  regulatory limits. Treatment for perchlorate and nitrate is required before being activated.
- Well 52: This well has a design capacity of 2,000 gpm and is planned to discharge directly into Pressure Zone 3. Treatment for perchlorate and nitrate is required before being activated.

### 7.5.5.1 Buildout Supply Improvements

The following section summarizes the supply improvements recommended for implementation within the buildout development horizon, which are briefly summarized as follows:

 Well 7: This well has a design capacity of 2,100 gpm and is planned to discharge directly into water storage reservoir 3-2. According to District records this well is currently blind flanged.

- **Well 8A:** This well has a design capacity of 2,400 gpm and discharges directly into water storage reservoir 3-2. Currently this well experiences high levels of arsenic and wellhead treatment is required prior to activation.
- Well 22A: This well has a design capacity of 1,500 gpm and discharges directly into Pressure Zone 4. Currently, this well experiences high levels of nitrate and wellhead treatment is recommended to bring online. This well will require further study to determine the best methodology to mitigate the ongoing nitrate contamination.
- Well 34B: This well has a planned design capacity of 2,000 gpm and discharges directly
  into Pressure Zone 4. This well is replacing a previously destroyed well and will require redrilling and equipping. It is also assumed that this well will require wellhead treatment for
  arsenic levels required prior to activation.
- Well 35C: This well has a planned design capacity of 2,000 gpm and discharges directly
  into Pressure Zone 4. A casing currently exists at this well location and a new study is
  recommended to confirm the construction and water quality requirements of this well. It is
  also assumed that this well will require wellhead treatment for arsenic levels required prior
  to activation.
- **Well 36:** This well has a design capacity of 2,700 gpm and discharges directly into water storage reservoir 3-2. Currently, this well experiences high levels of arsenic and wellhead treatment is required prior to activation.
- Well 51: This well has a design capacity of 2,000 gpm and is planned to discharge directly
  into Pressure Zone 2. The specific location of this well has not been determined and well
  site investigations should include a water quality study to determine the need for
  treatment. It should be noted that the construction of this well will reduce the required PRV
  flow from Pressure Zone 3.

### 7.5.6 Water Supply Treatment Evaluation

This section documents the groundwater and surface water treatment options for the District, as recommended by Kleinfelder.

### 7.5.6.1 Groundwater Treatment

Table 7.6 documents the existing conditions of the District's groundwater wells. There are currently 12 active production wells. Some of the production wells are contaminated with perchlorate, nitrate, arsenic, or have issues with air entrapment producing milky water and inducing customer complaints. The District has been proactive in its efforts to install wellhead treatment to maintain the operational status of these wells, and provide high quality drinking water.

Table 7.6 Well Production Capacity and Water Quality Issues
Water Facilities Master Plan
West Valley Water District

Water Capacity 2055 (afy) (gpm) 2055 (afy) (gpm) 2,400 2,400 2,000 2,000 2,000 1,2,860 6,700 1,500 1,500 1,500 1,500 1,500 2,600 2,600 2,600 2,600			Five Year Pro	Five Year Projections, 2022		5	Ultimate Buildout, 2055	out. 2055		
Second Paris   Paris	: 0:+630 -		Product	Severe			Product.			
Seek Basin		Pump Capacity					10	Drought Water Capacity Demand 2055	Current Condition of Use	Water Quality Issues
3.4 LC (8871 Martin Road, San Bernardino         2,100         2.0         1.0         2,100           3,4         LC (8871 Martin Road, San Bernardino)         2,400         2.3         0.9         2,400           3,4         LC (20600 Walnut Avenue, San Bernardino)         2,400         2.3         0.9         2,400           4         LC (20600 Walnut Avenue, San Bernardino)         2,800         2.7         1.6         2,800           4         LC (20600 Walnut Avenue, Rialto         2,800         2.7         1.6         2,800           4         LC (20600 Walnut Avenue, Rialto         2,800         2.7         1.6         2,800           4         LC (20600 Walnut Avenue, Rialto (Future))         7,300         2.7         1.6         2,800           4         LC (20610 Walnut Avenue, Rialto (Future))         7,300         7.0         3.5         1,200           4         LC (20610 Walnut, Rialto (Future))         7,300         7.0         3.5         1,500           4         LC (20610 Walnut, Rialto (Future))         1,250         1.2         0.6         1,500           5         R (20610 Walnut, Rialto (Future))         1,200         1.7         0.8         1,500           2         R (20610 Walnut, Rial		(mdg)	(mgd)	(mgd)	(afy)	(gpm)	(mgd)	(mgd) (afy)		
3,4         LC         6871 Martin Road, San Bernardino         2,400         2.3         0.9         1,00         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400         2,400										
3,4         LC         G871 Martin Road, San Bernardino         2,400         2.3         0.9         2,400           3,4         LC         20600 Walnut Avenue, Rialto         2,800         2.7         1.6         2,700           4         LC         19973 Country Club Drive, Rialto         2,800         2.7         1.6         2,800           4         LC         19914 N. Sycamore Avenue, Rialto         7,300         7.0         1.6         2,000           4         LC         5914 N. Sycamore Avenue, Rialto (Future)         7,300         7.0         3.5         2,000           4         LC         5835 N. Sycamore Avenue, Rialto (Future)         7,300         7.0         3.5         1,260           4         LC         5835 N. Sycamore Avenue, Rialto (Future)         7,300         7.0         3.5         1,560           4         LC         5835 N. Sycamore Avenue, Rialto (Future)         1,500         7.0         3.5         1,560           5         R         2065 S. Eucalyptus Avenue, Rialto (Future)         1,500         1,2         0.0         1,500           2         R         2065 S. Eucalyptus Avenue, Rialto (Future)         1,800         1,7         0.9         1,500           3		2,100	2.0	1.0		2,100	2.0	1.0	Not in operation, Blind flanged	
3,4         LC         206000 Walnut Avenue, San Bernardino         2,800         2.7         1.6         2,700           4         LC         19523 Country Club Drive, Rialto         2,800         2.7         1.6         2,800           4         LC         19514 N. Sycamore Avenue, Rialto         2,800         2.7         1.6         2,800           4         LC         5914 N. Sycamore Avenue, Rialto (Future)         7,300         7.0         3.5         2,000           4         LC         5955 N. Sycamore Avenue, Rialto (Future)         7,300         7.0         3.5         2,000           4         LC         5855 N. Sycamore Avenue, Rialto (Future)         7,300         7.0         3.5         12,860           4         LC         5855 N. Sycamore Avenue, Rialto (Future)         7,300         7.0         3.5         12,860           4         LC         5855 N. Sycamore Avenue, Rialto (Future)         1,250         1.4         0.8         1,550           2         R         296 S. Eucalyptus Avenue, Rialto (Future)         1,250         1.2         0.6         1,550           2         R         404 S. Acacia Avenue, Rialto (Future)         1,250         1.7         0.9         1,500           3.A	_	2,400	2.3	6:0		2,400	2.3	6.0	Not currently used, arsenic issue	Low level arsenic
4         LC         19523 Country Club Drive, Rialto         2,800         2.7         1.6         2,800           4         LC         5914 N. Sycamore Avenue, Rialto         2,800         2.7         1.6         2,600           4         LC         5914 N. Sycamore Avenue, Rialto (Future)         7,300         7.0         3.5         2,000           4         LC         5825 N. Sycamore Avenue, Rialto (Future)         7,300         7.0         3.5         1,2860           TOTAL LC Current           TOTAL LC Current           TOTAL LC Basin         7,300         7.0         3.5         1,2860           TOTAL LC Basin         1,500         0.0         0.0         0.0         1,280           TOTAL LC Basin         1,500         1,4         0.8         1,500           Coltone Basin           TOTAL LC Basin         1,500         1,2         0.0         0.0         0.0         1,250           TOTAL LC Basin         1,500         1,2         0.6         1,500         1,500           Eucalyptus Avenue, Rialto (Future)         1,250         1,2         0.6         0.0         0.0         0.0         <	enue, San Bernardino					2,700	2.6	6.0	Not currently used	Arsenic removal required
4         LC         19973 Country Club Drive, Rialto         2,800         2.7         1.6         2,800           4         LC         5914 N. Sycamore Avenue, Rialto         T. T. T. T. S.	ub Drive, Rialto					260	0.7	9.0	Not currently used due to declining water level	
4         LC         5914 N. Sycamore Avenue, Rialto         7.300         7.0         3.5         2.500           4         LC         19655 Country Club Drive, Rialto (Future)         7,300         7.0         3.5         2,000           4         LC         19655 Country Club Drive, Rialto (Future)         7,300         7.0         3.5         12,860           4         LC         5855 N. Sycamore Avenue, Rialto (Future)         7,300         7.0         3.5         12,860           7-001 LC EUTURE         TOTAL LC Basin         7,300         7.0         3.5         19,560           5         R         206 S. Eucalyptus Avenue, Rialto         1,500         1.4         0.8         1,500           2         R         404 S. Acacia Avenue, Rialto (Future)         1,250         1.2         0.6         1,500           3A         R         238 W. Victoria Street, Rialto (Future)         1,800         2,6         0.0         0.0         1,7         0.9         1,800           3A         R         855 W. Baseline Road, Rialto (Future)         2,600         2.5         1,3         2,600         2,5         1,3         2,600         2,600         2,600         2,600         2,600         2,600         2,600	ub Drive, Rialto	2,800	2.7	1.6		2,800	2.7	1.6	Has arsenic treatment, coagulation line	Arsenic
4         LC         19655 Country Club Drive, Rialto (Future)         7,300         7.0         3.5         2,000           4         LC         19655 Country Club Drive, Rialto (Future)         7,300         7.0         3.5         2,000           4         LC         5855 N. Sycamore Avenue, Rialto (Future)         7,300         7.0         3.5         12,860 <td< td=""><td>e Avenue, Rialto</td><td></td><td></td><td></td><td></td><td>2,600</td><td>2.5</td><td>6.0</td><td>Not currently used due to declining water level</td><td></td></td<>	e Avenue, Rialto					2,600	2.5	6.0	Not currently used due to declining water level	
4         LC         5855 N. Sycamore Avenue, Rialto (Future)         7,300         7.0         3.5         2,000           TOTAL LC Current           Colton Basin           TOTAL LC FUTURE         7,300         7.0         3.5         12,860           Colton Basin         7,300         7.0         3.5         12,860           Colton Basin         1,500         1.4         0.8         1,560           2         R         296 S. Eucalyptus Avenue, Rialto         1,500         1.2         0.6         1,500           2         R         404 S. Acacia Avenue, Rialto         1,800         1.7         0.9         1,500           3A         R         238 W. Victoria Street, Rialto         1,800         1.7         0.9         1,500           3A         R         855 W. Baseline Road, Rialto         2,600         2.5         1.3         2,600           4         R         5700 N. Riverside Avenue, Rialto (Future)         200         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.00         0.00         0.00         0.00         0.00         0.00         0.00         0.00         0.00	e Avenue, Rialto					2,200	2.1	1.0	Not currently used due to declining water level	
4         LC         5855 N. Sycamore Avenue, Rialto (Future)         7,300         7.0         3.5         12,860           Colton Basin         7,300         7.0         3.5         12,860         6,700           Colton Basin         TOTAL LC FUTURE         7,300         7.0         3.5         19,560           2         R         206 S. Eucalyptus Avenue, Rialto         1,500         1.4         0.8         1,500           2         R         404 S. Acacia Avenue, Rialto (Future)         1,250         1.2         0.6         1,500           3A         R         Eucalyptus Avenue, Rialto (Future)         1,800         1.7         0.9         1,800           3A         R         855 W. Baseline Road, Rialto (Future)         2,600         2.5         1.3         2,600           4         R         8700 N. Riverside Avenue, Rialto (Future)         200         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0	ub Drive, Rialto (Future)					2,000	1.9	8.0	Not constructed, replacement for Well 34B	Assumed As removal
Colton Basin         TOTAL LC FUTURE         7,300         7.0         3.5         12,860           Colton Basin           2         R         296 S. Eucalyptus Avenue, Rialto         1,500         1.4         0.8         1,500           2         R         404 S. Acacia Avenue, Rialto (Future)         1,250         1.2         0.6         1,500           3A         R         238 W. Victoria Street, Rialto         1,800         1.7         0.9         1,800           3A         R         855 W. Baseline Road, Rialto (Future)         2,600         2.5         1.3         2,600           4         R         5700 N. Riverside Avenue, Rialto (Future)         200         0.0         0.0         0.0         0.0           6         R         4334 N. Riverside Avenue, Rialto         Rialto         0.0         0.0         0.0         0.0         0.0         0.0	e Avenue, Rialto (Future					2,000	1.9	8.0	Not constructed, replacement for capped Well 35C	Assumed As removal
Colton Basin         TOTAL LC FuTURE         0         0.0         0.0         0.0         6,700           TOTAL LC Basin           Colton Basin           2         R         296 S. Eucalyptus Avenue, Rialto         1,500         1.4         0.8         1,500           2         R         404 S. Acacia Avenue, Rialto (Future)         1,800         1.7         0.6         1,500           3A         R         Eucalyptus Avenue, Rialto (Future)         1,800         1.7         0.9         1,800           3A         R         238 W. Victoria Street, Rialto         2,600         2.5         1.3         2,600           3A         R         855 W. Baseline Road, Rialto (Future)         2,600         2.5         1.3         2,600           6         R         4334 N. Riverside Avenue, Rialto         200         2.0         0.0         2.0		7,300	7.0	3.5		12,860	12.3	0.9		
TOTAL LC Basin         7,300         7.0         3.5         19,560           Colton Basin         2         R         296 S. Eucalyptus Avenue, Rialto         1,500         1.4         0.8         1,500           2         R         404 S. Acacia Avenue, Rialto (Future)         1,250         1.2         0.6         1,500           3A         R         Eucalyptus Avenue, Rialto (Future)         1,800         1.7         0.9         1,800           3A         R         855 W. Baseline Road, Rialto         2,600         2.5         1.3         2,600           4         R         5700 N. Riverside Avenue, Rialto (Future)         200         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0		0	0.0	0.0		6,700	6.4	2.5		
-Colton Basin           2         R         296 S. Eucalyptus Avenue, Rialto         1,500         1.4         0.8         1,500           2         R         404 S. Acacia Avenue, Rialto (Future)         1,250         1.2         0.6         1,500           3A         R         Eucalyptus Avenue, Rialto (Future)         1,800         1.7         0.9         1,800           3A         R         855 W. Baseline Road, Rialto         2,600         2.5         1.3         2,600           4         R         5700 N. Riverside Avenue, Rialto (Future)         200         0.0         0.0         0.0         0.0         200         0.0         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200         200		7,300	7.0	3.5		19,560	18.7	8.5		
2         R         296 S. Eucalyptus Avenue, Rialto         1,500         1.4         0.8         1,500           2         R         404 S. Acacia Avenue, Rialto (Future)         1,250         1.2         0.6         1,550           3         R         Eucalyptus Avenue, Rialto (Future)         1,800         1.7         0.9         1,800           3A         R         855 W. Baseline Road, Rialto (Future)         2,600         2.5         1.3         2,600           4         R         5700 N. Riverside Avenue, Rialto (Future)         200         0.2         0.0         0.0         200										
2       R       404 S. Acacia Avenue, Rialto (Future)       1,250       1.2       0.6       1,500         2       R       Eucalyptus Avenue, Rialto (Future)       1,800       1.7       0.9       1,800         3A       R       238 W. Victoria Street, Rialto       2,600       2.5       1.3       2,600         3A       R       855 W. Baseline Road, Rialto (Future)       2,600       2.5       1.3       2,600         4       R       5700 N. Riverside Avenue, Rialto (Future)       200       0.0       0.0       0.0       0.0	Avenue, Rialto	1,500	1.4	0.8		1,500	1.4	8.0	Current IX for perchlorate, Not usedpump shaft	Perchlorate, Now nitrate
2       R       Eucalyptus Avenue, Rialto (Future)       1,800       1.7       0.9       1,800         3A       R       238 W. Victoria Street, Rialto       2,600       2.5       1.3       2,600         3A       R       855 W. Baseline Road, Rialto       2,600       2.5       1.3       2,600         4       R       5700 N. Riverside Avenue, Rialto       200       0.0       0.0       0.0       200	nue, Rialto	1,250	1.2	9.0		1,250	1.2	9.0	Current IX for perchlorate, Operational	Perchlorate
3A         R         238 W. Victoria Street, Rialto         1,800         1.7         0.9         1,800           3A         R         855 W. Baseline Road, Rialto         2,600         2.5         1.3         2,600           4         R         5700 N. Riverside Avenue, Rialto (Future)         1,500         1,500           6         R         4334 N. Riverside Avenue, Rialto         200         0.2         0.0         200	e, Rialto (Future)					1,500	1.4	0.7	Not constructed	
3A         R         855 W. Baseline Road, Rialto         2,600         2.5         1.3         2,600           4         R         5700 N. Riverside Avenue, Rialto         Future)         1,500           6         R         4334 N. Riverside Avenue, Rialto         200         0.0         0.0         200	rreet, Rialto	1,800	1.7	6.0		1,800	1.7	6.0	Current perchlorate FBR, runs when Well 6 is off	
4         R         5700 N. Riverside Avenue, Rialto (Future)         1,500           6         R         4334 N. Riverside Avenue, Rialto         200         0.0         200	koad, Rialto	2,600	2.5	1.3		2,600	2.5	1.3	Not in use, FBR has no capacity, Need to add IX	Perchlorate
6 R 4334 N. Riverside Avenue, Rialto 200 0.2 0.0 200	Avenue, Rialto (Future)					1,500	1.4	0.7	Well constructed & deactivated, needs treatment	Nitrate >MCL
	Avenue, Rialto	200	0.2	0:0		200	0.2	0.0	Not regularly used. Serve as standby for zone 3	
W-24 6 R 4334 N. Riverside Avenue, Rialto 600 0.6 0.3 600 0.6	Avenue, Rialto	009	9.0	0.3		009	9.0	0.3	OK, Operational	

Table 7.6 Well Production Capacity and Water Quality Issues
Water Facilities Master Plan
West Valley Water District

				Fi	ve Year Proj	Five Year Projections, 2022		UI	Ultimate Buildout, 20	out, 2055		
Well	Zone	Basin	Location	Pump Capacity	Product. Capacity 16h/d Ops	Severe Drought Capacity	Water Cal	Pump F Capacity C 2055 16	Product. Capacity C	Severe Drought Water Capacity Demand 2055	Current Condition of Use	Water Quality Issues
				(mdg)	(mgd)	(mgd)	(afy)	(gpm)	(mgd)	(mgd) (afy)		
W-54	9	٣	Duncan Canyon Road, Fontana	1,000	1.0	9.0	T	1,000	1.0	9.0	Air in water, customer complaints, Operational	
			TOTAL RC Current	8,950	8.6	4.4	8	8,950	8.6	4.4	-	
			TOTAL RC FUTURE	0	0.0	0.0	ж -	3,000	2.8	1.4		
			IOIAL RC Basin	8,950	o. O	4.	<del>-</del>	11,950	11.4	y.0	_	
Bunker	Bunker Hill Basin						_				_	
W-15	3, 3A, 2	ВН	1915 W. 9th Street, San Bernardino	2,700	5.6	9.0	2	2,700	5.6	9.0	OK, Operational	
W-30	3, 3A, 2	ВН	2015 W. 9th Street, San Bernardino	3,100	3.0	3.0	<u>е</u>	3,100	3.0	3.0	OK, Operational	
W-43	3, 3A, 4	ВН	Along Baseline Feeder (Future)		0.0		es .	3,500	3.4	3.4	Not constructed, Options: BH or through Baseline Feeder	
W-44	3, 3A, 4	ВН	Along Baseline Feeder (Future)		0.0			3,500	3.4	3.4	Not constructed, Options: BH or through Baseline Feeder	
W-45	3, 3A, 4	ВН	Along Baseline Feeder (Future)		0.0		3	3,500	3.4	3.4	Not constructed, Options: BH or through Baseline Feeder	
W-46	3A	ВН	Along Baseline Feeder (Future)		0.0			3,500	3.4	3.4	Not constructed, Options: BH or through Baseline Feeder	
W-47	3A	ВН	Along Baseline Feeder (Future)		0.0			3,500	3.4	3.4	Not constructed, Options: BH or through Baseline Feeder	
W-48	3A	ВН	Along Baseline Feeder (Future)		0.0		<u> </u>	3,500	3.4	3.4	Not constructed, Options: BH or through Baseline Feeder	
			TOTAL BH Current	5,800	5.6	5.6	2	5,800	5.6	3.6		
			TOTAL BH FUTURE	0	0.0	0.0	2.	21,000	20.4	20.4		
			TOTAL BH Basin	2,800	5.6	5.6	76	26,800	26.0	24.0		
North R	North Riverside Basin	<b>3asin</b>					-					
W-18A	2	Z R	1783 S. Sycamore Avenue, Colton	2,700	2.6	1.3	2	2,700	2.6	1.3	Current IX perchlorate	Perchlorate, Now nitrate, Oil
W-41	7	Z K	3353 S. Industrial, Rialto	2,200	2.1	1.1		2,200	2.1	1.1	Currently off	Now perchlorate
W-42	8	Z R	295 E. San Bernardino, Rialto	2,200	2.1	1.1	2	2,200	2.1	1.1	Current IX for perchlorate. OK, Operational	Perchlorate, Now nitrate = 6ppm
W-19A	7	Z R	TBD (Future)		0.0			2,100	1.5	0.7	Not constructed	
W-29A	7	N N	180 W. Slover Avenue, Rialto (Future)		0.0		П	1,500	1.0	0.5	Not constructed	
W-38	2	Z R	TBD (Future)		0.0			1,900	1.4	0.7	Not constructed	
W-40	2	N R	157 W. Resource Drive, Rialto (Future)		0.0		1	1,500	1.0	0.5	Drilled but not equipped	
W-53	2	N R	TBD (Future)		0.0		2	2,100	1.7	6.0	Not constructed	

Table 7.6 Well Production Capacity and Water Quality Issues Water Facilities Master Plan West Valley Water District

												PRELIMINARY
				ы	ive Year Pro	Five Year Projections, 2022		Π	Ultimate Buildout, 2055	out, 2055		
Well	Zone	Basin	Location	Pump Capacity	Product. Capacity 16h/d Ops	Severe Drought Capacity	Water Demand	Pump Capacity ( 2055 1	Product. Capacity C	Severe Drought Water Capacity Demand 2055	r nd Current Condition of Use	Water Quality Issues
				(gpm)	(mgd)	(mgd)	(afy)	(gpm)	(mgd)	(mgd) (afy)		
W-51	2	NR	TBD (Future)		0.0			3,000	2.2	1.1	Not constructed	
W-52	æ	Z R	TBD (Future)		0.0			2,000	2.2	1.1	Not constructed	
W-50	3	NR	Willow Ave. and San Bernardino Ave. (Future)		0:0			1,500	1.0	0.5	Not constructed	
			TOTAL NR Current	7,100	8.9	3.4		7,100	8.9	3.4		
			TOTAL NR FUTURE	0	0.0			15,600	12.0	0.9		
			TOTAL NR Basin	7,100	8.9	3.4		22,700	18.8	9.4		
Chino Basin	asin											
W-39	8	U	10272 Cedar Place, San Bernardino Co (Future)		0.0			4,000	3.8	2.0	High levels of nitrate Drilled but not equipped	
			TOTAL C Current	0	0.0	0:0		0	0.0	0.0		
			TOTAL C FUTURE	0	0.0	0.0		4,000	3.8	2.0		
			TOTAL C Basin	0	0.0	0.0		4,000	3.8	2.0		
		T0T	TOTAL Ground Water Current	29,150	28.0	16.8	-	34,710	33.3	17.3		
		T0T,	TOTAL Ground Water FUTURE	0	0.0	0.0	-	50,300	45.4	32.3		
			TOTAL Ground Water	29,150	28.0	16.8	ŕ	85,010	78.7	49.6		

1. Table prepared by Kleinfelder, Inc staff February 2018.

2. Annual average and maximum water demand for intermediate water supply conditions by year 2022 can be satisfied by utilizing all existing wells. This assumes all currently running wells shall be operable which will requires regular and preventive maintenance. 3. To satisfy intermediate water supply demand, capital improvements by implementing wellhead treatments will be required to bring the currently constructed but not running wells in operation by 2022

4. Capacity of the current and the identified additional ground water wells has potential for production of 84.8 MGD which exceeds the average and daily maximum demands of 30.55 MG and 58.68 MGD, respectively

5. Under sever drought conditions, Baseline Feeder and/or SWP shall be utilized to provide supplemental water supply during peak day demands for intermediate condition of 2022 and for built out conditions of 2055.

6. The OPR WFF with its current capacity of 14.4 MG provides supplemental water supply to the proposed wellhead supply for the intermediate water supply conditions through 2022. The planned 6 MGD expansion shall be realized to satisfy ultimate buildout water demand

The District owns seven non-operating wells that have been inactivated due to mechanical failure of the equipment, or due to contamination such as perchlorate, nitrate, arsenic. For example, W-16, which already has an ionic exchange; wellhead treatment for perchlorates, has a malfunction of the shaft of the pump, W-8A is contaminated with arsenic; and W-33 and W-41 have perchlorate levels that exceed the current MCL. Each of these wells will require treatment or rehabilitation prior to activation.

### 7.5.6.2 Surface Water Treatment

The Roemer WFF uses raw water from Lytle Creek, and supplemental water from the SWP to treat and deliver high quality drinking water to the existing District customers. The Roemer WFF is operated up to the design capacity and, with regular and planned maintenance, is producing drinking water in compliance with current water quality standards, including TOC reduction to above regulated 35 percent.

### 7.6 STORAGE ANALYSIS

The section documents the District's existing domestic water storage capacity. Additionally, this section identifies the existing and future storage requirements to meet the storage capacity criteria by pressure zone.

### 7.6.1 Storage Requirements

The following sections summarize the storage requirements under existing, 5-year, and buildout development conditions. The storage requirements for each development condition are calculated based on criteria discussed in the System Performance and Design Criteria chapter and are summarized on Table 7.7.

### 7.6.1.1 Existing Development

Existing storage requirements were identified for each pressure zone and are summarized in Table 7.7. The table lists the existing domestic water demands and operational, pumping, and fire storage for each pressure zone. As summarized on this table the total required storage for existing domestic water demands is 51.8 MG. The current usable storage capacity is 71.86 MG. There are two inactive reservoirs: R6-1 (0.25 MG) and R2-2 (0.5 MG). Reservoir R2-2 is tar lined and R6-6 needs interior recoating. The cost to rehabilitate these two older reservoirs is quite substantial compared to their limited storage capacity.

### 7.6.1.2 5-Year Development

The storage requirements due to 5-year development were identified based on the planned five year growth and are summarized by pressure zone on **Table 7.7**. The table lists the additional domestic water demands due to 5-year development and identifies the operational, pumping, and fire storage for each pressure zone. As summarized on this table the total required storage for

**Table 7.7 Storage Requirements** 

Water Facilities Master Plan West Valley Water District

**PRELIMINARY** 

						PRELIMINARY
	Water De	emands	V	Vater Storage	e Requireme	nts
Pressure Zone	Average Day Demand <sup>1</sup>	Peak Day Demand <sup>2</sup>	Operational at 100%	Fire Protection <sup>3</sup>	Pumping Storage <sup>4,5</sup>	Total, By Pressure Zone
	(mgd)	(mgd)	(MG)	(MG)	(MG)	(MG)
<b>Existing Stora</b>	age Require	ments				
South System Pres	sure Zones					
2	2.65	4.51	4.51	0.96	-	5.47
3	3.87	6.57	6.57	0.96	-	7.53
3A	1.04	1.77	1.77	0.54	-	2.31
Subtotal	7.56	12.85	12.85	2.46	0.00	15.31
North System Pres	sure Zones					
4	1.96	3.32	3.32	0.54	7.85	11.72
5	1.98	3.36	3.36	0.54	5.87	9.78
6	3.18	5.40	5.40	0.96	2.70	9.06
7	2.46	4.18	4.18	0.54	0.24	4.96
8	0.24	0.41	0.41	0.54	-	0.95
Subtotal	9.81	16.67	16.67	3.12	16.66	36.46
<b>Existing Storage Re</b>	quirements					
	17.37	29.52	29.52	5.58	16.66	51.77
<b>New Storage</b>	Requireme	nts (Near-	-Term 5-Ye	ear Plann	ing)	
South System Pres	sure Zones					
2	0.13	0.22	0.22	0.96	-	1.18
3	0.27	0.45	0.45	0.96	-	1.41
3A	0.03	0.06	0.06	0.54	-	0.60
Subtotal	0.43	0.73	0.73	2.46	0.00	3.19
North System Pres	sure Zones					
4	0.04	0.07	0.07	0.54	3.69	4.30
5	0.66	1.12	1.12	0.54	3.03	4.69
6	1.19	2.02	2.02	0.96	1.84	4.83
7	1.59	2.70	2.70	0.54	0.26	3.49
8	0.26	0.44	0.44	0.54		0.98
Subtotal	3.73	6.34	6.34	3.12	8.82	18.29
New Storage Requ	irements					
	4.16	7.07	7.07	5.58	8.82	21.48

**Table 7.7 Storage Requirements** 

Water Facilities Master Plan West Valley Water District

**PRELIMINARY** 

	Water De	emands	V	Vater Storage	e Requireme	nts
Pressure Zone	Average Day Demand <sup>1</sup>	Peak Day Demand <sup>2</sup>	Operational at 100%	Fire Protection <sup>3</sup>	Pumping Storage <sup>4,5</sup>	Total, By Pressure Zone
	(mgd)	(mgd)	(MG)	(MG)	(MG)	(MG)
<b>New Storage</b>	Requireme	nts (Year 6	through	<b>Buildout</b>	Planning	3)
South System Pres	sure Zones					
2	1.77	3.00	3.00	0.96	-	3.96
3	2.50	4.26	4.26	0.96	_	5.22
3A	0.04	0.07	0.07	0.54	_	0.61
Subtotal	4.31	7.32	7.32	2.46	0.00	9.78
North System Pres	sure Zones					
4	0.27	0.46	0.46	0.54	5.51	6.51
5	0.19	0.33	0.33	0.54	5.31	6.18
6	2.44	4.16	4.16	0.96	2.87	7.98
7	2.47	4.19	4.19	0.54	0.40	5.14
8	0.40	0.68	0.68	0.54	_	1.22
Subtotal	5.78	9.83	9.83	3.12	14.09	27.04
New Storage Requi	irements					
The state of the s	10.09	17.15	17.15	5.58	14.09	36.82
Taral Channe						55.62
Total Storag	ge Kequire	ments at	Bullaout			
South System Pres	sure Zones					
2	4.55	7.74	7.74	0.96	-	8.70
3	6.63	11.28	11.28	0.96	-	12.24
3A	1.11	1.89	1.89	0.54	-	2.43
Subtotal	12.30	20.90	20.90	2.46	0.00	23.36
North System Pres	sure Zones 2.27	3.85	3.85	0.54	17.05	17.44
5	2.27	3.85 4.81	3.85 4.81	0.54	17.05	17.44 15.57
6	6.81	11.58	11.58	0.96	7.41	19.95
7	6.51	11.07	11.07	0.54	0.90	12.51
8	0.90	1.53	1.53	0.54	-	2.07
Subtotal	19.32	32.84	32.84	3.12	39.58	67.54
Total Storage Requ	irements					
AKEL	31.62	53.75	53.75	5.58	39.58	90.91

Notes: 2/6/2019

<sup>1.</sup> Existing average day demands based on 2014 production less 10%, where the demand distribution by pressure zone is based on 2016 water billing records.

<sup>2.</sup> Peak Day Demand = 1.7 x Average Day Demand

<sup>3.</sup> Fire Protection requirement represents largest fire requirement for each zone, based on account types listed in water billing records

<sup>4.</sup> Zones 4-7 include a pumping storage capacity which is equal to 1-day storage of ADD for the higher zones.

<sup>5.</sup> The pumping storage shown in this table is the maximum pumping storage required and does not take into account the 4.0 MG of pumping storage available at the OPR WFF during emergency conditions.

5-year domestic water demands is 21.5 MG, which excludes the demands due to existing development.

### 7.6.1.3 Buildout Development Storage Requirements

The storage requirements due to buildout development of the District service area are summarized by pressure zone on Table 7.7. The table lists the additional domestic water demands due to buildout development and identifies the operational, pumping, and fire storage for each pressure zone. The table also lists the total required storage for buildout domestic water demands at 36.8 MG, which excludes the demands due to existing and 5-year development.

### 7.6.2 Storage Analysis and Recommended New Storage Facilities

The existing and future storage requirements, shown on Table 7.7, were compared with existing District storage facilities in each pressure zone and the required storage facility improvements for the 5-year (Table 7.8) and Buildout (Table 7.9) development horizons were identified; these tables list existing storage facilities for each zone, identifies existing storage capacity deficiencies, and identifies future storage capacity requirements to meet the needs from future growth.

### 7.6.2.1 5-year Development Storage Analysis

Based on the storage analysis shown on Table 7.8, the majority of the existing pressures zones have sufficient storage capacity to meet existing and five-five year requirements. The storage improvements recommended for construction within the five-year development horizon include the replacement of the existing Pressure Zone 8 storage reservoirs and the construction of a planned aeration reservoir, which are briefly summarized as follows:

**Pressure Zone 8:** In order to meet the storage capacity requirements due to the 5-year development within this pressure zone, an additional 0.5 MG of storage capacity is required. However, in order provide additional capacity for buildout development within the pressure zone a total capacity of 2.1 MG is recommended, which will provide surplus storage capacity to meet growing storage requirements as development continues beyond the 5-year development planning horizon. This storage volume also accounts for the demolition of the existing Zone 8 storage reservoirs.

Lord Ranch Facility: The current designs for the Lord Ranch Facility include the
construction of one new aeration reservoir. This reservoir is not intended to float on the
District's distribution system and will serve as a forebay to the Lord Ranch Facility pump
station expansion.

The proposed storage reservoir improvements for the 5-year development horizon are included on Table 7.10 and graphically shown on Figure 7.3, and described as follows:

• **Z8-R8-3**: Replace the existing 0.10 MG and 0.41 MG Zone 8 water storage reservoirs with a 2.1 MG storage reservoir at the existing Zone 8 Tank site.

Table 7.8 Storage Capacity Analysis - 5 Year Growth

Water Facilities Master Plan

West Valley Water District

State   Stat			Dem	Demands		Operational + Emergency Storage		Pumping Storage <sup>1,2</sup>	orage <sup>1,2</sup>	l			Exi	Existing Storage Reservoirs	orage l	Reserv	oirs		anitsix=		Prop	Proposed New Storage Reservoirs	lew Sto	orage R	leservo	irs			gni1six∃
1   1   1   1   1   1   1   1   1   1	Pressure Zone					Bnitsix3	5-Year Growth	gnitsix3	5-Year Growth	roitoeford Protection	A bne gnitzixal lstoT məriupəЯ əgerot2					9 əuoz	∑ 9uoZ	8 əuoZ		Spuemag			Z ənoZ	9 9no∑	√ 9no∑	<sup>9</sup> 8 9noZ		Total Storage	Storage Balance for I
Step 1.5																													

Notes: 1. Total Required Storage for Pressure Zone 2, 3, 3A, 8 : Operational + Fire

<sup>2.</sup> Total Required Storage for Pressure Zone 4, 5, 6, 7: Operational + Fire + Pumping Storage

<sup>3.</sup> Pumping Storage defined as 100% Average Day Demand (ADD) for supply dependent pumping zone.

<sup>4.</sup> The pumping storage shown in this column is the maximum pumping storage required and does not take into account the 4.0 MG of pumping storage available and the OPR WFF.

5. The total pumping requirement for Zone 4 and Zone 5 reflects a 4.0 MG reduction in pumping storage due to supply available at the OPR WFF under emergency operational conditions.

<sup>6.</sup> Proposed new Zone 8 storage tank volume based on buildout land use demand requirements, which exceed the storage requirements due to 5 year growth.

Table 7.9 Storage Capacity Analysis - Buildout

Water Facilities Master Plan West Valley Water District

	f sonsige Balance f Tuobliud bns	(MG)		2.30	0.01	3.57	5.89		0.56	0.03	0.05	0.04	0.03	1.22	7.10
əBt	stot Stot	(MG)		11.00	12.25	00.9	29.25		18.00	15.60	20.00	12.55	2.10	68.76	98.01
	lstoT	(MG)		00.00	3.25	0.00	3.25	-	7.00	2.60	9.00	3.40	2.10	24.10	27.35
	8 ənoZ	(MG)											2.10		
ervoirs	∑ əuo∑	(MG)										3.40			
age Res	9 əuoz	(MG)									9.00				
Proposed New Storage Reservoirs	ζ əuo∑	(MG)								2.60					
osed Ne	₱ əuoZ	(MG)							7.00						
Prop	A£ ənoZ	(MG)			ы										
	£ ənoZ	(MG)			3.25										
CILIGIINO	Z 9uoZ	(MG)	. <u>-</u>		4		-	_	4			<u> </u>	<u> </u>	<u>∞</u>	īυ.
	f spieled Balance f	(MG)	. <u>-</u>	) 2.30	-3.24	3.57	0 2.64		) -6.44	) -2.57	.8.95	-3.36	-1.56	5 -22.88	5 -20.25
	lstoT	(MG)	. <u>-</u>	11.00	9.00	6.00	26.00	-	11.00	13.00	11.00	9.15	0.51	44.66	70.66
	8 9noZ	G) (MG)										15	0.51		
rvoirs	9 əno∑ √ əno∑	(MG) (MG)									11.00	9.15			
e Rese	S ∍noZ	(MG)								13.00	11				
Storag	₽ əuoZ	(MG)							11.00	13					
Existing Storage Reservoirs	A& ənoZ	(MG)				00.9			11						
Ë	£ əuoZ	I) (EMG)			9.00	Ø									
	Z əuoZ	) (MG)		11.00	6										
	Total Existing an	(MG)		8.70	12.24	2.43	23.36		17.44	15.57	19.95	12.51	2.07	67.54	90.91
<sup>ε</sup> noi:	Fire Protect	(MG)		96.0	96.0	0.54	2.46		0.54	0.54	96.0	0.54	0.54	3.12	5.58
Pumping Storage <sup>1,2</sup>	Future	(MG)		0.00	0.00	0.00	0.00		9.20	8.35	4.71	99.0	0.00	22.92	22.92
Pun	Bnistix∃	(MG)		0.00	0.00	0.00	0.00		7.85	5.87	2.70	0.24	0.00	16.66	16.66
onal + ency age	Future	(MG)		3.23	4.71	0.12	8.06		0.53	1.45	6.18	68.9	1.12	16.17	24.22
Operational + Emergency Storage	gnitsix3	(MG)		4.51	6.57	1.77	12.85		3.32	3.36	5.40	4.18	0.41	16.67	29.52
	Future Peak Day Demand	(MGD)		3.23	4.71	0.12	8.06		0.53	1.45	6.18	6.89	1.12	16.17	24.22
spu	Future Average Day Demand	(MGD)		1.90	2.77	0.07	4.74		0.31	0.85	3.63	4.05	99.0	9.51	14.25
Demands	Existing Peak Day Demand	(MGD)		4.51	6.57	1.77	12.85		3.32	3.36	5.40	4.18	0.41	16.67	29.52
	egerəvA gnitzix∃ Dagməd yed	(MGD)		2.65	3.87	1.04	7.56		1.96	1.98	3.18	2.46	0.24	9.81	17.37
	Pressure Zone		South System	Pressure Zone 2	Pressure Zone 3	Pressure Zone 3A	Subtotal	North System	Pressure Zone 4	Pressure Zone 5	Pressure Zone 6	Pressure Zone 7	Pressure Zone 8	Subtotal	Total

Notes:

1. Pumping Storage defined as 100% Average Day Demand (ADD) for supply dependent pumping zone.

<sup>2.</sup> The pumping storage shown in this column is the maximum pumping storage required and does not take into account the 4.0 MG of pumping storage available and the OPR WFF.

<sup>3.</sup> Fire storage requirement is the greatest fire flow volume of existing and future customers for each pressure zone.

<sup>4.</sup> The total pumping requirement for Zone 4 and Zone 5 reflects a 4.0 MG reduction in pumping storage due to supply available at the OPR WFF under emergency operational conditions.

### **Table 7.10 Proposed Storage Reservoirs**

### Water Facilities Master Plan West Valley Water District

**PRELIMINARY** 

Tank ID	Pressure Zone	<b>Volume</b> (MG)	Bottom Elevation (ft)
R3-4	3	3.25	1,260
LR-R3-5	3	1.00	1,156
R4-4	4	7.00	1,500
R5-4	5	2.60	1,638
R6-5	6	6.00	1,860
R6-6	6	3.00	1,860
R7-5	7	3.40	2,120
R8-3	8	2.10	2,363
R-BH-AER	-	1.00	2,345
Total		29.35	
ENGINEERING GROUP, INC.			1/11/2019

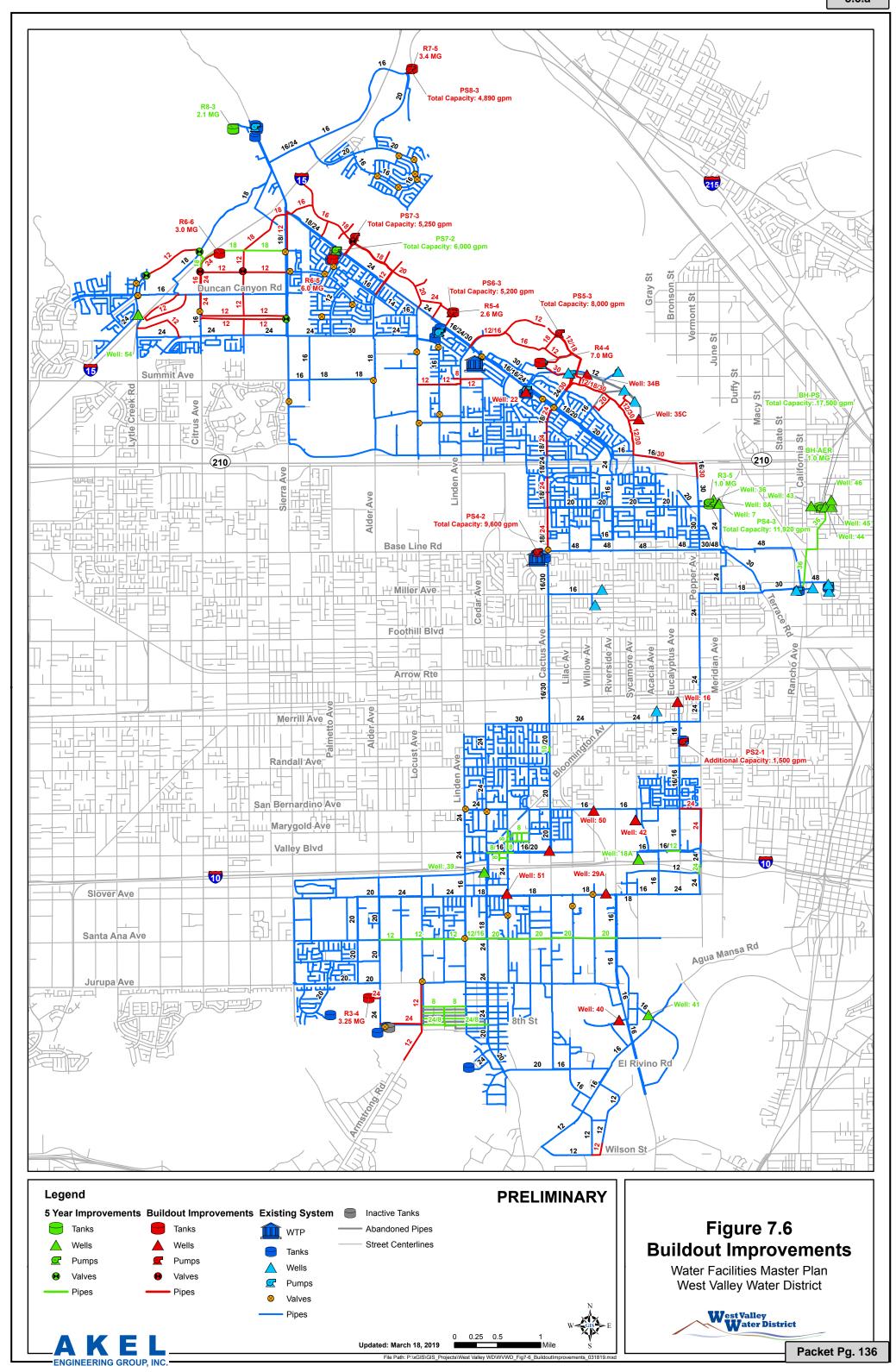
• LR-R3-5: Construct a new 1.0 MG water storage reservoir at the existing Lord Ranch Facility.

### 7.6.2.2 Buildout Development Storage Analysis

Based on the storage analysis shown on **Table 7.9**, the existing storage capacity of multiple pressure zones is unable to meet the storage requirements at buildout of the District service area. In order to mitigate these storage deficiencies multiple storage reservoirs are recommended, as summarized on **Table 7.10** and shown graphically on **Figure 7.6**.

These storage deficiencies and recommended improvements are also briefly summarized below:

- **Pressure Zone 2:** Pressure Zone 2 is expected to have surplus storage capacity at buildout demands, and no improvements are recommended.
- Pressure Zone 3: In order to meet the storage capacity requirements at the buildout of the
  District service area, an additional 3.25 MG of storage capacity is recommended. This
  additional capacity is planned to be provided by the construction of one new storage
  reservoir.
- **Pressure Zone 3A:** Pressure Zone 3A is expected to have surplus storage capacity at buildout demands and no improvements are recommended.
- Pressure Zone 4: In order to meet the storage capacity requirements at the buildout of the
  District service area, an additional 7.0 MG of storage capacity is recommended. This
  additional capacity is planned to be provided by the construction of one new storage
  reservoir.
- Pressure Zone 5: In order to meet the storage capacity requirements at the buildout of the
  District service area, an additional 2.6 MG of storage capacity is recommended. This
  additional capacity is planned to be provided by the construction of one new storage
  reservoir.
- Pressure Zone 6: In order to meet the storage capacity requirements at the buildout of the
  District service area an additional 9.0 MG of storage capacity is recommended. This
  additional capacity is planned to be provided by the construction of two new storage
  reservoirs.
- Pressure Zone 7: In order to meet the storage capacity requirements at the buildout of the
  District service area, an additional 3.4 MG of storage capacity is recommended. This
  additional capacity is planned to be provided by the construction of one new storage
  reservoir.
- Pressure Zone 8: As described in a previous section, the existing Zone 8 storage



reservoirs are planned for replacement as part of the 5-year planning horizon. The recommended tank volume is sized to meet the buildout storage need.

• **Bunker Hill Well Field:** Plans for the Bunker Hill supply include the construction of a 1.0 MG aeration tank, which will serve as an equalization reservoir for the discharge of planned groundwater wells 43, 44, 45, and 46.

The proposed storage reservoirs summarized on Table 7.10 are briefly described as follows:

- **Z3-R3-4**: Construct a new 3.25 MG storage reservoir approximately 1,100 ft southwest of the intersection of Jurupa Avenue and Alder Avenue.
- **Z4-R4-4**: Construct a new 7.0 MG storage reservoir at the existing water storage reservoir 4-3 site.
- Z5-R5-4: Construct a new 2.6 MG storage reservoir within the planned Lytle Creek Ranch development approximately 1,000 feet northeast of the existing water storage reservoir 5-1 site.
- **Z6-R6-5**: Construct a new 6.0 MG storage reservoir at the existing water storage reservoir 6-2 site.
- **Z6-R6-6**: Construct a new 3.0 MG storage reservoir approximately 1,100 feet east of the intersection of Citrus Avenue and Segovia Lane.
- **Z7-R7-5**: Construct a new 3.4 MG storage reservoir at the intersection of Clearwater Parkway and Glen Helen Parkway.

### 7.7 PUMP STATION CAPACITY ANALYSIS

The section documents the existing pump station capacity, as well as the requirements to meet existing and future pumping needs. The pump station capacity evaluation is consolidated by pressure zone, and improvements are documented where necessary.

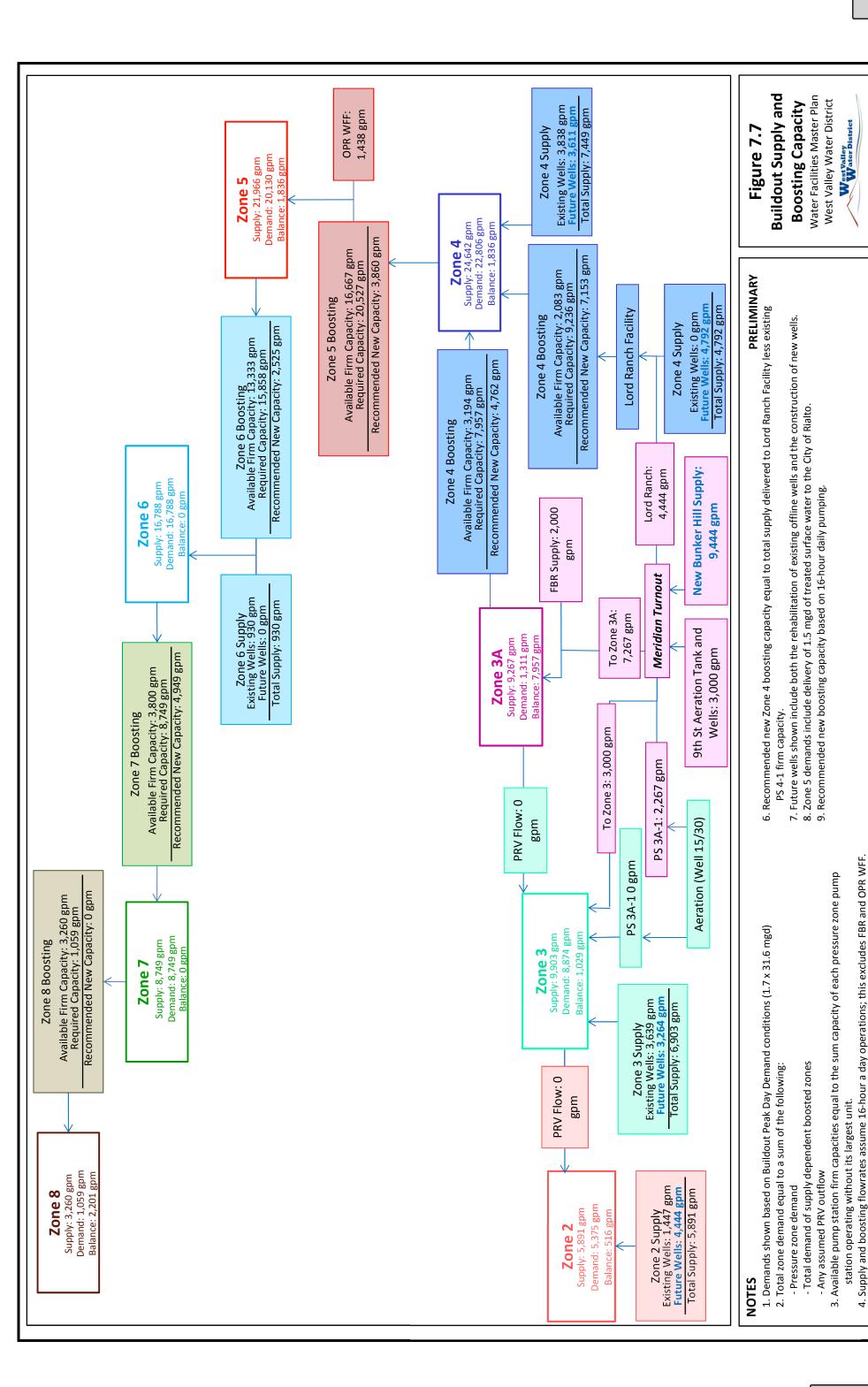
### 7.7.1 Existing Pump Station Capacity Requirements

The existing pump station requirements were identified for each station and are summarized on Table 7.11. The table lists the existing pump station capacities and identifies the required capacity, based on the District criteria. The existing pump station capacity analysis indicates the District's current pump stations have adequate capacity to service existing customers.

### 7.7.2 Future Pump Station Capacity Requirements

Future pump station requirements were identified for each pressure zone and are summarized on Figure 7.7. Based on the pump station criteria discussed in the System Performance and Design Criteria chapter, the combined firm capacity of each zone pump station is required to meet the Peak Day Demands of each zone in addition to any supply dependent zones. Pump station capacity requirements will vary based on supply scenarios discussed in an earlier section.

March 11, 2019



5. OPR WFF treating minimum reliable Lytle Creek supply of 4,000 AFY.

**Table 7.11 Existing Pump Station Analysis**Water Facilities Master Plan
West Valley Water District

	Pressu	Pressure Zone		Pressur	Pressure Zone Demands	lands			Pump St	Pump Station Capacity Analysis	v Analysis	PRELIMINARY
Pump Station Name	Source	Destination	<b>Destination</b> <b>Zone</b>	Supply Dependent Zones	Avera Destination Zone	Average Day Demand Supply ion Dependent	and Total	Total Capacity <sup>1</sup>	Total Firm Capacity <sup>1</sup> Capacity <sup>2</sup>	Required Capacity <sup>3</sup>	Credit for Firm Supply <sup>4</sup>	Surplus/ Deficiency
			(mdg)			(gpm)						(mdg)
South System												
Pressure Zone 2												
Wells (W18A)		Э						1,447	0			
Total			ε		1,291	0	1,291	1,447	0	2,195	0	-2,195
Pressure Zone 3												
2-1 PS	Wells 16, 17	3						1,000	0			
3A-1 PS	Wells 15, 30	3						1,933	1,933			
9th Street PS		3, 4						4,000	1,000			
Wells (W42)	,	3						1,447	0			
Total			ε	2	1,903	1,291	3,194	8,380	2,933	5,429	0	-2,496
Pressure Zone 3A												
3A-1 PS	Wells 15, 30	3, 3A						0	0			
Total	wells 0, 11		34		602	c	602	2,000	2,000	1 206	c	794
North System			5		2		8	20047	200,47	004/1		
Pressure Zone 4												
4-1 PS	m	4	4					3,400	2,067			
4-2 PS	3A	4	4					4,800	3,200			
Total			4		1,273	3,733	900'5	8,200	5,267	8,511	0	-3,244
Pressure Zone 5												
5-1 PS	4	5						8,000	000′9			
5-2 PS	4	2						12,800	10,667			
Oliver P. Roemer WFF Effluent Pumps											2,484	
Total			2	6, 7, 8	1,313	2,420	3,733	20,800	16,667	6,346	2,484	12,804

Table 7.11 Existing Pump Station Analysis Water Facilities Master Plan

West Valley Water District

	Press	Pressure Zone		Pressu	Pressure Zone Demands	nands			Pump St	Pump Station Capacity Analysis	ty Analysis	sis
Pump Station Name	Source	Source Destination	<b>Destination</b> <b>Zone</b>	Supply Dependent Zones	Aver Destination Zone	Average Day Demand Supply Dependent Zones	and Total	Total Capacity <sup>1</sup>	Total Firm Capacity <sup>1</sup> Capacity <sup>2</sup>	Required Capacity <sup>3</sup>	Credit for Firm Supply <sup>4</sup>	Surplus/ Deficiency
			(mdg)	(mdB)	(mdg)	(mdB)	(gpm)	(mdg)	(mdg)	(mdg)	(mdg)	(mdg)
Pressure Zone 6												
6-1 PS	2	9						6,200	4,733			
6-2 PS	2	9						10,360	8,633			
Wells (W24, W54)											317	
Total			9	7, 8	1,443	978	2,420	16,560	13,367	4,115	317	695'6
Pressure Zone 7												
7-1 PS	9	7						5,267	3,800			
Total			7	8	938	40	878	5,267	3,800	1,662		2,138
Pressure Zone 8												
8-2 PS	7	8						4,375	3,260	0		
Total			8	ı	40	0	40	4,375	3,260	89	0	3,192
AKEL												4/2/2018

Supply Scenario 2 represents the most conservative pump station capacity requirements and improvements recommended are consistent with this scenario. The proposed pump stations are briefly described by pressure zone in the following sections.

**Pressure Zone 2:** This pressure zone has no existing pump stations and the existing wells, in addition to the planned future wells, will provide sufficient supply capacity to meet the peak day demands of the zone.

**Pressure Zone 3:** The pump station capacity requirements for this zone are supplied by Pump Station 3A, Pump Station 2-1, and the 9<sup>th</sup> Street Pump Station through the Meridian Turnout. This zone has no supply dependent demands and a portion of the zone demands are provided by existing and planned future wells. Based on the firm capacity of the existing pump stations, this pressure zone has pumping capacity to meet the peak day demand requirements. However, in order to create firm capacity at the existing Pump Station 2-1, one new pump is recommended.

• **Z3-PS2-1:** Construct one additional 1,500 gpm pump at the existing Pump Station 2-1 site. This will increase the total station capacity to 3,000 gpm and create a firm capacity of 1,500 gpm.

**Pressure Zone 3A:** The pump station capacity requirements for this zone are supplied by pump station 3A and the 9<sup>th</sup> Street Pump Station through the Meridian Turnout. This zone has no supply dependent demands and a portion of the zone demands are provided by the FBR and existing and planned future wells. Based on the firm capacity of the existing pump stations, this zone has a pump station capacity surplus and no improvements are recommended.

**Pressure Zone 4:** The pump station capacity requirements for this zone are supplied by Pump Station 4-1 and Pump Station 4-2. In addition to meeting the peak day demands for Pressure Zone 4, these pump stations must also provide water to Pressure Zones 5, 6, 7 and 8, which are supply dependent pressure zones. Based on these requirements approximately 16,000 gpm of additional pump station capacity is recommended in this zone, which is planned to be met through the construction of two new pump stations.

- **Z4-PS4-2**: Construct a new pump station at the existing Pump Station 4-2 site. This pump station is planned to have four 2,400 gpm pumps, three duty and one standby, for a total station capacity of 9,600 gpm. It should be noted that if space is available the recommended pumps could be incorporated into the existing Pump Station 4-2.
- **Z4-PS4-3**: Construct a new pump station at the existing Lord Ranch Facility. This pump station is planned to have four 2,980 gpm pumps, three duty and one standby.. This pump station is planned to discharge into the existing 30-inch transmission main in Pepper Avenue north to Highland Avenue.

**Pressure Zone 5:** The pump station capacity requirements for this zone are supplied by Pump Station 5-1 and Pump Station 5-2. In addition to meeting the peak day demands for Pressure

Zone 5 these pump stations must also provide water to Pressure Zone 6, 7, and 8, which are supply dependent pressure zones. A portion of these demands will be met by surface water treatment at Roemer WFF. Based on the existing pumping capacity and planned supply capacity at the Roemer WFF, approximately 6,000 gpm of additional pump station capacity is recommended in this zone, which is planned to be met through the construction of one new pump station.

• **Z5-PS5-3**: Construct a new pump station within the planned Lytle Creek Ranch development approximately 2,200 feet northeast of the existing water storage reservoir 4-3 site. This pump station is planned to have four 2,000 gpm pumps, three duty and one standby, for a total station capacity of 8,000 gpm.

Pressure Zone 6: The pump station capacity requirements for this zone are supplied by Pump Station 6-1 and Pump Station 6-2. In addition to meeting the peak day demands for Pressure Zone 6, these pump stations must also provide water to Pressure Zone 7 and 8, which are supply dependent pressure zones. A portion of these demands are offset by an existing groundwater well. Based on these requirements approximately 3,900 gpm of additional pump station capacity is recommended in this zone, which is planned to be met through the construction of one new pump station.

• **Z6-PS6-3:** Construct a new pump station within the planned Lytle Creek Ranch development approximately 1,000 feet northeast of the existing water storage reservoir 5-1 site. This pump station is planned to have four 1,300 gpm pumps, three duty and one standby, for a total station capacity of 4,200 gpm.

**Pressure Zone 7:** The pump station capacity requirements for this zone are supplied by Pump Station 7-1. In addition to meeting the peak day demands for Pressure Zone 7, this pump station must also provide water to Pressure Zone 8, which is a supply dependent pressure zone. Based on these requirements approximately 7,500 gpm of additional pump station capacity is recommended in this zone, which is planned to be met through the construction of two new pump stations.

- **Z7-PS7-2:** Construct a new pump station at the existing Pump Station 7-1 location. This pump station is planned to have three 2,000 gpm pumps, two duty and one standby, for a total station capacity of 6,000 gpm.
- **Z7-PS7-3**: Construct a new pump station within the planned Lytle Creek Ranch development approximately 1,500 feet northeast of the existing water storage reservoir 6-2 site. This pump station is planned to have three 1,750 gpm pumps, two duty and one standby, for a total station capacity of 5,250 gpm.

**Pressure Zone 8:** The pump station capacity requirements for this zone are provided by Pump Station 8-2. The existing pump station is capable of meeting the buildout peak day demands.

However, in order to create hydraulic reliability in this zone one new pump station is recommended with a capacity equal to the existing Pump Station 8-1.

• **Z8-PS8-3:** Construct a new pump station at the intersection of Clearwater Parkway and Glen Helen Parkway. This pump station is planned to have three 1,630 gpm pumps, two duty and one standby, for a total station capacity of 4,890 gpm.

**Bunker Hill Well Field:** The new Bunker Hill wellfield, comprised of future Wells 43, 44, 45, and 46 as discussed in a previous section, will require a new pump station to transfer the extracted groundwater from the planned aeration tank to the existing 30-inch transmission main at the existing pump station 3A-1 site. This pump station is planned to have a firm capacity of 14,000 gpm, which is equal to the sum of the planned design capacities of the recommended Bunker Hill supply wells.

• **BH-PS**: Construct a new pump station with five 3,500 gpm pumps, four duty and one standby, for a total station capacity of 17,500 gpm.

### 7.8 PIPELINE IMPROVEMENTS TO SERVE FUTURE GROWTH

The buildout of the District's service area includes development outside of the extent of the existing domestic water distribution system. New pipelines are recommended to serve future growth as well as increase the hydraulic reliability of the domestic water distribution system. Each pipeline improvement is assigned a uniquely coded identifier, which is intended to aid in defining the location of the improvements for mapping purposes. These identifiers reflect the pressure zone and sequence in the improvement schedule. The pipeline improvements are described in detail on the following pages.

### 7.8.1 Pressure Zone 2

The following section documents pipeline improvements within Pressure Zone 2.

- **Z2-P1:** Construct new parallel 24-inch and 8-inch pipelines in Eighth Street from Locust Avenue to Cedar Avenue.
- **Z2-P2:** Construct a new 8-inch pipeline in Eighth Street from Locust Avenue to Linden Avenue.
- **Z2-P3**: Replace an existing 4-inch and 6-inch pipeline in Ninth Street from Locust Avenue to Linden Avenue with a new 8-inch pipeline.
- **Z2-P4:** Replace an existing 6-inch and 8-inch pipeline in Tenth Street from Locust Avenue to Linden Avenue with a new 8-inch pipeline.
- **Z2-P5**: Construct a new 8-inch pipeline in Eleventh Street from Locust Avenue to Linden Avenue.

- **Z2-P6**: Replace an existing 6-inch pipeline in Maple Street from Eleventh Street to Eighth Street with a new 12-inch pipeline.
- **Z2-P7**: Construct a new 12-inch pipeline in Santa Ana Avenue from Linden Avenue to Cedar Avenue.
- **Z2-P8**: Replace an existing 12-inch pipeline in Santa Ana Avenue with a new 20-inch pipeline from Cedar Avenue to Riverside Avenue.
- **Z2-P9**: Construct a new 24-inch pipeline in Pepper Avenue from approximately 1,200 ft north of Slover Avenue to approximately 300 ft south of I-10. This pipeline includes a casing to cross beneath the South Pacific Railway.
- **Z2-P10:** Construct a new 24-inch pipeline in Pepper Avenue and San Bernardino Avenue from approximately 400 ft north of the intersection of Valley Boulevard and Pepper Avenue to the intersection of San Bernardino Avenue and Eucalyptus Avenue.

### 7.8.2 Pressure Zone 3

The following section documents pipeline improvements within Pressure Zone 3.

- **Z3-P1**: Construct a new 24-inch pipeline in future right-of-way from planned reservoir 3-4 to Alder Avenue.
- **Z3-P2**: Construct a new 24-inch pipeline in future right-of-way from Alder Avenue to Locust Avenue.
- **Z3-P3**: Construct a new 12-inch pipeline in Locust Avenue and Armstrong Road from Jurupa Avenue to approximately 2,200 ft southwest of Eighth Street.
- **Z3-P4:** Replace existing 4-inch, 6-inch, and 12-inch pipelines in Santa Ana Avenue with a new 12-inch pipeline from Alder Avenue to Linden Avenue.
- **Z3-P5**: Construct a new 16-inch pipeline in Santa Ana Avenue from Linden Avenue to Cedar Avenue.
- **Z3-P6**: Replace existing 2-inch, 4-inch, and 6-inch pipelines in and north of Valley Boulevard generally between Cedar Avenue and larch Avenue.
- **Z3-P7**: Replace existing 4-inch and 6-inch pipelines north of Valley Boulevard generally between Olive Street and Spruce Avenue.
- **Z3-P8:** Construct a new 16-inch pipeline in Valley Boulevard from approximately 850 ft west of Eucalyptus Avenue to Eucalyptus Avenue.

### 7.8.3 Pressure Zone 3A

The following section documents pipeline improvements within Pressure Zone 3A.

• **Z3A-P1**: Construct a new 10-inch pipeline in Cactus Avenue from James Street to Alru Street.

### 7.8.4 Pressure Zone 4

The following section documents pipeline improvements within Pressure Zone 4.

- **Z4-P1**: Construct a parallel 24-inch pipeline in Cactus Avenue from Baseline Road to I-210.
- **Z4-P2**: Construct a parallel 24-inch pipeline in Cactus Avenue from Casmalia Street to Riverside Avenue.
- **Z4-P3:** Construct a parallel 30-inch pipeline in Pepper Avenue, Highland Avenue, Oakdale Avenue, and in the planned Lytle Creek Ranch development from the existing Lord Ranch facility to reservoir 4-3.
- **Z4-P4**: Construct a new 12-inch pipeline in the Planned Lytle Creek Ranch development to Well 35C.
- **Z4-P5**: Construct a new 12-inch pipeline in the Planned Lytle Creek Ranch development to Well 5A.
- **Z4-P6**: Construct a new 12-inch pipeline in the Planned Lytle Creek Ranch development to Well 4A.
- Z4-P7: Construct a new 18-inch pipeline in the Planned Lytle Creek Ranch development.
- **Z4-P8**: Construct a new 20-inch pipeline in Future ROW from Sycamore Avenue to Planned Lytle Creek Ranch development.
- **Z4-P9**: Construct a new 12-inch pipeline in the Planned Lytle Creek Ranch development.
- **Z4-P10**: Construct a new 12-inch pipeline in the Planned Lytle Creek Ranch development to Well 34B.
- **Z4-P11**: Construct a new 24-inch pipeline in the Planned Lytle Creek Ranch development.
- **Z4-P12:** Construct a new 12-inch pipeline in the Planned Lytle Creek Ranch development.
- Z4-P13: Construct a new 24-inch pipeline in the Planned Lytle Creek Ranch development.
- **Z4-P14**: Construct a new 24-inch pipeline in the planned Lytle Creek Ranch development.

- **Z4-P15**: Construct a new 18-inch pipeline in the planned Lytle Creek Ranch development.
- **Z4-P16**: Construct a new 24-inch pipeline in the planned Lytle Creek Ranch development.

### 7.8.5 Pressure Zone 5

The following section documents pipeline improvements within Pressure Zone 5.

- **Z5-P1**: Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- Z5-P2: Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- Z5-P3: Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- **Z5-P4**: Construct a new 18-inch pipeline in the planned Lytle Creek Ranch development.
- **Z5-P5**: Construct a new 16-inch pipeline in the planned Lytle Creek Ranch development.
- Z5-P6: Construct a new 24-inch pipeline in the planned Lytle Creek Ranch development.

### 7.8.6 Pressure Zone 6

The following section documents pipeline improvements within Pressure Zone 6.

- **Z6-P1**: Construct a new 12-inch pipeline in Persimmon Street and Summit Avenue generally between Locust Avenue and Cedar Avenue
- **Z6-P2**: Replace existing 4-inch and 6-inch pipelines in Persimmon Street and Summit Avenue with a new 8-inch pipeline generally between Locust Avenue and Cedar Avenue.
- Z6-P3: Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- **Z6-P4:** Construct a new 24-inch pipeline in the planned Lytle Creek Ranch development.
- Z6-P5: Construct a new 20-inch pipeline in the planned Lytle Creek Ranch development.
- Z6-P6: Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- Z6-P7: Construct a new 16-inch pipeline in the planned Lytle Creek Ranch development.
- Z6-P8: Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- Z6-P9: Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- **Z6-P10**: Construct a new 18-inch pipeline in the planned Lytle Creek Ranch development.
- **Z6-P11**: Construct a new 24-inch pipeline in the planned Lytle Creek Ranch development.

- **Z6-P12**: Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- **Z6-P13**: Construct a new 12-inch pipeline in Sunrise Drive from Sierra Avenue to Citrus Avenue.
- **Z6-P14**: Construct a new 12-inch pipeline in Cypress Avenue from Sunrise Avenue to Casa Grande Avenue.
- **Z6-P15**: Construct a new 24-inch pipeline in Citrus Avenue from planned reservoir 6-6 to approximately 1,000 ft south of Duncan Canyon Road.
- **Z6-P16**: Construct a new 12-inch pipeline in future right-of0way from Knox Avenue to Citrus Avenue.

### 7.8.7 Pressure Zone 7

The following section documents pipeline improvements within Pressure Zone 7.

- **Z7-P1**: Construct a new 12-inch pipeline in Alder Avenue from Via Bello Drive to Lytle Creek Ranch Development.
- Z7-P2: Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development
- **Z7-P3**: Construct a new 18-inch pipeline in the planned Lytle Creek Ranch development.
- **Z7-P4**: Construct a new 16-inch pipeline in the planned Lytle Creek Ranch development.
- Z7-P5: Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- **Z7-P6:** Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- **Z7-P7:** Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- Z7-P8: Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- Z7-P9: Construct a new 18-inch pipeline in the planned Lytle Creek Ranch development.
- **Z7-P10**: Construct a new 18-inch pipeline in the planned Lytle Creek Ranch development.
- Z7-P11: Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- **Z7-P12:** Construct a new 12-inch pipeline in the planned Lytle Creek Ranch development.
- **Z7-P13**: Construct a new 12-inch pipeline in Cypress Avenue from Terra Vista Drive to Sunrise Drive.
- **Z7-P14:** Construct a new 16-inch pipeline in from Terra Vista Drive to Duncan Canyon Road.

- **Z7-P15**: Construct a new 12-inch pipeline in Sunrise Drive from Sierra Avenue to Citrus Avenue
- **Z7-P16**: Construct a new 18-inch pipeline in future right-of-way from Citrus Avenue to Lytle Creek Road. This pipeline includes a casing to cross beneath I-10.
- Z7-P17: Construct a new 12-inch pipeline in Coyote Canyon Road from Lytle Creek Road to Hawk Ridge Road.
- **Z7-P18:** Construct a new 12-inch pipeline in planned future development south of Duncan Canyon Road.

### 7.8.8 Bunker Hill Supply

The following section documents pipeline improvements to convey future Bunker Hill supply to the existing District transmission system.

- **BH-P1:** Construct new 18-inch pipelines from the planned wells 43, 44, 45, and 46 to the planned in Alder Avenue from Via Bello Drive to Lytle Creek Ranch Development.
- **BH-P1:** Construct new 18-inch pipelines from the planned wells 43, 44, 45, and 46 to the planned in Alder Avenue from Via Bello Drive to Lytle Creek Ranch Development.
- **BH-P2**: Construct a new 36-inch pipeline from the planned Bunker Hill supply to the existing pump station 3A site.

### CHAPTER 8 – CAPITAL IMPROVEMENT PROGRAM

This chapter provides a summary of the recommended domestic water system improvements to mitigate existing capacity deficiencies and to accommodate anticipated future growth. The chapter also presents the cost criteria and methodologies for developing the capital improvement program. Finally, a capacity allocation analysis, usually used for cost sharing purposes, is also included.

### 8.1 COST ESTIMATE ACCURACY

Cost estimates presented in the CIP were prepared for general master planning purposes and, where relevant, for further project evaluation. Final costs of a project will depend on several factors including the final project scope, costs of labor and material, and market conditions during construction.

The Association for the Advancement of Cost Engineering (AACE International), formerly known as the American Association of Cost Engineers has defined three classifications of assessing project costs. These classifications are presented in order of increasing accuracy: Order of Magnitude, Budget, and Definitive.

Order of Magnitude Estimate. This classification is also known as an "original estimate",
 "study estimate", or "preliminary estimate", and is generally intended for master plans and
 studies.

This estimate is not supported with detailed engineering data about the specific project, and its accuracy is dependent on historical data and cost indexes. It is generally expected that this estimate would be accurate within -30 percent to +50 percent.

- Budget Estimate. This classification is also known as an "official estimate" and generally intended for predesign studies. This estimate is prepared to include flow sheets and equipment layouts and details. It is generally expected that this estimate would be accurate within -15 percent to +30 percent.
- **Definitive Estimate.** This classification is also known as a "final estimate" and prepared during the time of contract bidding. The data includes complete plot plans and elevations, equipment data sheets, and complete specifications. It is generally expected that this estimate would be accurate within -5 percent to + 15 percent.

Costs developed in this study should be considered "Order of Magnitude" and have an expected accuracy range of -30 percent and +50 percent.

### 8.2 COST ESTIMATE METHODOLOGY

Cost estimates presented in this chapter are opinions of probable construction and other relevant costs developed from several sources including cost curves, Akel experience on other master planning projects, and input from District staff. Where appropriate, costs were escalated to reflect the more current Engineering News Records (ENR) Construction Cost Index (CCI).

This section documents the unit costs used in developing the opinion of probable construction costs, the Construction Cost Index, the land acquisition costs, and markups to account for construction contingency and other project related costs.

### 8.2.1 Unit Costs

Table 8.1. Domestic water pipeline unit costs are based on length of pipes, in feet. Storage reservoir unit costs are based on capacity, per million gallons (MG). Pump Station costs are based on an equation that utilizes the total recommended pump station improvement capacity. Well construction costs are preliminary and are intended for planning purposes; a well site investigation is recommended to determine site specific costs involved in new well construction.

The unit costs are intended for developing the Order of Magnitude estimate and do not account for site specific conditions, labor and material costs during the time of construction, final project scope, implementation schedule, detailed utility and topography surveys for reservoir sites, investigation of alternative routings for pipes, and other various factors. The capital improvement program included in this report accounts for construction and project-related contingencies as described in this chapter.

### 8.2.2 Treatment Costs

Kleinfelder used an analogous cost estimating methodology, which consisted of researching similar facilities and documenting those costs for the purposes of estimating proposed capital improvements costs for the water treatment facilities for the District. Based on water quality data, the best available technology was identified for each specific water source and its associated contaminant (s).

- Microsand based Actiflo coagulation and sedimentation is selected as a practical technology for wellhead treatment to remove arsenic from ground water most commonly occurring in the Lytle Creek Basin water.
- Single pass IX technology is selected for perchlorate removal, a contaminant identified in ground waters of the Rialto Basin and Riverside-Arlington Basin.
- Regenerable IX technology is selected for nitrate removal, the contaminant in the ground water wells of Rialto Basin, Riverside-Arlington Basin and Chino Basin.

### **Table 8.1 Unit Costs**

## Water Facilities Master Plan West Valley Water District

**PRELIMINARY** 

Pipe	PRELIMINARY lines <sup>1</sup>
Pipe Size	New/Parallel/Replacement
(in)	(\$/unit length)
6	100
8	133
10	167
12	200
16	267
18	300
20	333
24	400
30	500
36	600
42	700
Pump	Station
	Station Project Cost=
2.075*10 <sup>(0.7583*log(Q)+3.</sup>	O.
Sto	rage <sup>2</sup>
	/ gallon
Land Acqui	isition Cost <sup>3</sup>
\$7.99 per	square foot
Pipeline	e Casings
\$24 per inch diam	eter per linear foot
Groundw	ater Wells
\$3,000,00	00 per well
AKEL ENGINEERING GROUP, INC.	

### Notes:

- Pipeline unit cost based on \$15/in.-diameter/foot, consistent with 2014 East Valley Water District Water Master Plan
- 2. Source: 2014 East Valley Water District Water Master Plan
- 3. Source: Land appraisal report received from District staff October 12, 2017.
- 4. Unit costs escalated based on an ENR CCI Index Value of 10,889 (01/2018)

The above water treatment technologies were selected solely for purpose of construction cost estimates for this Water Master Plan, as representatives of reasonable cost technologies.

To estimate costs for the proposed facilities, known cost of similar designed or constructed facilities were prorated proportionally with the flow rates. To accommodate the economy of scale and to come up with cost "multipliers", the prorated values are powered with power index varying from 0.5 to 0.65. Finally, the costs were adjusted using an "Escalation Factor," which was calculated for each individual facility assuming 2.5% for annual inflation.

Details of the applied methodology, selected treatment technologies, sources of analogous cost information (Cost basis), calculated *Multipliers* and *Escalation Factors*, and estimated cost for the proposed wellhead treatments are presented in Table 8.2.

### 8.2.3 Construction Cost Index

Costs estimated in this study are adjusted utilizing the Engineering News Record (ENR) Construction Cost Index (CCI), which is widely used in the engineering and construction industries.

The costs in this Water Facilities Master Plan were benchmarked using a 20-City national average ENR CCI of 10,889, reflecting a date of January 2018.

### 8.2.4 Land Acquisition

Construction of pipelines is generally assumed to be within existing or future street right-of-ways. A land acquisition fee for the construction of storage reservoirs and pump stations was assumed based on recent land acquisitions.

### 8.2.5 Construction Contingency Allowance

Knowledge about site-specific conditions for each proposed project is limited at the master planning stage; therefore, construction contingencies were used. The estimated construction costs in this master plan include a **20 percent** contingency allowance to account for unforeseen events and unknown field conditions.

### 8.2.6 Project Related Costs

The capital improvement costs also account for project-related costs, comprising of engineering design, project administration (developer and District staff), construction management and inspection, and legal costs. The project related costs in this master plan were estimated by applying an additional 15 percent to the estimated construction costs.

Table 8.2 CIP Cost Estimates for Wellhead Treatments

Water Master Plan Update West Valley Water District **PRELIMINARY** 

No.	Well	Contaminant	Applicable Technology	Well Capacity,	Multiplier	Escalation factor, 2.5%	CIP Wellhead Treatment Cost	reatment Cost	Comment
			:	(mdg)		per year	2024	2055	
Lytle Cre	Lytle Creek Basin								
1	W7	No WQ issues	NA	2,100	NA	NA	50,000	0	Rehab and retest existing well
2	W8A	As	Coagulation	2,400	0.93	A N	3,288,359	0	Construct Arsenic treatment , assumed Actiflo
8	W36	As	Coagulation	2,700	1.00	ΑN	3,550,000	0	Construct Arsenic treatment , assumed Actiflo
4	W34B	Assumed, As	Coagulation	2,000	0.82	ĄN	0	2,920,864	Construct Arsenic treatment , assumed Actiflo
2	W35C	Assumed, As	Coagulation	2,000	0.82	Ą	0	2,920,864	Construct Arsenic treatment , assumed Actiflo
					Subtotal - Lytle Creek Basin	Creek Basin	6,888,359	5,841,728	
Rialto Basin	sin								
9	W16	CIO4, NO2	IX for nitrate	1,500	1.00	1.22	0	5,716,015	Current ClO4 is OK. Construct IX for NO2 only
7	W17	CI04	Existing IX is OK	1,250	NA	Ϋ́	0	0	Current IX for ClO4 is OK. Regular maintenance, only
∞	W22A	NO2	IX for nitrate	1,500	1.00	1.22	0	5,716,015	Construct IX for NO2 only
6	W24	No WQ issue	ΝΑ	009	ΝΑ		0	0	Regular Maintenance, Only
10	W54	Air	Dearation , break tanks	1,000	Ϋ́	A N	150,000	0	Install 30 minute RT break tank
					Subtotal -	Subtotal - Rialto Basin	150,000	11,432,030	
Bunker F	Bunker Hill Basin								
11	W15	No WQ issue	AN	2,700	AN		0	0	Regular Maintenance, Only
12	W30	No WQ issue	ΑN	3,100	AN		0	0	Regular Maintenance, Only
13	W43	No WQ issue	NA	3,500	NA		0	0	Well construction
14	W44	No WQ issue	٧٧	3,500	ΝΑ	1	0	0	Well construction
15	W45	No WQ issue	٧N	3,500	NA		0	0	Well construction
16	W46	No WQ issue	ΨZ	3,500	Ϋ́	-	0	0	Well construction
					Subtotal - Bunker Hill Basin	ker Hill Basin	0	0	
North Ri	North Riverside Basin								
17	W18A	CIO4, NO2	IX, FBR	2,700	1.34	1.22	7,668,839	0	Current ClO4 is OK. IX for NO2 is proposed
18	W41	CIO4	IX, FBR	2,200	0.84	1.22	250,000		IX for NO2 only <sup>2</sup>
19	W42	CIO4 and NO2	IX, FBR	2,200	1.28	1.22	0	9,246,213	IX for Nitrate and IX for Perchlorate
20	W29A	CIO4 and NO2	IX, FBR	1,500	1.00	1.22	0	7,208,559	IX for Nitrate and IX for Perchlorate
21	W40	ClO4 and NO2	IX, FBR	1,500	1.00	1.22	0	7,208,559	IX for Nitrate and IX for Perchlorate
22	W51	ClO4 and NO2	IX, FBR	3,000	1.57	1.22	0	11,311,441	IX for Nitrate and IX for Perchlorate
23	W52	CIO4 and NO2	IX, FBR	2,000	1.21	1.22	0	8,690,777	IX for Nitrate and IX for Perchlorate
24	W50	CIO4 and NO2	IX, FBR	1,500	1.00	1.22	0	7,208,559	IX for Nitrate and IX for Perchlorate
				Subt	Subtotal - North Riverside Basin	rerside Basin	8,218,839	50,874,108	
Chino Basin	ısin								
25	W39	No WQ issue	AN	4,000	NA	1	9,334,214	0	Well drilled but not equipped. Requires treatment.
					Subtotal -	Subtotal - Chino Basin	9,334,214	0	
				Subtotal - 1	Subtotal - Total Wellhead Treatments	d Treatments	24,591,412	68,147,866	
Notes:									3/7/2019
1. Table p	repared by Kleinfelde	1. Table prepared by Kleinfelder, Inc staff February 2018.							

2. District staff indicated that 2 available treatment vessels are currently unused at the Reservoir 2-1 site. Those vessels may potentially be moved to W41 for treatment purposes. Cost shown accounts for this assumption.

3. Well costs include master planning contingencies provided by Kleinfelder, Inc staff, which include overhead, margin, insurance and bonding, and contingency.

### 8.3 CAPITAL IMPROVEMENT PROGRAM

This section documents the capital improvement program and the allocation of costs to meet the requirements of AB1600.

### 8.3.1 Capital Improvement Costs

The Capital Improvement Program costs for the projects identified in this master plan for mitigating existing system deficiencies and for serving anticipated future growth throughout the District are summarized by improvement type on Table 8.3 through Table 8.6.

As summarized in previous chapters the District is currently planning a 6.0 mgd expansion to the OPR WFF; however, in the event additional surface water supplies become available the District may elect to increase this expansion up to 16.0 mgd. Therefore, for conservative planning purposes, this capital improvement program includes the cost of a 16.0 mgd expansion. This cost estimate, prepared by Carollo Engineers, is summarized on Table 8.4.

Each improvement was assigned a unique coded identifier associated with the improvement type, and are summarized graphically on Figure 8.1 through Figure 8.4. A hydraulic profile schematic of the buildout of the water distribution system is provided on Figure 8.5.

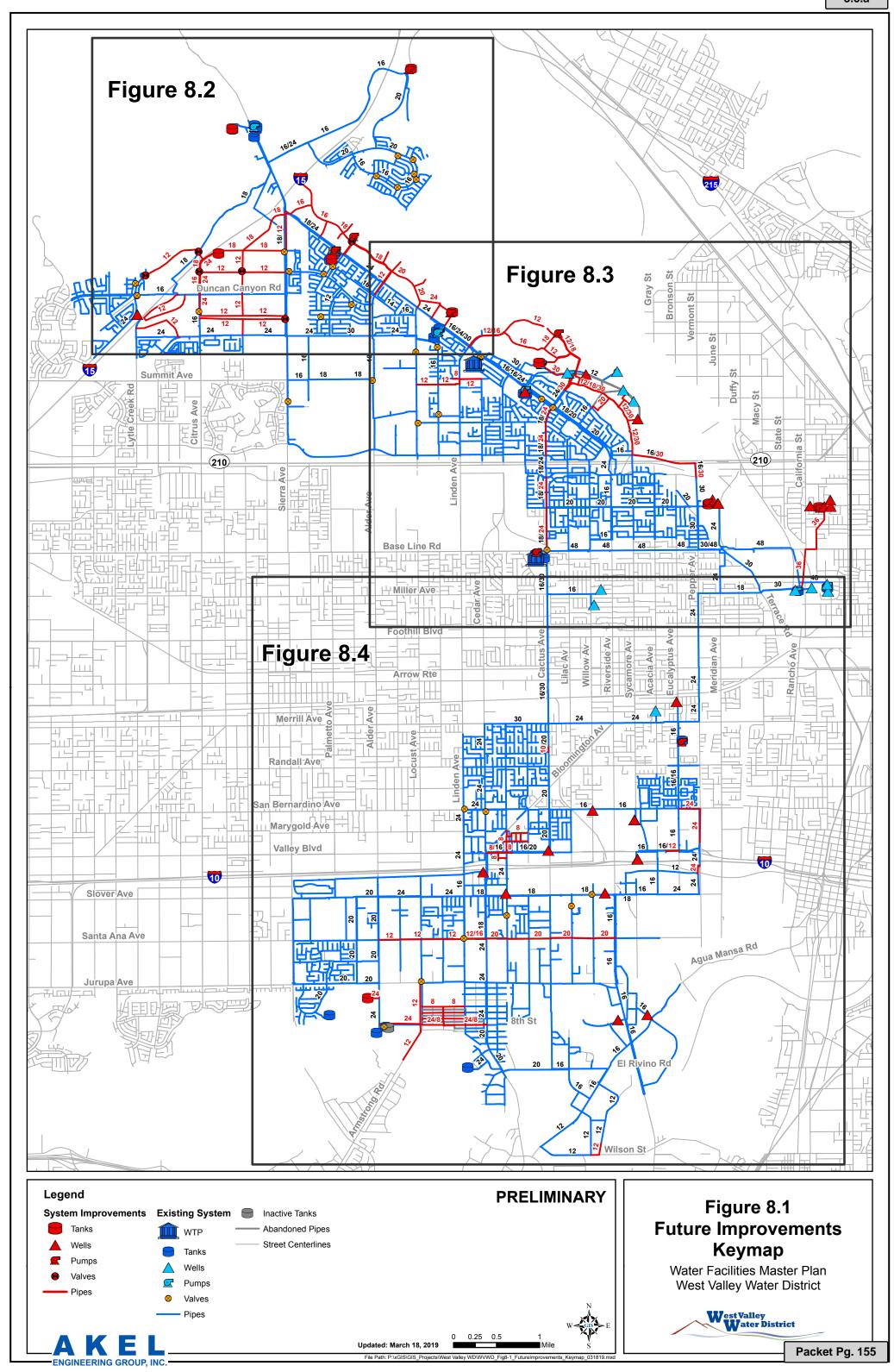
The estimated construction costs include the baseline costs plus **20 percent** contingency allowance to account for unforeseen events and unknown field conditions, as described in a previous section. Capital improvement costs include the estimated construction costs plus **15 percent** project-related costs (engineering design, project administration, construction management and inspection, and legal costs. It should be noted that contingencies for costs associated with well construction and treatment were provided by Kleinfelder Inc and account for margin, overhead, insurance and bonding, or contingencies.

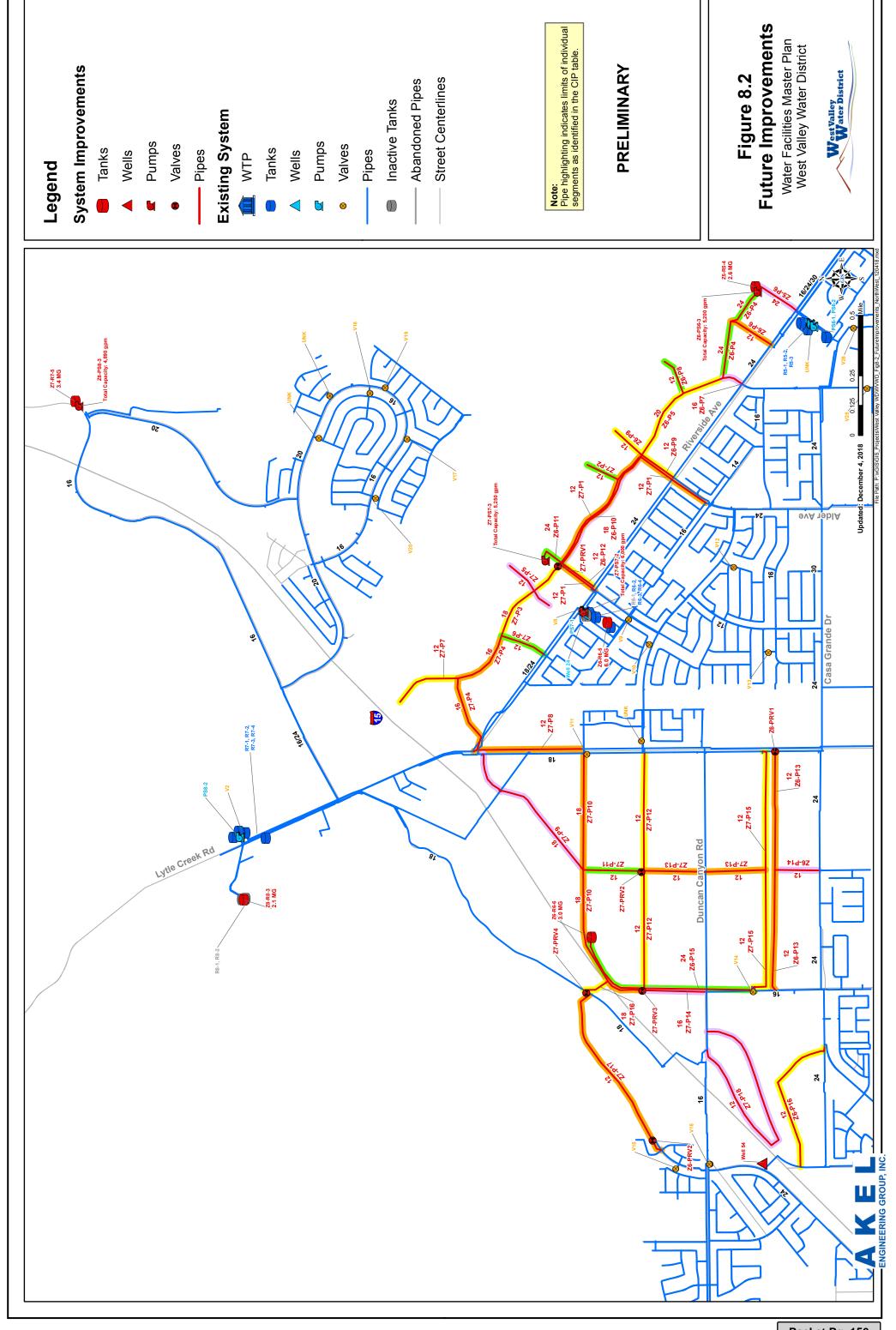
### 8.3.2 Recommended Cost Allocation Analysis

Cost allocation analysis is needed to identify improvement funding sources, and to establish a nexus between development impact fees and improvements needed to service growth. In compliance with the provisions of Assembly Bill AB 1600, the analysis differentiates between the project needs of servicing existing users and for those required to service anticipated future developments. The cost responsibility is based on model parameters for existing and future land use, and may change depending on the nature of development. Table 8.3 lists each improvement, and separates the cost by responsibility between existing and future users.

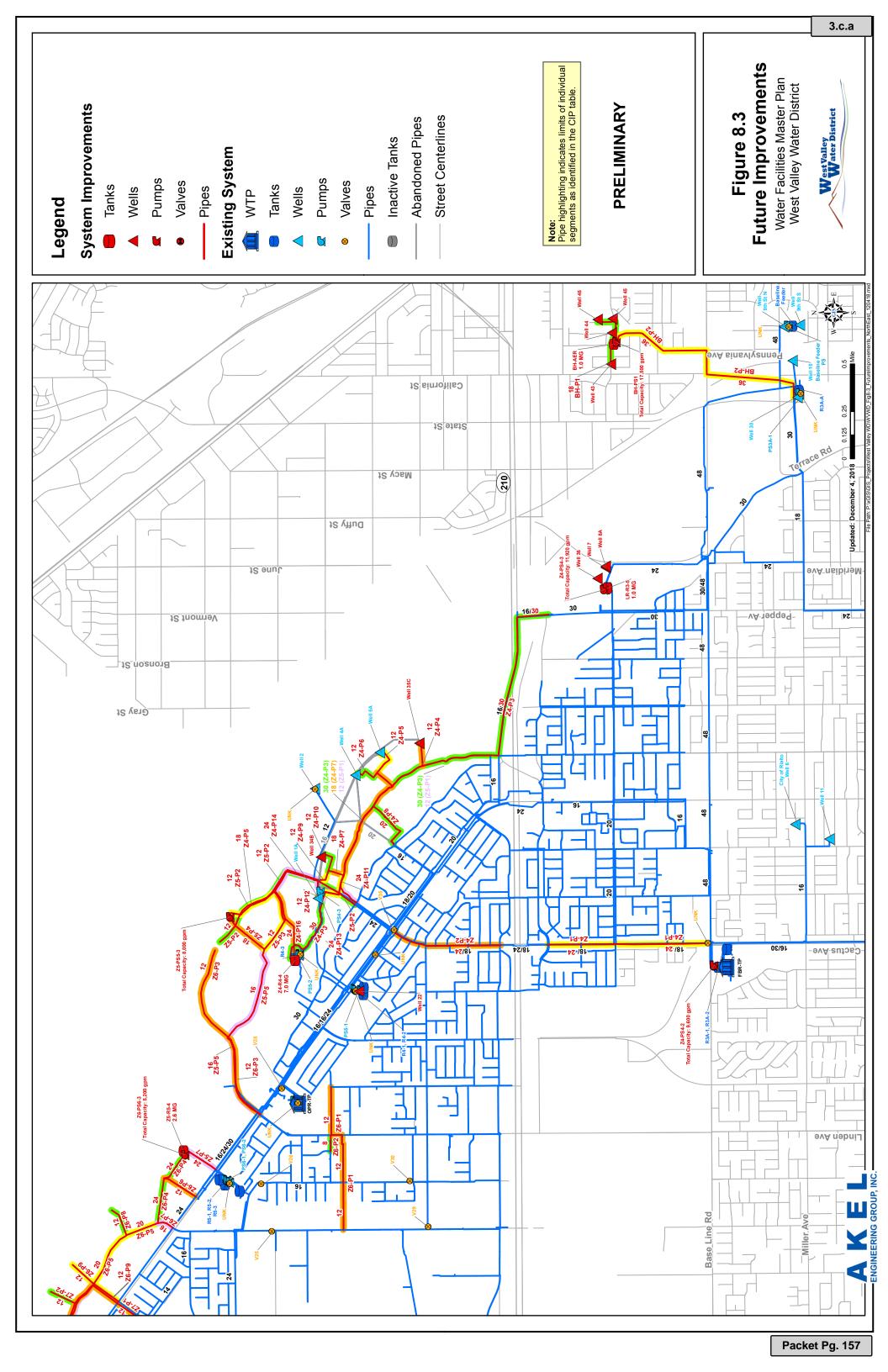
### 8.3.3 5-Year Capital Improvement Costs and Phasing

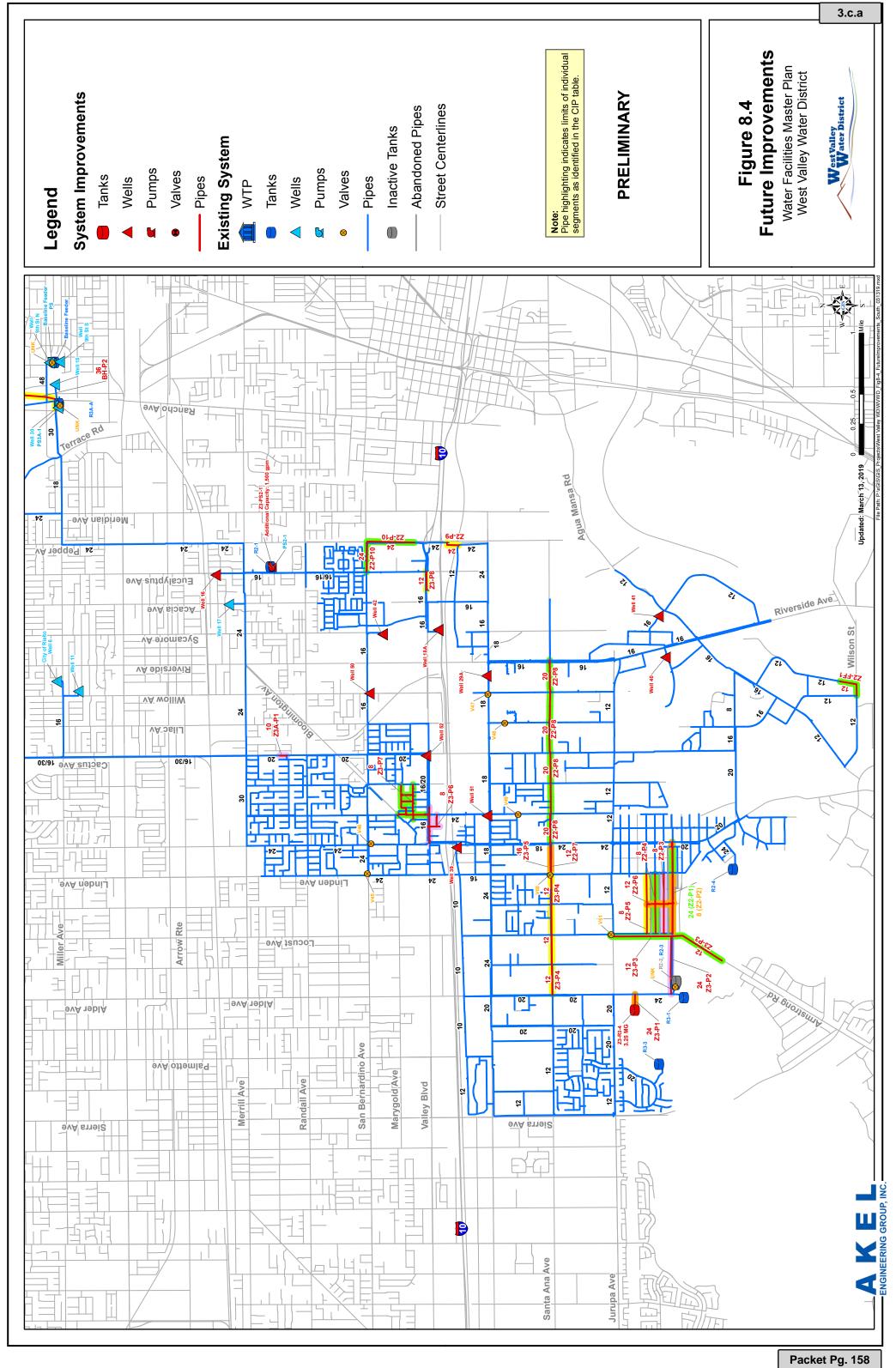
The capital improvement program costs and phasing for the next five years are summarized on Table 8.7. This plan includes the total costs for pipelines, tanks, booster stations, and valves to be

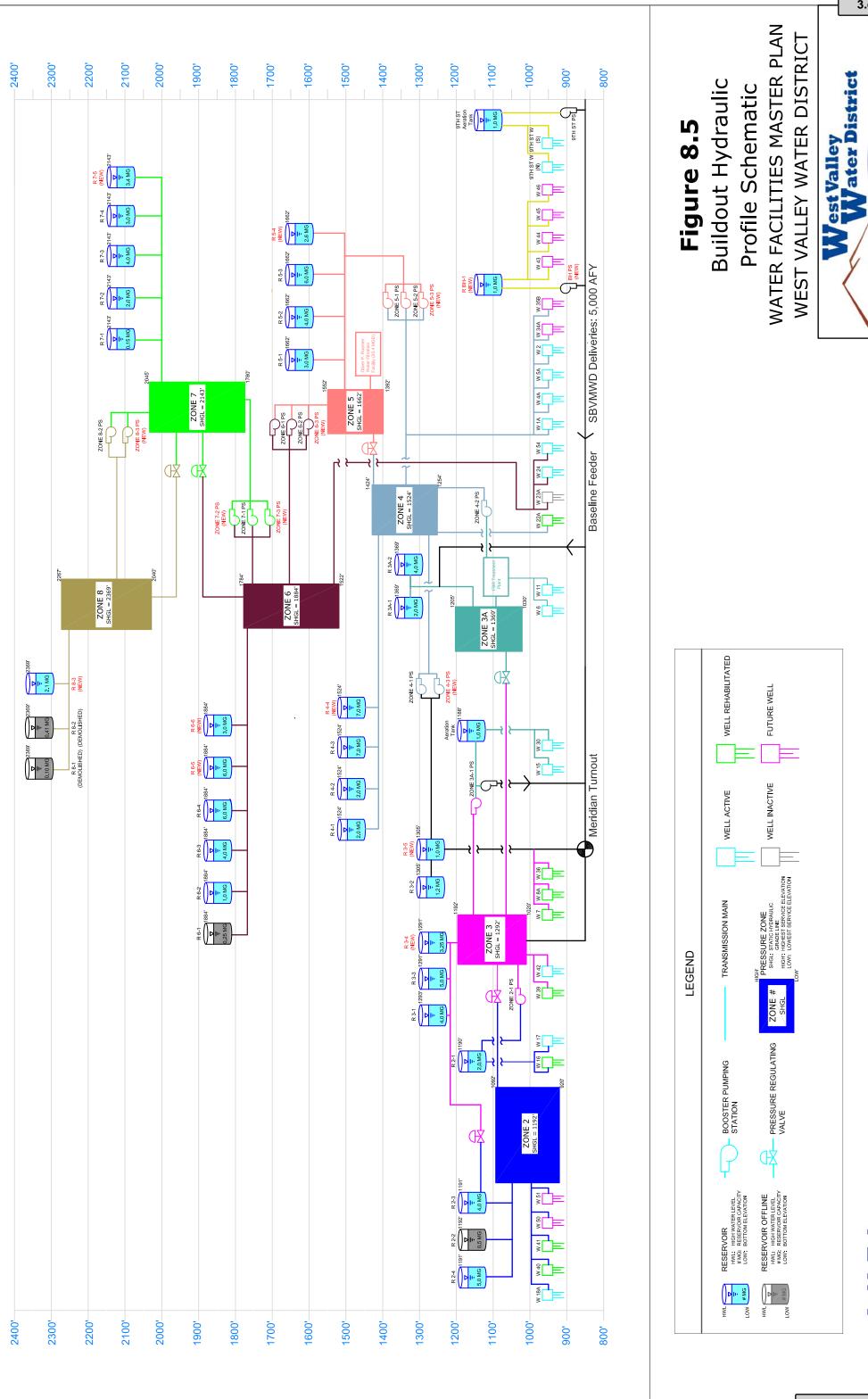




3.c.a







3.c.a

# Table 8.3 Capital Improvement Costs - Pipelines

Water Facilities Master Plan West Valley Water District **PRELIMINARY** 

1,394,000 10,070,000 1,863,000 1,449,000 1,725,000 1,636,000 387,000 2,497,000 194,000 263,000 235,000 1,863,000 3,230,000 738,000 118,000 221,000 21,000 69,000 994,000 704,000 0 0 0 0 0 0 0 0000 0 0 0 0 0 Cost Sharing 1,484,000 517,000 1,040,000 10,824,000 2,222,000 650,000 3,794,000 585,000 850,000 650,000 650,000 400,000 380,000 304,000 210,000 3,713,000 143,500 339,000 462,000 0 0 0 0 0 0 0 0 0 0 0 **Existing Users** Future Users Suggested Cost Allocation 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% % % %0 %0 % %0 %0 % % % 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% 100% %0 %0 %0 % % % %0 %0 % %0 % % % % % % % As Development Occurs With PS 4-2 Expansion With PS 4-2 Expansion With Reservoir 3-4 With Reservoir 4-4 With Well 35C Immediate Buildout Buildout Buildout Five-Year Five-Year Five-Year Five-Year Five-Year Five-Year Five-Year Buildout Five-Year Buildout Five-Year Five-Year Buildout Buildout Buildout Buildout Buildout Buildout Buildout Buildout Five-Year Five-Year Five-Year Five-Year Five-Year Five-Year Five-Year Buildout Buildout Buildout Buildout Capital Improv. Costs<sup>2,3</sup> 1,040,000 1,394,000 10,070,000 2,222,000 380,000 3,794,000 1,863,000 12,687,000 1,449,000 1,484,000 2,497,000 1,725,000 21,000 69,000 994,000 650,000 400,000 650,000 339,000 387,000 462,000 517,000 210,000 143,500 194,000 263,000 235,000 1,636,000 850,000 650,000 304,000 585,000 6,943,000 738,000 118,000 221,000 704,000 143,500 Estimated Const.
Costs<sup>1</sup> 330,000 1,290,000 1,500,000 1,620,000 264,000 6,315,000 336,000 1,212,000 1,260,000 449,000 904,000 2,171,000 8,756,000 168,000 228,000 204,000 1,422,000 641,000 102,000 192,000 60,000 5,852,000 612,000 508,000 294,000 401,000 18,000 0 Baseline Constr. Costs 1,075,000 1,010,000 2,749,000 1,350,000 1,250,000 275,000 220,000 423,000 5,262,000 280,000 1,050,000 334,000 374,000 4,876,000 1,809,000 7,296,000 140,000 190,000 170,000 1,185,000 534,000 753,000 160,000 720,000 245,000 85,000 15,000 50,000 0 279,832 1,009,392 1,249,248 2,748,345 1,349,188 Subtotal - Pressure Zone 2 1,049,368 1,074,353 Subtotal - Pressure Zone 3 Subtotal - Pressure Zone 3A 1,808,911 7,295,608 274,835 1,184,287 219,868 373,109 189,886 169,898 719,567 244,853 752,880 139,916 159,904 509,693 422,400 333,133 533,012 84,949 14,991 49,970 Infrastructure Costs 200 200 200 133 333 400 400 200 400 400 500 200 200 200 300 200 200 400 400 400 24 14,600 1,225 4,000 4,075 2,625 1,275 1,375 8,250 3,375 2,525 5,250 5,375 1,250 2,800 5,650 3,125 1,600 2,625 2,625 3,950 1,275 75 125 1,800 875 700 550 400 700 950 850 425 800 12 12 16 30 24 12 24 24 24 8 8 112 12 20 24 24 24 12 12 10 24 24 12 12 12 18 20 12 12  $\infty$ ∞  $\infty$   $\infty$ Replace Replace Replace Replace Replace Replace Replace Replace Parallel Parallel Parallel New 4, 6, 12 2, 4, 6 2, 4, 6 4,6 4,8 4,6 16 - 9 12 ı 18 1 Generally between Cedar Ave and Larch Ave Generally Between Olive St and Spruce Ave From approx. 850' w/o Eucalyptus Ave to Facility to reservoir 4-3 site From planned reservoir 3-4 site to Alder Ave s/o railway to 150' n/o n/o the intersection of From Sycamore Ave to Lytle Creek Ranch From the intersection of Brown Ave and Wilson St to approx 700' n/o Wilson St to approx. 2,200' sw/o From approx. 1,200 ft n/o Slover Ave to Bernardino Ave and From Well 34B to Lytle Creek Ranch From Well 35C to Lytle Creek Ranch Development From Well 5A to Lytle Creek Ranch From Well 4A to Lytle Creek Ranch to Riverside Ave Development

Development

Development From Cedar Ave to Riverside Ave Development Development Development to Linden Ave From Locust Ave to Linden Ave to Cedar Ave to Cedar Ave to Cedar Ave From Eleventh St to Eighth St From Alder Ave to Linden Ave Limits From Baseline Rd to I-210 From James St to Alru St approx. 300 ft s/o I-10 From approx. 150' s/o From approx. 400' Lytle Creek Ranch Valley Blvd and Pe intersection of Sai From Alder Ave to Lytle Creek Ranch From Locust Ave From Jurupa Ave From Lord Ranch From Locust Ave From Locust Ave From Linden Ave From Linden Ave From Casmalia St railway (Casing) **Eucalyptus Ave Eucalyptus Ave** Development Development Development Development Eighth St Pepper Ave, Highland Ave, Oakdale Pepper Ave, San Bernardino Ave Valley Blvd, s/o Valley Blvd Valley Blvd, s/o Valley Blvd Locust Ave, Armstrong Rd Generally n/o Valley Blvd Holly St and Wilson St **Bloomington Phase 4 Bloomington Phase 4 Bloomington Phase 3 Bloomington Phase 5** Bloomington Phase 3 Bloomington Phase 3 Ave, Future ROW Santa Ana Ave Santa Ana Ave Santa Ana Ave Santa Ana Ave **Future ROW Future ROW Future ROW** Pepper Ave Pepper Ave Future ROW Cactus Ave Future ROW **Future ROW** Future ROW Future ROW **Future ROW** Future ROW Future ROW Future ROW Cactus Ave Cactus Ave 7 4 4 4 7 7 7 7 7 7 7 7 3 3 က 3  $\alpha$ m Pressure Zone 3A m 4 4 4 4 4 4 4 4 4 Pressure Zone 4 Pressure Zone 2 Pressure Zone 3 Z3A-P1 Z4-P11 Z2-P10 Z4-P10 Z4-P12 Z4-P13 **Z2-P9C** Z2-FF1 Z4-P14 Z4-P9 Z2-P1 Z2-P2 Z2-P3 Z2-P4 Z2-P5 Z2-P6 Z2-P7 Z2-P8 Z2-P9 Z3-P1 Z3-P2 Z3-P3 Z3-P4 Z3-P5 23-P6 Z3-P7 Z3-P8 Z4-P1 Z4-P2 Z4-P3 Z4-P4 24-P5 Z4-P6 Z4-P7 Z4-P8

Table 8.3 Capital Improvement Costs - Pipelines
Water Facilities Master Plan
West Valley Water District

															4	PRELIMINARY
Pressure Improv. No. Zone	Alignment	Limits	Pipeline Improvements  New/Parallel/ Existing Diameter Replace	orovements New/Parallel/ Replace	Diameter	Infrastructure Costs  Length Unit Cost	Cost Infr. Cost	Baseline Constr. Costs	r. Estimated Const. Costs <sup>1</sup>	. Capital Improv. Costs <sup>2,3</sup>	Improvement Horizon	Construction Trigger	Suggested Co Existing Users	Suggested Cost Allocation xisting Users Future Users	Cost Sharing Existing Users Fut	ring Future Users
Z4-P15 4	Future ROW	Lytle Creek Ranch Development	( <u>E</u> )	New	18	(f) (S) (S) (1,550 300	) (S) (S) (D) (A) (A) (A) (B) (B) (B) (B) (B) (B) (B) (B) (B) (B	0 465,000	(8) 558,000	642,000	Buildout	As Development Occurs	%0	100%	0	642,000
<b>Z4-P16</b> 4	Future ROW	Lytle Creek Ranch Development	-	New	24	2,125 400		000'058 6	1,020,000	1,173,000	Buildout	As Development Occurs	%0	100%	0	1,173,000
						Subtotal -	Subtotal - Pressure Zone 4	15,429,000	18,516,000	21,300,000					0	21,300,000
Pressure Zone 5	ANG Contained	the state of the s			ç			_	000	1000	467110	the second secon	790	1000/	c	000
	Future ROW	Lytie Creek Ranch Development		New	71 71	002 006,0			1,656,000	1,374,000	Buildaut	As Development Occurs	% 6	100%	o 0	1,374,000
	ratale now	Lytie Creek halloli Developillelit	•	אַט אַ					1,134,000	1,374,000	buildout	As Development Occurs	%0	100%	<b>&gt;</b> 0	1,374,000
	Future ROW	Lytie Creek Kanch Development		New					462,000	532,000	Buildout	As Development Occurs	%0	3001	0 (	532,000
	Future ROW	Lytle Creek Ranch Development		New					460,000	529,000	Buildout	As Development Occurs	%0	100%	0	529,000
<b>ZS-P5</b> 5	Future ROW	Lytle Creek Ranch Development		New				` '	1,728,000	1,988,000	Buildout	As Development Occurs	%0	100%	0	1,988,000
<b>ZS-P6</b> 5	Future ROW	Lytle Creek Ranch Development	-	New	24	1,000 400	399,759	400,000	480,000	552,000	Buildout	As Development Occurs	%0	100%	0	552,000
						Subtotal -	Subtotal - Pressure Zone 5	4,983,000	5,980,000	6,880,000					0	6,880,000
Pressure Zone 6								_					_			
<b>Z6-P1</b> 6	Persimmon St and Summit Ave	Generally between Locust Ave and Cedar Ave	ı	New	12	4,375 200	0 874,474	4 875,000	1,050,000	1,208,000	Buildout	As Funding is Available	100%	%0	1,208,000	0
<b>Z6-P2</b> 6	Persimmon St and Summit Ave	Generally between Locust Ave and Cedar Ave	4, 6	Replace	∞	475 133	13 63,295	64,000	77,000	89,000	Buildout	As Funding is Available	100%	%0	89,000	0
9 <b>26-P3</b>	Future ROW	Lytle Creek Ranch Development		New	12	5,275 200	0 1,054,365	1,055,000	1,266,000	1,456,000	Buildout	As Development Occurs	%0	100%	0	1,456,000
	Future ROW	Lytle Creek Ranch Development		New		2,175 400	00 869,477	7 870,000	1,044,000	1,201,000	Buildout	As Development Occurs	%0	100%	0	1,201,000
	Future ROW	Lytle Creek Ranch Development		New					1,050,000	1,208,000	Buildout	As Development Occurs	%0	100%	0	1,208,000
	Future ROW	Lytle Creek Ranch Development		New		_			252,000	290,000	Buildout	As Development Occurs	%0	100%	0	290,000
	Future ROW	Lytie Creek Ranch Development		New	16 1,				153,000	176,000	Buildout	As Development Occurs	% %	100%	0 0	176,000
9 94-97	Future ROW	Lytte Creek Kallott Development		New Now	12	1,650 200	0 329 801	330,000	396,000	756,000	Buildout	As Development Occurs	% %	100%	<b>&gt;</b> C	756,000
	Future ROW	Lytle Creek Ranch Development	-	New					1,089,000	1,253,000	Buildout	As Development Occurs	%0	100%	0	1,253,000
	Future ROW	Lytle Creek Ranch Development		New					264,000	304,000	Buildout	As Development Occurs	%0	100%	0	304,000
<b>Z6-P12</b> 6	Future ROW	Lytle Creek Ranch Development	-	New	12	875 200	174,895	5 175,000	210,000	242,000	Buildout	As Development Occurs	%0	100%	0	242,000
<b>Z6-P13</b> 6	Sunrise Dr	From Sierra Ave to Citrus Ave	,	New		5,325 200	0 1,064,359	1,065,000	1,278,000	1,470,000	Buildout	As Development Occurs	%0	100%	0	1,470,000
<b>Z6-P14</b> 6	Cypress Ave	From Sunrise Ave to Casa Grande Ave	•	New	12	975 200	0 194,883	3 195,000	234,000	270,000	Buildout	As Development Occurs	%0	100%	0	270,000
<b>Z6-P15</b> 6	Citrus Ave	From planned reservoir 6-6 site to approx. 1,000' s/o Duncan Canyon Rd	ı	New	24	4,350 400	0 1,738,953	1,739,000	2,087,000	2,401,000	Buildout	With Reservoir 6-6	%0	100%	0	2,401,000
<b>26-P16</b> 6	Future ROW	From Knox Ave to Citrus Ave	1	New	12	3,325 200	00 664,600	000(299 0	798,000	918,000	Buildout	As Development Occurs	%0	100%	0	918,000
Pressure Zone 7						Subtotal -	Subtotal - Pressure Zone 6	9,542,000	11,452,000	13,177,000					1,297,000	11,880,000
		طومتوا بامميل مالبنا مليط مالموا مزارا مسميا														
<b>Z7-P1</b> 7	Alder Ave	From via bello Dr to Lytie Creek Kalitii Development	1	New	12	5,750 200	1,149,308	1,150,000	1,380,000	1,587,000	Buildout	As Development Occurs	%0	100%	0	1,587,000
Z7-P2 7	Future ROW	Lytle Creek Ranch Development	1	New	12	775 200		7 155,000	186,000	214,000	Buildout	As Development Occurs	%0	100%	0	214,000
	Future ROW	Lytle Creek Ranch Development		New					712,000	819,000	Buildout	As Development Occurs	%	100%	0	819,000
27-P4 /	Future ROW	Lytle Creek Kanch Development		New	16 17	3,275 267	8/2,808	8 8/3,000	1,048,000	1,206,000	Buildout	As Development Occurs	% %	100%	<b>-</b>	1,206,000
	Future BOW	Lytic Creek Ranch Development		New					246.000	283.000	Buildout	As Development Occurs	% %	100%	0 0	283.000
	Future ROW	Lytle Creek Ranch Development		New					360,000	414,000	Buildout	As Development Occurs	%0	100%	0	414,000
Z7-P8 7	Sierra Ave	From Riverside Ave to Segovia Ln	1	New		2,250 200	0 449,729	450,000	540,000	621,000	Five-Year	As Development Occurs	%0	100%	0	621,000
Z 64-7Z	Future ROW	From the intersection of Riverside Ave and Sierra Ave to the intersection of Conress Ave	1	New	18	3,625 300	1,086,846	1,087,000	1,305,000	1,501,000	Buildout	As Development Occurs	%0	100%	0	1,501,000
Z7-P10 7	Segovia Ln	From Sierra Ave to Citrus Ave	•	New	18	5,950 300	0 1,783,926	1,784,000	2,141,000	2,463,000	Buildout	As Development Occurs	%0	100%	0	2,463,000
	Cypress Ave	From Segovia Ln to Terra Vista Dr		New					294,000	339,000	Buildout	As Development Occurs	%0	100%	0	339,000
Z7-P12 7	Terra Vista Dr	From Sierra Ave to Citrus Ave		New		5,225 200	00 1,044,371	1,045,000	1,254,000	1,443,000	Buildout	As Development Occurs	%0	100%	0	1,443,000
Z7-P13 7	Cypress Ave	From Terra Vista Dr to Sunrise Dr		New		2,650 200	00 529,681	1 530,000	636,000	732,000	Buildout	As Development Occurs	%0	100%	0	732,000
	Citrus Ave	From Terra Vista Dr to Duncan Canyon Rd	ı	New					432,000	497,000	Buildout	As Development Occurs	%0	100%	0	497,000
	Sunrise Dr	From Sierra Ave to Citrus Ave		New	12				1,350,000	1,553,000	Buildout	As Development Occurs	%0	100%	0	1,553,000
Z7-P16 7	Future ROW	From Citrus Ave to Lytle Creek Rd	-	New	18	,			216,000	249,000	Buildout	As Development Occurs	%0	100%	0	249,000
	Future ROW	From Citrus Ave to Lytle Creek Rd (Casing)		New	, (				658,000	757,000	Buildout	As Development Occurs	%0	100%	0 0	757,000
/ /14-/7	Coyote Canyon Kd	From Lytie Creek Kd to Hawk Kidge Kd	1	New	17	4,150 200	10 829,501	1 830,000	996,000	1,146,000	FIVe-Year	As Development Occurs	%0	100%	0	1,146,000

Table 8.3 Capital Improvement Costs - Pipelines
Water Facilities Master Plan
West Valley Water District

														PRELIMINARY
Pressure			Pipeline Improvements		Infrastructure Costs	Baseline Constr.	Baseline Constr. Estimated Const.	Cal			Suggested Cost Allocation	Allocation	Cost Sharing	ring
improv. No. Zone	Alignment	E I I I I I I I I I I I I I I I I I I I	Existing Diameter Replace	Diameter	Length Unit Cost Infr. Cost		Costs <sup>1</sup>	Costs <sup>2,3</sup>	Improvement norizon	Construction ingger	Existing Users Future Users		Existing Users	Future Users
Z7-P18 7	Future ROW	Planned Development south of Duncan Canyon Rd	- New		īv ,	1,17	1,41	1,622,000	Buildout	As Development Occurs	%0	100%	0	1,622,000
					Subtotal - Pressure Zone 7	7 12,890,000	15,470,000	17,798,000					0	17,798,000
Bunker Hill Supply											_			
BH-P1 -	To be determined	From planned wells 43, 44, 45, and 46 to planned Bunker Hill aeration tank		18	2,025 300 607,134	4 608,000	730,000	840,000	Five-Year	With Well 43	%0	100%	0	840,000
ВН-Р2 -	To be determined	From planned Bunker Hill supply to existing pump station 3A site		36	6,375 600 3,822,699	3,823,000	4,588,000	5,277,000	Five-Year	With Well 43	%0	100%	0	5,277,000
					Subtotal - Pressure Zone 8	8 4,431,000	5,318,000	6,117,000					0	6,117,000
Total Improvement Cost														
					Pressure Zone 2	5, <b>262,000</b>	6,315,000	12,687,000					10,824,000	1,863,000
					Pressure Zone 3	e 3 <b>4,876,000</b>	5,852,000	6,943,000					3,713,000	3,230,000
					Pressure Zone 3A	3A -		143,500					143,500	0
					Pressure Zone 4	e 4 15,429,000	18,516,000	21,300,000					0	21,300,000
					Pressure Zone 5	e 5 <b>4,983,000</b>	5,980,000	6,880,000					0	6,880,000
					Pressure Zone 6	e 6 <b>9,542,000</b>	11,452,000	13,177,000					1,297,000	11,880,000
					Pressure Zone 7	e 7 <b>12,890,000</b>	15,470,000	17,798,000					0	17,798,000
					Bunker Hill Supply	ply <b>4,431,000</b>	5,318,000	6,117,000					0	6,117,000
2				To	Total Improvement Costs	57,413,000	68,903,000	85,045,500				1	15,977,500	000'890'69
ENGINEERING GROUP, INC.														3/7/2019

Projuerative Group, INC.

Nortes:

1. Baseline construction costs plus 20% to account for unforeseen events and unknown conditions.

2. Estimated construction costs plus 15% to cover other costs including: engineering design, project administration (developer and District staff), construction management and inspection, and legal costs.

3. Costs for improvements shown with only Capital Improvement Cost are based on information provided by WWWD staff.

Table 8.4 Capital Improvement Costs - Storage Reservoirs, Pump Stations, Pressure Reducing Valves

ring	Future Users			6,190,000	13,331,000	4,952,000	11,427,000	5,714,000	6,476,000	3,672,000	1,905,000	1,905,000	55,572,000		0	4,698,000	3,000,000	4,091,000	2,951,000	3,289,000	2,973,000	2,818,000	7,406,000	31,226,000	104,000	104,000	104,000	104,000	104,000
Cost Sharing	Existing Users			0	0	0	0	0	0	408,000	0	0	408,000		320,000	0	0	0	0	0	0	0	0	320,000	0	0	0	0	0
st Allocation	Future Users			100%	100%	100%	100%	100%	100%	%06	100%	100%			%0	100%	100%	100%	100%	100%	100%	100%	100%		100%	100%	100%	100%	100%
Suggested Cost Allocation	Existing Users			%0	%0	%0	%0	%0	%0	10%	%0	%0			100%	%0	%0	%0	%0	%0	%0	%0	%0		%0	%0	%0	%0	%0
	Construction Trigger	(EDU)		2,200 EDUs	4,900 EDUs	10,900 EDUs	2,900 EDUs	As Development Occurs	6,300 EDUs	Immediate	With Well 43	Immediate			With Well 16	With Well 43	Immediate	As Development Occurs	As Development Occurs	Immediate	As Development Occurs	As Development Occurs	With Well 43		As Development Occurs	As Development Occurs	As Development Occurs	As Development Occurs	As Development Occurs
	Improvement Horizon			Buildout	Buildout	Buildout	Buildout	Buildout	Buildout	Five-Year	Buildout	Five-Year			Buildout	Buildout	Five-Year	Buildout	Buildout	Five-Year	Buildout	Buildout	Buildout		Five-Year	Five-Year	Five-Year	Buildout	+1.001111.0
Capital Improv.	Costs <sup>2,3</sup>	(\$)		6,190,000	13,331,000	4,952,000	11,427,000	5,714,000	6,476,000	4,080,000	1,905,000	1,905,000	55,980,000		320,000	4,698,000	3,000,000	4,091,000	2,951,000	3,289,000	2,973,000	2,818,000	7,406,000	31,546,000	104,000	104,000	104,000	104,000	000
Estimated Const.		(\$)		5,382,000	11,592,000	4,306,000	9,936,000	4,968,000	5,631,000	ı	1,656,000	ı	43,471,000		ı	4,085,000	1	3,557,000	2,566,000	2,860,000	2,585,000	2,450,000	6,440,000	24,543,000	000'06	000'06	000'06	000'06	000
Baseline Constr. E		(\$)		4,485,000	9,660,000	3,588,000	8,280,000	4,140,000	4,692,000		1,380,000	1	36,225,000		1	3,404,000	1	2,964,000	2,138,000	2,383,000	2,154,000	2,041,000	5,366,000	20,450,000	75,000	75,000	75,000	75,000	75 000
	Infr. Cost	(\$)	_	4,485,000	9,660,000	3,588,000	8,280,000	4,140,000	4,692,000	ı	1,380,000	ı	rovement Costs		ı	3,403,097	ı	2,963,680	2,137,782	2,382,814	2,153,351	2,040,427	5,365,545	rovement Costs	75,000	75,000	75,000	75,000	75,000
Infrastructure Costs	Recommended Capacity		(MG)	3.25	7.00	2.60	6.00	3.00	3.40	2.10	1.00	1.00	Total Reservoir Improvement Costs	(gpm)	1,500	009'6	11,920	8,000	5,200	6,000	5,250	4,890	17,500	   Total Pump Station Improvement Costs 					
New/	Replace			New	New	New	New	New	New	Replace	New	New			New	New	New	New	New	New	New	New	New	- <b>-</b> -	New	New	New	New	New
	Location		ements	Approx. 1,100' sw/o the intersection of Jurupa Ave and Alder Ave	Reservoir 4-3 site	Lytle Creek Ranch Development, approx. 1,000' ne/o reservoir 5-1 site	Reservoir 6-2 site	Approx. 1,100' e/o the intersection of Citrus Avenue and Segovia Ave	Intersection of Clearwater Pkwy and Glen Helen Pkwy	Existing Z8 Tank Site		Existing Lord Ranch Facility		nts	Existing Pump Station 2-1 site	Existing Pump Station 4-2 site	Lord Ranch Facility	Lytle Creek Ranch development, approx. 2,200' ne/o reservoir 4-3 site	Lytle Creek ranch development, approx. 1,000' ne/o reservoir 5-1 site	Existing Pump Station 7-1 site	Lytle Creek Ranch development, approx. 1,500' ne/o reservoir 6-2 site	Intersection of Clearwater Pkwy and Glen Helen Pkwy	Intersection of 16th St and Pennsylvania Ave	Pressure Reducing Valve Improvements	Sierra Ave, approx. 1,000' n/o Casa Grande Dr	Coyote Canyon Rd, approx. 300' ne/o Hawk Ridge Ave	Lytle Creek Ranch Development	Intersection of Terra Vista Dr and Cypress Ave	Intersection of Terra Vista Dr and
	Pressure Zone		Storage Reservoir Improvements	ю	4	Ŋ	9	9	7	∞	ı	ı		Pump Station Improvements	3	4	4	Ŋ	9	7	7	<b>∞</b>	1	Jucing Valve I	7B-6	78-6	7-78	7-78	1
	Improv. No.		Storage Res	Z3-R3-4	Z4-R4-4	Z5-R5-4	Z6-R6-5	Z6-R6-6	Z7-R7-5	Z8-R8-3	BH-AER	LR-R3-5		Pump Statio	Z3-PS2-1	Z4-PS4-2	Z4-PS4-3	Z5-PS5-3	Z6-PS6-3	Z7-PS7-2	Z7-PS7-3	Z8-PS8-3	BH-PS	Pressure Rec	Z6-PRV1	Z6-PRV2	Z7-PRV1	Z7-PRV2	77 DDV/2

Table 8.4 Capital Improvement Costs - Storage Reservoirs, Pump Stations, Pressure Reducing Valves

3/7/2019 **PRELIMINARY** 55,572,000 31,226,000 87,422,000 **Future Users** 624,000 104,000 624,000 Cost Sharing **Existing Users** 408,000 728,000 0 0 0 Existing Users Future Users **Suggested Cost Allocation** 100% % **Construction Trigger** Immediate Improvement Horizon Five-Year Capital Improv. Costs<sup>2,3</sup> 55,980,000 31,546,000 88,150,000 624,000 104,000 624,000 43,471,000 24,543,000 68,554,000 540,000 540,000 90,000 Costs<sup>1</sup> Baseline Constr. 36,225,000 20,450,000 57,125,000 450,000 450,000 75,000 **Total Improvement Costs Storage Reservoir Improvements** Pump Station Improvements **Pressure Reducing Valve Improvements Fotal Pressure Reducing Valve Improvement Costs** 75,000 Infrastructure Costs ded Capacity New/ Replace New Lytle Creek Rd, nw/o Monarch Hills Development Pressure Zone **Total Improvement Costs** 7-7B Improv. No. Z7-PRV4

1. Baseline construction costs plus 20% to account for unforeseen events and unknown conditions.

2. Estimated construction costs plus 15% to cover other costs including: engineering design, project administration (developer and District staff), construction management and inspection, and legal costs.

3. Costs for improvements shown with only Capital Improvement Cost are based on information provided by WVWD staff.

# Table 8.5 Capital Improvement Costs - OPR WFF Expansion

Water Facilities Master Plan West Valley Water District

Capital Improvement **PRELIMINARY** 20,160,000 71,747,120 15,719,030 18,276,187 1,376,530 1,552,386 3,000,000 2,174,933 1,722,652 3,342,534 3,216,401 525,960 680,507 Cost Administration Building (7,000 s.f.) Raw Water Control Structures **Total Improvement Cost** Influent Blending Ponds Membrane Pumping **Equalization Storage Chlorine Contact GAC Contactors** Microfiltration **Expansion Item** Sludge Ponds **Civil Costs** Chemicals  $\geq$ Item No. 10 11 6  $\mathcal{C}$ 2 9 /  $\infty$ 7 4

Notes:

2/19/2019

1. Capital Improvement costs extracted from opinion of probable costs prepared by

Carollo Engineers March 19, 2019 and provided by District staff.

2. Estimate is based on a number of assumptions and limited information, approximate accuracy is +50% to -30%.

Table 8.6 Capital Improvement Costs - Supply

126,104,320 **PRELIMINARY** 71,747,120 54,357,200 54,357,200 71,747,120 71,747,120 1,441,800 11,312,000 1,143,400 3,000,000 3,000,000 3,000,000 3,289,000 2,921,000 2,921,000 1,849,400 1,867,000 1,738,200 1,441,800 1,441,800 3,000,000 3,550,000 1,533,800 110,000 50,000 **Cost Sharing Existing Users** 50,388,800 50,388,800 6,135,200 50,388,800 7,397,600 5,767,200 120,000 7,468,000 6,952,800 4,573,600 5,767,200 5,767,200 440,000 0 0 0 0 0 0 **Existing Users** Future Users **Suggested Cost Allocation** 100% 20% 100% 100% 100% 100% 100% 100% 20% 20% 20% 20% 20% 20% 20% 20% 80% 80% 80% 80% 80% 80% % Construction Trigger 2020 2020 2021 2022 2022 2023 2024 2024 2029 2029 2030 2022 2021 2023 Five-Year Buildout Buildout Buildout Buildout Five-Year Capital Improv. Costs<sup>2</sup> 176,493,120 104,746,000 71,747,120 104,746,000 71,747,120 71,747,120 11,312,000 3,000,000 2,921,000 3,000,000 3,000,000 3,000,000 3,289,000 3,550,000 2,921,000 7,209,000 5,717,000 7,209,000 7,209,000 150,000 9,247,000 9,335,000 8,691,000 550,000 50,000 **Estimated Const.** 0 0 0 0 eline Constr. Costs 0 0 **Total Supply Improvement Costs** 104,739,278 Surface Water Supply Improvements **Groundwater Supply Improvements** 3,000,000 11,311,441 5,716,015 7,208,559 7,208,559 3,000,000 3,288,359 3,550,000 2,920,864 9,334,214 7,208,559 3,000,000 3,000,000 2,920,864 9,246,213 8,690,777 Fotal Infr. Cost 550,000 50,000 Subtotal - Groundwater Supply Improvements face Water Supply Improvements Treatment Cost 3,288,359 3,550,000 11,311,441 5,716,015 9,334,214 7,208,559 8,690,777 5,716,015 7,208,559 2,920,864 2,920,864 150,000 9,246,213 7,668,839 550,000 50,000 1 • Infrastructure Costs struction Cost 3,000,000 3,000,000 7,208,559 3,000,000 16.0 mgd expansion Subtotal - Sur 2,000 1,500 3,500 4,000 2,200 1,500 1,500 1,500 3,500 3,500 3,500 2,100 2,400 2,700 3,000 2,000 2,000 Groundwater Supply Improvements<sup>3,4</sup> Surface Water Supply Improvements<sup>5</sup> New/ Rehabilitate New **Total Improvement Cost** Improv. No. **OPR WFF** W29A W18A W34B W22A W50 W16 W40 W43 W44 W46 W39 W41 W45 W8A **W36** W42 **W52** W51 8 Y Y

Notes:

1. Baseline construction costs plus 25% to account for unforeseen events and unknown conditions.

2. Estimated construction costs plus 22% to cover other costs including: engineering design, project administration (developer and District staff), construction management and inspection, and legal costs.

3. Costs and contingencies shown provided by Kleinfelder, Inc.

4. Costs shown for new wells include both construction costs and costs for any potential treatment identified

5. Costs shown prepared by Carollo Engineers and provided by District staff April 1, 2019.

Table 8.7 5-year Improvement Phasing
Water Facilities Master Plan
West Valley Water District

Part							Fiscal Year Improvement Phasing	nt Phasing			
The control co	용	Project Name	Project Description	FY 2018/19	FY 2019/20	FY 2020/21	FY 2021/22	FY 2022/23	FY 2023/24	Total Impro	ement Cost
### Control of Control	2									Existing Users	Future Users
	Pipeline Impro	vements rements									
	Z2-P1	Bloomington Pipeline Replacement (Phase 4)	Construct new 24-inch transmission main in Eighth St				2,222,000			2,222,000	0
Control by protein b	Z2-P2	Bloomington Pipeline Replacement (Phase 4)	Construct new 8-inch pipelines in Eighth St				850,000			850,000	0
Control of the cont	Z2-P3	Bloomington Pipeline Replacement (Phase 5)	Replace existing 4-inch and 6-inch pipelines with new 8-inch pipelines in Ninth St					650,000		650,000	0
Automotive behinders by the control of the contro	Z2-P4	Bloomington Pipeline Replacement (Phase 3)	Replace existing 4-inch and 8-inch pipelines with new 8-inch pipelines in Tenth St	000'029						000'059	0
This contains to protection by parts   Misked stability objectives by 10 median to protection by parts   Misked stability objectives by 10 median to protection by parts   Misked stability objectives by 10 median to protection by 10 median by	Z2-P5	Bloomington Pipeline Replacement (Phase 3)	Construct new 8-inch pipelines in Eleventh St	400,000						400,000	0
Particular   Control treat Planta (Planta Planta	Z2-P6	Bloomington Pipeline Replacement (Phase 3)	Replace existing 6-inch pipelines with new 12-inch pipelines in Maple St	000'059						650,000	0
Exercise   Processing   Proce	Z2-P7	Zone 2 Santa Ana Transmission	Construct new 12-inch transmission main on Santa Ana Ave			380,000				380,000	0
Zone 2 Hold Cossing         Control 2 Hold Cossing         Control 2 Hold Cossing         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000         3 54,000	Z2-P8	Zone 2 Santa Ana Transmission	Replace existing 12-inch pipelines with new 20-inch transmission main on Santa Ana Ave					3,794,000		3,794,000	0
2002 5100 Cross transfer	Z2-P9	Zone 2 I-10 Crossing	Construct 24-inch transmission main crossing I-210			304,000				304,000	0
2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000	Z2-P9C	Zone 2 I-10 Crossing	Casing for pipeline crossing I-210			585,000				585,000	0
2000 S. Santa Anal Transcription   Construct now 15 Anal Anal Anal Anal Anal Anal Anal Anal	Z3-P4	Zone 3 Santa Ana Transmission	Replace existing 4-inch, 6-inch, and 12-inch pipelines with new 12-inch pipeline in Santa Ana Ave				1,484,000			1,484,000	0
Available beginter registry and 1-beginter with 5 min properties.         Available beginter registry and 1-beginter with 5 min properties.         Available beginter registry and 1-beginter with 5 min properties.         Available beginter registry and 1-beginter with 5 min properties.         Available beginter registry and 1-beginter with 5 min properties.         Available beginter registry and 1-beginter with 5 min properties.         Available beginter registry and 1-beginter in valid 9 bit of 2000 or 2-beginter in valid 9 bit of 2000 or 2-beginter in valid 9 bit of 2000 or 2-beginter in valid 9 bit of 2-beginter	Z3-P5	Zone 3 Santa Ana Transmission	Construct new 16-inch transmission main in Santa Ana Ave			462,000				462,000	0
2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000   2000	Z3-P6	Valley Blvd Pipeline Replacements	Replace existing 2-inch, 4-inch, and 6-inch pipelines with 8-inch pipelines					517,000		517,000	0
Zone 3 Hydraulic Reliability         Construct a new 12-inch pipeline in Cattury Avea         5.000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000         1.05,000	Z3-P7	Valley Blvd Pipeline Replacements	Replace existing 4-inch and 6-inch pipelines with 8-inch pipelines					1,040,000		1,040,000	0
Zone 3 H yidaulic fielability         Construct a new 16-inch pipeline in Cartus Ave         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,500         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600         143,600	Z3-P8	Zone 3 Hydraulic Reliability	Construct a new 12-inch pipeline in Valley Blvd	50,000	160,000					210,000	0
Enter   Construct a new 18-inch transmission   Construc	Z3A-P1	Zone 3A Hydraulic Reliability	Construct a new 10-inch pipeline in Cactus Ave			35,000	108,500			143,500	0
Construct a new 18-hich turner area serial forchtides casing for 1   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,000   1,500,0	Z7-P10	Zone 7 Transmission	Construct a new 18-inch transmission main in Segovia Ln from Sierra Ave to Citrus Ave						2,463,000	0	2,463,000
Bunker Hill Well Field Transmission         From Bunker Hill Seration tank baraction tank be setzing a seration tank be setzing a seration tank be setzing and storate setzing tank baraction tank to existing Purp Sation 3.1750,000         1.756,000         6.117,000         4,664,500         6,010,000         9         0         3,469,000         14,341,530           Zone 8 Reservoir Replacement Lord Render Hill Aeration Tank         Replace existing Zone 8 storate servoir with new 2.1 MG         8,000         72,000         3,600,000         3,600,000         4,664,500         6,001,000         9         3,469,000         14,341,500           Lord Ranch Hail Aeration Tank         Construct a new aeration tank at the Bunker Hill satisfy and Construct a new aeration tank at the Bunker Hill Supply         2,000,000         3,600,000         1,905,000         0         1,905,000         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0 <td>Z7-P16</td> <td>Zone 7 Transmission</td> <td>Construct a new 18-inch transmission main within future ROW from Citrus Ave to Lytle Creek Rd (includes casing for I- 15 crossing)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>1,006,000</td> <td>0</td> <td>1,006,000</td>	Z7-P16	Zone 7 Transmission	Construct a new 18-inch transmission main within future ROW from Citrus Ave to Lytle Creek Rd (includes casing for I- 15 crossing)						1,006,000	0	1,006,000
Bunker Hill Well Field Transmission         From Bunker Hill aeration tank to existing Dump Station 3A         1,750,000         1,766,000         1,766,000         6,117,000         4,664,500         6,001,000         0         3,469,000         14,341,500           Zone 8 Reservoir Replacement         Replace existing Zone 8 storage reservoir with new 2.1 MG         8,000         72,000         400,000         3,600,000         1,905,000         1,905,000         1,905,000         1,905,000         1,905,000         0         1,905,000         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0 </td <td>BH-P1</td> <td>Bunker Hill Well Field Transmission</td> <td>From planned wells 43, 44, 45, and 46 to the Bunker Hill aeration tank</td> <td></td> <td></td> <td>840,000</td> <td></td> <td></td> <td></td> <td>0</td> <td>840,000</td>	BH-P1	Bunker Hill Well Field Transmission	From planned wells 43, 44, 45, and 46 to the Bunker Hill aeration tank			840,000				0	840,000
Subtotal - Capacity Improvements         1,750,000         0         1,750,000         0         1,750,000         0         1,750,000         0         3,469,000         0         3,469,000         14,341,500           Zone Reservoir Replacement         Replace existing Zone 8 storage reservoir with new 2.1 MG aeration reservoir at Lord Ranch Aeration Tank         8,000         72,000         3,600,000         3,600,000         3,600,000         3,600,000         3,600,000         3,600,000         0         1,905,000         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0	BH-P2	Bunker Hill Well Field Transmission	From Bunker Hill aeration tank to existing Pump Station 3A			5,277,000				0	5,277,000
Zone 8 Reservoir Replace existing Zone 8 storage reservoir with new 2.1 MG         8,000         72,000         400,000         3,600,000         3,600,000         400,000         3,600,000         400,000         400,000         400,000         400,000         400,000         400,000         1,905,000         0         0         0         0         0         408,000           Bunker Hill Aeration Tank         Subtotal - Reservoir Improvements         8,000         1,977,000         400,000         3,600,000         0         0         0         0         0         0         0         0         0         0         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000         7,890,000 <t< td=""><td></td><td></td><td>Subtotal - Capacity Improvements</td><td>50,000</td><td>000'091</td><td>,883,0</td><td>664,500</td><td>,001,000</td><td>3,469,00</td><td>14,341,500 23,92</td><td>9,586,000</td></t<>			Subtotal - Capacity Improvements	50,000	000'091	,883,0	664,500	,001,000	3,469,00	14,341,500 23,92	9,586,000
Zone 8 Reservoir Replace existing Zone 8 Storage reservoir with new 2.1 MG         8,000         72,000         3,600,000         3,600,000         400,000         3,600,000         400,000         3,600,000         400,000         400,000         4,000,000         1,905,000         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0	Reservoir Improv	ements									
Lord Ranch Aeration Tank         Construct a new 1.0 MG aeration reservoir at Lord Ranch Facility         1,905,000         0         1,905,000         0         1,905,000         0         0         1,905,000         0         0         0         0         0         0         0         0         0         0         0         0         0         7,890,000           Bunker Hill Aeration Tank         Subtotal - Reservoir Improvements         8,000         1,977,000         4,000,000         3,600,000         0         0         0         0         0         0         0         0         7,890,000	Z8-R8-3	Zone 8 Reservoir Replacement	Replace existing Zone 8 storage reservoir with new 2.1 MG reservoir							408,000	3,672,000
Bunker Hill Aeration Tank         Construct a new aeration tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply         A Manage of Tank at the Bunker Hill supply	LR-R3-5	Lord Ranch Aeration Tank	Construct a new 1.0 MG aeration reservoir at Lord Ranch Facility	1,905,000						0	1,905,000
8,000         1,977,000         400,000         3,600,000         0         1,905,000         0         0         0         0         0         408,000           1,985,000         4,000,000         1,905,000         0         0         0         0         7,890,000	BH-AER	Bunker Hill Aeration Tank	Construct a new aeration tank at the Bunker Hill supply location							0	1,905,000
			Subtotal - Reservoir Improvements	,985,	000	0 1,905,0	0	0	0		7,482,000

Table 8.7 5-year Improvement Phasing
Water Facilities Master Plan
West Valley Water District

										FRELIMINARI
						Fiscal Year Improvement Phasing	nt Phasing			
G G	Project Name	Project Description	FY 2018/19	FY 2019/20	FY 2020/21	FY 2021/22	FY 2022/23	FY 2023/24	Total Improvement Cost	ement Cost
≘			Existing Future Users Users	Existing Future Users (S) (S)	Existing Future Users Users	Existing Future Users Users	Existing Future Users Users	Existing Future Users Users	Existing Users	Future Users
Pump Station Improvements	ovements									
Z4-PS4-3	Lord Ranch Pump Station	Construct new Pressure Zone 4 pump station at Lord Ranch Facility	3,000,000						0	3,000,000
Z7-PS7-2	New Zone 7 Pump Station	Construct a new pump station adjacent to existing PS 7-1			0 4,091,000				0	4,091,000
BH-PS	New Bunker Hill supply Pump Station	Construct a new pump station at the Bunker Hill supply location			0 7,406,000				0	7,406,000
		Subtotal - Pump Station Improvements	000'000'E 0	0 0	0 11,497,000 11,497,000	0 0	0 0	0 0	0 14,497,000	14,497,000
Pressure Reducing \	Pressure Reducing Valves Improvements									
Z6-PRV1	Zone 6 PRV	Construct new pressure reducing station on Sierra Ave	104,000						0	104,000
Z6-PRV2	Zone 6 PRV	Construct new pressure reducing station on Coyote Canyon Rd	104,000						0	104,000
Z7-PRV1	Zone 7 PRV	Construct new pressure reducing station within planned Lytle Creek Ranch development	104,000						0	104,000
Z7-PRV4	Zone 7 PRV	Construct new pressure reducing station on Lytle Creek Rd, northwest of planned Monarch Hills Development	104,000						0	104,000
		Subtotal - Pressure Reducing Valves Improvements	0 416,000 416,000	0 0	0 0	0 0	0 0	0 0	0 416,000	416,000
Supply Improvements	nts									
W41	Well 41 Rehabilitation	Implement ion-exchange treatment for nitrate	440,000 110,000						440,000	110,000
W39	Well 39 Rehabilitation	Existing well drilled but not equipped		7,468,000 1,867,000					7,468,000	1,867,000
W7	Well 7 Rehabilitation	Existing well blind flanged			20,000				0	20,000
W8A	Well 8A Rehabilitation	Implement arsenic removal			3,289,000				0	3,289,000
W36	Well 36 Rehabilitation	Implement arsenic removal					3,550,000		0	3,550,000
W18A	Well 18A Rehabilitation	Implement ion-exchange treatment for nitrate		6,135,200 1,533,800					6,135,200	1,533,800
OPR WFF		Design and Construct OPR WFF expansion	200,000	71,747,120					0	72,247,120
W43		Construct new well			3,000,000				0	3,000,000
W44		Construct new well				3,000,000			0	3,000,000
W45		Construct new well					3,000,000		0	3,000,000
W46		Construct new well						3,000,000	0	3,000,000
		Subtotal - Supply Improvements	440,000 610,000 1,050,000	13,603,200 75,147,920 88,751,120	000'628'9 0	000'000'E 0	00°052°9 00°052°9	000'000'E 0	14,043,200 94 108,690,120	94,646,920 0,120
Other Currently Planned Projects	nned Projects									
Property Acquisit	Property Acquisition for Reservoir R3-4 (1.5 acres)	Purchase land for future reservoir R3-4				523,000			0	523,000
Property Acquisit	Property Acquisition for Reservoir R6-6 (1.5 acres)	Purchase land for future reservoir R6-6					523,000		0	523,000
Property Acquisit	Property Acquisition for Bunker Hill Supply	Purchase land for future Bunker Hill wells, pump station, and aeration reservoir		1,300,000					0	1,300,000
R7-5 Reservoir Site Investigation	ite Investigation	Conduct site investigation for future reservoir R7-5		000'65					0	29,000
Grading, Fencing	Grading, Fencing, and Paving at Lord Ranch Facility	Grade, pave, and erect fencing at Lord Ranch facility				700,000			700,000	0

Table 8.7 5-year Improvement Phasing
Water Facilities Master Plan
West Valley Water District

							Fi	Fiscal Year Improvement Phasing	ement Phasing					
CIP Project Name	Project Description		FY 2018/19	FY 2019/20	C	FY 2020/21		FY 2021/22	FY	FY 2022/23	FY 203	FY 2023/24	Total Improvement Cost	ement Cost
2		E E	ng Fr.	gu s	e v	ng F.		품 그	Ä ¬	Future Users	Existing Users	Future Users	Existing Users	Future Users
Sierra Ave, Developer Pipeline Capacity Increase	Increase size of development required pipe to accommodate additional future development		(5) (5)	(s)	(\$)	(5)	(3)	(s)	(S)	(s)	(\$)	(s)	( <u>S</u> )	120,000
Cedar Pl, Developer Pipeline Capacity Increase	Increase size of development required pipe to accommodate additional future development	mmodate	84,000										0	84,000
Well 54 Deaeration Tank	Construct deaeration tank at existing well 54		330,000										330,000	0
	Subtotal - Other Currently Planned Projects		330,000 204,000 534,000	0 1,359,000	1,359,000	0 0		700,000 523,000 1,223,000	0	523,000 523,000	0	0 0	1,030,000	2,609,000
Total Improvement Costs														
	Existing/Future Users Fiscal Y	Fiscal Year Total \$2,5	\$2,528,000 \$6,207,000	\$14,163,200 \$80,106,920		\$1,766,000 \$25,858,000		\$5,364,500 \$3,523,000	\$6,001,000	\$7,073,000	\$0	\$6,469,000	•	
	Cumulat	Cumulative Total \$2,5	\$2,528,000 \$6,207,000	\$16,691,200 \$86,313,920		\$18,457,200 \$112,171,920		\$23,821,700 \$115,694,920	920 \$29,822,700	\$122,767,920	\$29,822,700	\$129,236,920	\$29,822,700	\$129,236,920
	Combined Project Costs Fiscal Year Total	ar Total	\$8,735,000	\$94,270,120	20	\$27,624,000		\$8,887,500	\$13,	\$13,074,000	\$6,46	\$6,469,000	1	
AKEL	Cumulative Total	ve Total	\$8,735,000	\$103,005,120	.20	\$130,629,120		\$139,516,620	\$152	\$152,590,620	\$159,0	\$159,059,620	\$159,059,620	9,620
ENGINEERING GROUP, INC.														4/5/2019

constructed in the near-term period. This table also includes currently planned projects identified by District staff that support the existing water system, such as land acquisition and site development. The projects included in this 5-year capital improvement program are based on current District priorities and may not include all improvements identified for construction within the 5-year development horizon.

### 8.3.4 Existing and Buildout EDUs

The calculation of total EDUs, under existing and future conditions, enables the District to effectively plan for capital improvement funding and to appropriately adjust water rates and impact fees as necessary. The calculation methodology for determining the existing, 5-year, and buildout EDU totals is briefly summarized as follows:

- Existing: Consistent with the 2012 WMP the existing number of EDUs were based on meter sizes of existing customers; the conversion factors utilized in determining the existing EDUs are summarized on Table 8.8. It should be noted the existing EDUs were based on 2016 account information provided by District staff.
- **5-year Development:** The additional EDUs added through the 5-year development horizon were based on development information summarized in Table 2.5.
- Buildout Development: The additional EDUs added through the Buildout development
  horizon were determined based on demand projections summarized in a previous chapter.
  The demand was converted to EDUs using a factor of 670 gpd/EDU, which is based on
  meter sizes and quantities, as provided by District staff, and using industry standard
  conversion factors.

The total number of EDUs at the existing, 5-year, and Buildout development horizons are summarized on Table 8.9.

**Table 8.8 Water Meter EDUs** 

**PRELIMINARY** 

Meter Size 5/8" & 3/4" Po	Meter Type  positive Displacement Type	Safe Maximum Operating Flow <sup>1,2</sup> (gpm) 30	<b>EDU</b>
5/8" & 3/4" Pc	ositive Displacement Type	30	1.0
1" PC	ositive Displacement Type	50	1.7
1-1/2" Pc	ositive Displacement Type	100	3.3
2"	Turbine Type	160	5.3
3"	Turbine Type	350	11.7
4"	Turbine Type	630	20.0
6"	Turbine Type	1,300	41.7
8"	Turbine Type	1,800	60.0
AKEL ENGINEERING GROUP, IN	c.		4/2/2018

Notes:

- 1: Source: WVWD 2012 Master Plan
- 2. Flows are based on safe maximum operating flow per AWWA standards C701-15

**Table 8.9 EDUs by Pressure Zone** 

**PRELIMINARY** 

Pressure Zone	Existing <sup>1</sup> (2016)	Total, 5-Year Projection <sup>2</sup> (2022)	Total, Buildout <sup>3</sup> (2055)
Zone 2	3,479	3,679	6,317
Zone 3	6,975	7,379	11,115
Zone 3A	2,120	2,170	2,227
Zone 4	3,209	3,269	3,675
Zone 5	3,232	4,232	4,522
Zone 6	5,051	6,858	10,506
Zone 7	4,199	6,611	10,293
Zone 8	91	481	1,081
Total	28,356	34,679	49,736
AKE	JP, INC.		4/5/2019

Notes:

1. Existing EDUs based on 2016 account information provided by WVWD staff.

- 2. Includes additional EDUs based on 5-year growth information provided by WVWD staff.
- 3. Includes additional EDUs based on demand projections, assuming 670 gpd/EDU

# **West Valley Water District**

# **APPENDICES**

# **West Valley Water District**

# **APPENDIX A**

**Demand Unit Factor Comparison** 

**Table 1 Average Daily Water Use Unit Factors** 

**PRELIMINARY** 

2012 V	Vater Master P	lan¹			2019 Water Facilities	Master Plan
Land Use Designation	Development Density (du/ac)	Persons/du	Wate		Land Use Designation <sup>3</sup>	Water Use
Residential	(uu/ac)		(gpm/ac)	(gpd/ac)		(gpd/ac)
Estate Residential	1	5.9	0.82	1,181		
Low Density	3	3.8	1.58	2,275		
Rural Residential	2	5.0	1.39	2,002	Residential 2	990
Medium Density	4	3.8	2.10	3,024		
Single Family	4	3.8	2.00	2,880		
Planned Community	4.5	3.2	1.75	2,520		
Medium High Density	9	2.1	2.62	3,773	Residential 6	2,650
Medium Density	9	2.1	2.62	3,773		
High Density	12	1.7	2.83	4,075	Residential 12	4,580
Very High Density	Not	included in 20.	12 WMP		Residential 21	5,630
Regional Mixed Use	-	-	2.62	3,773		
on-Residential						
Office	-	-	2.43	3,500	Office	1,410
Community Commercial	-	-	2.43	3,500	Commercial	1,800
Commercial Recreation	-	-	2.08	3,000	Retail	1,890
Industrial Park	-	-	1.39	2,000	Industrial	1,000
General Industrial	-	-	2.08	3,000	Heavy Industrial	1,530
Light Industrial	-	-	1.39	2,000	Light Industrial	500
Landfill	-	-	1.00	1,440		
School	-	-	2.43	3,500	Educational	1,790
Institutional	Not	included in 20.	12 WMP		Institutional	1,410
Public Facility	Not	included in 20.	12 WMP		Public Facility	230
Park	-	-	2.43	3,500	Landscape Irrigation	2,690
Golf Course	-	-	2.43	3,500		
Open Space	-	-	0.00	0		
Agricultural	-	-	0.00	0		
Public Utility Corridor (Greenbelt)	-	-	2.43	3,500		
Right of way	-	-	0.00	0		
Wells, Reservoirs, Energy	-	-	1.39	2,000	Utilities	10

Notes:

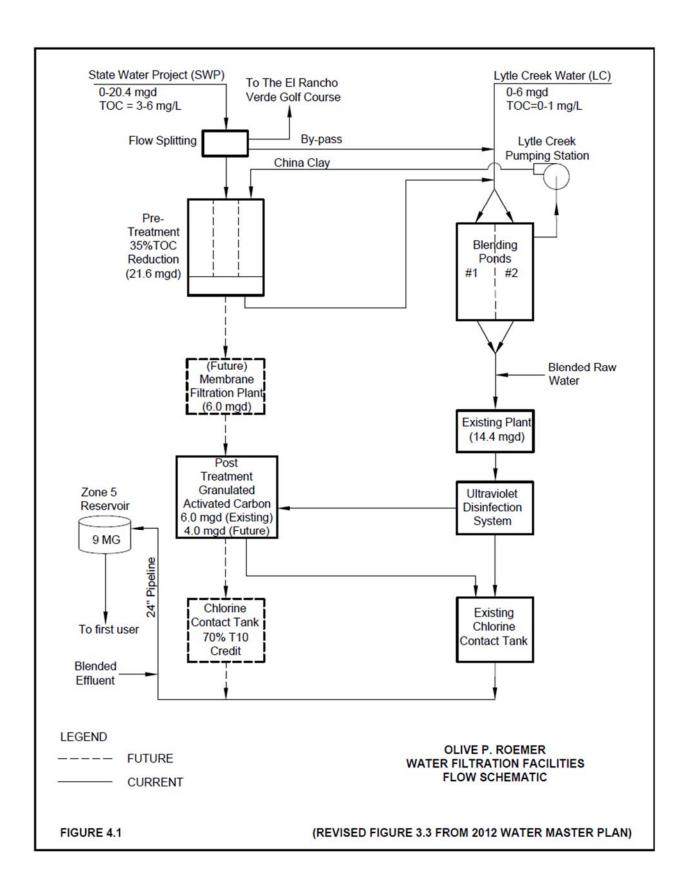
1. Land use designations and water use extracted from WVWD 2012 Water Master Plan, Table 5.1.

- 2. Residential water use factors calculated assuming 200 gallons per person per day.
- 3. Land use designations extracted from parcel database provided by WVWD staff July 5, 2017.
- 4. Water use factors calculated based on existing development and 2016 consumption records normalized to 2014 production minus 10%.

# **West Valley Water District**

# **APPENDIX B**

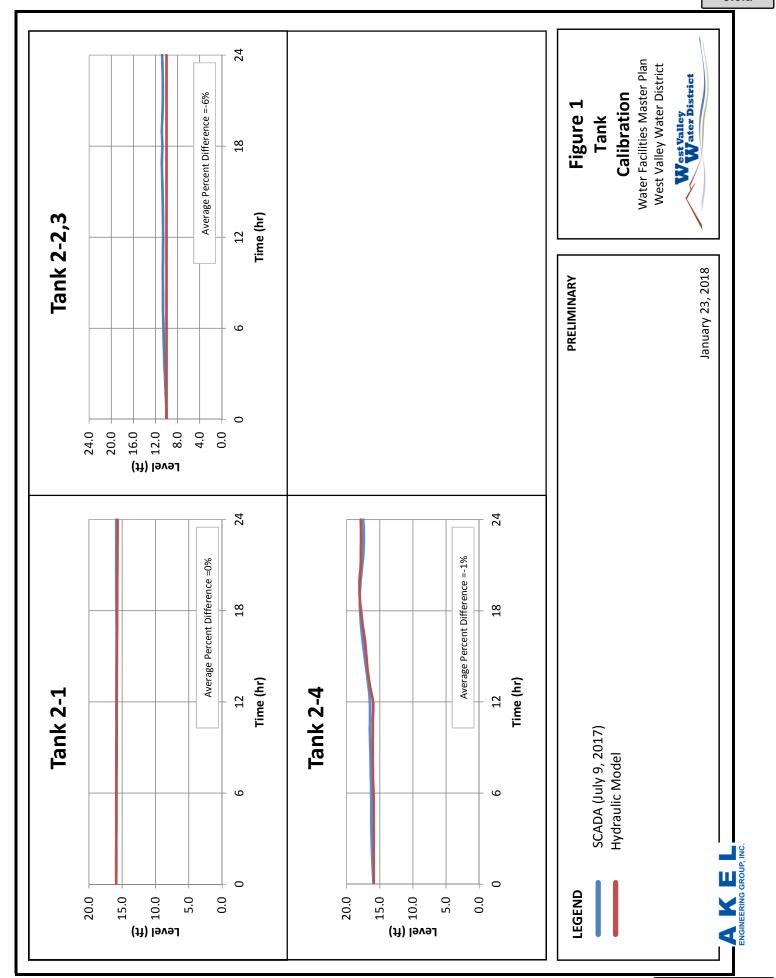
**OPR Facility Flow Schematic** 

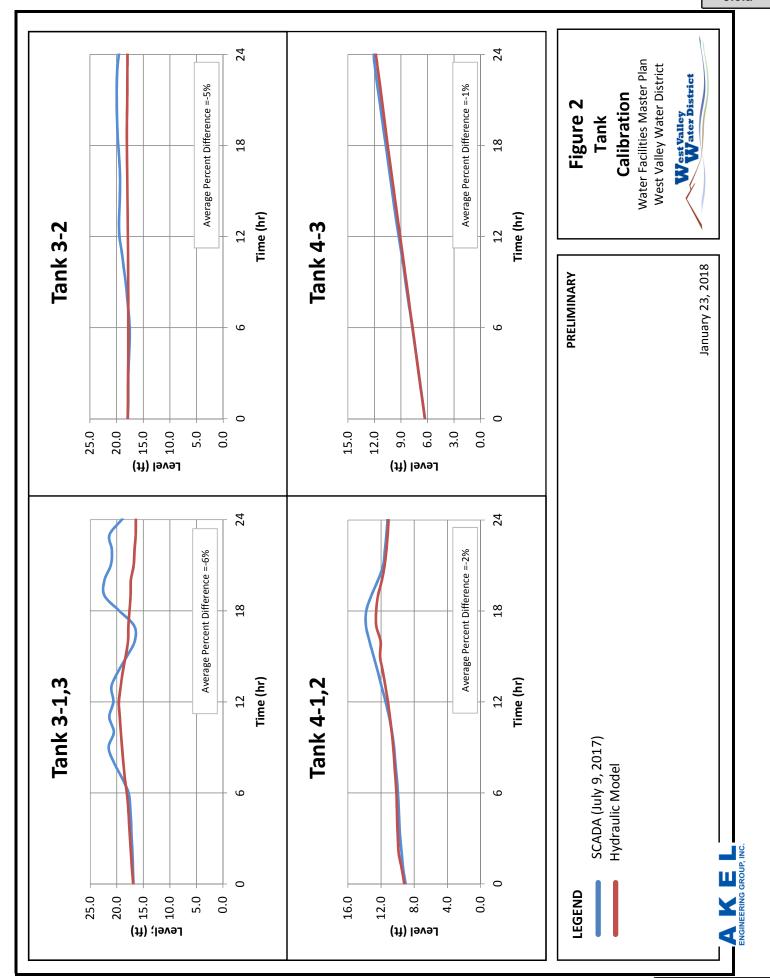


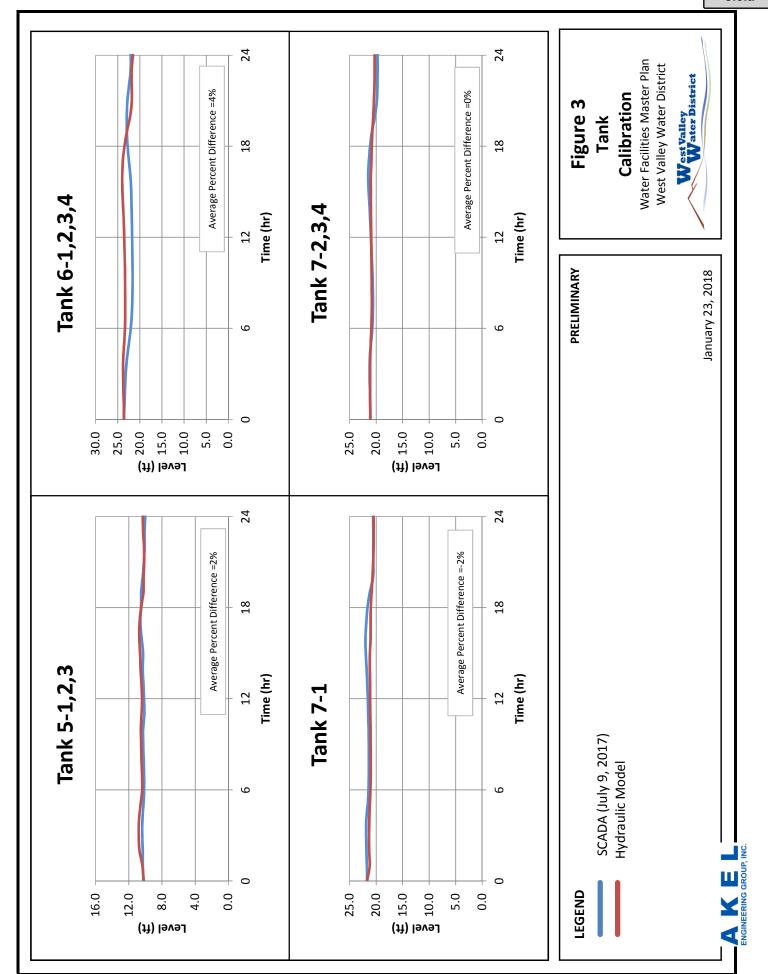
# **West Valley Water District**

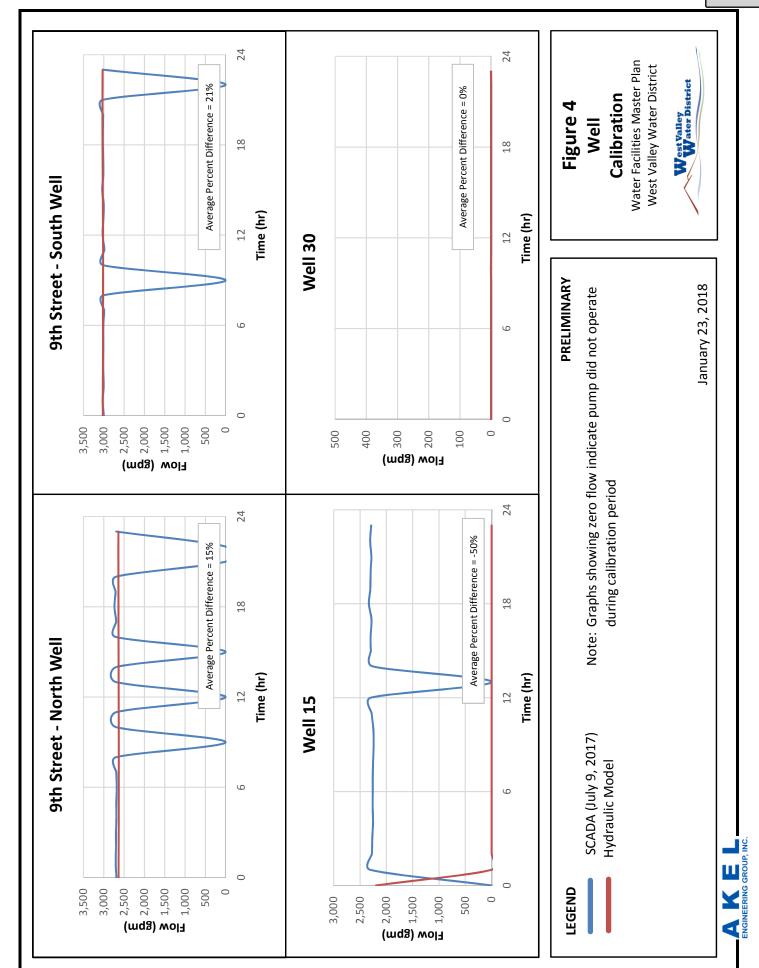
# **APPENDIX C**

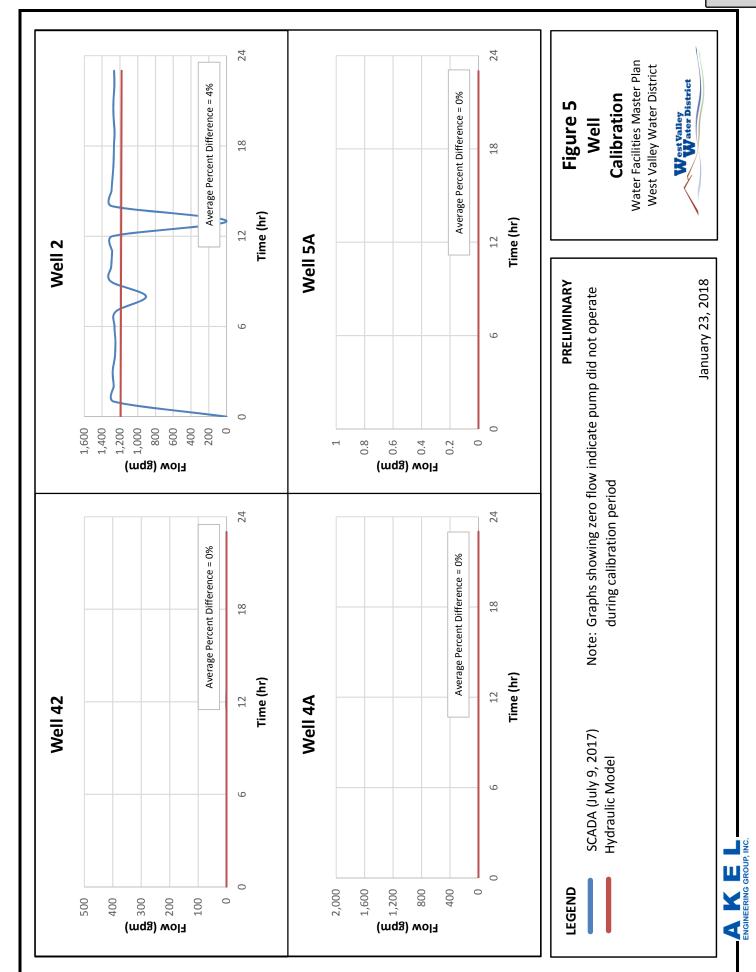
Hydraulic Model Calibration

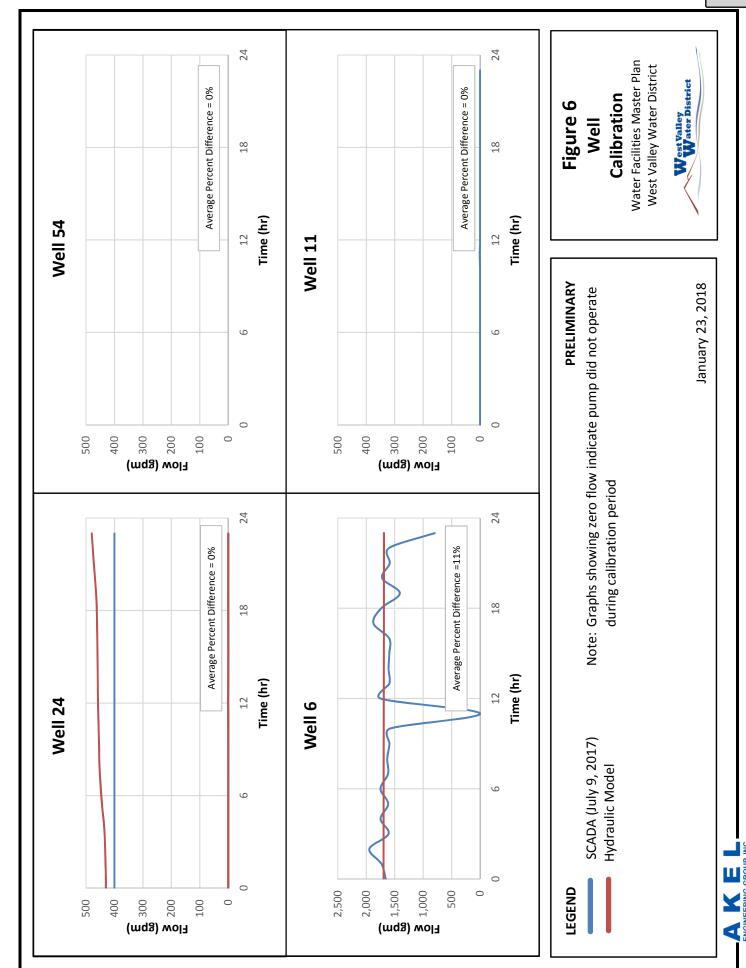


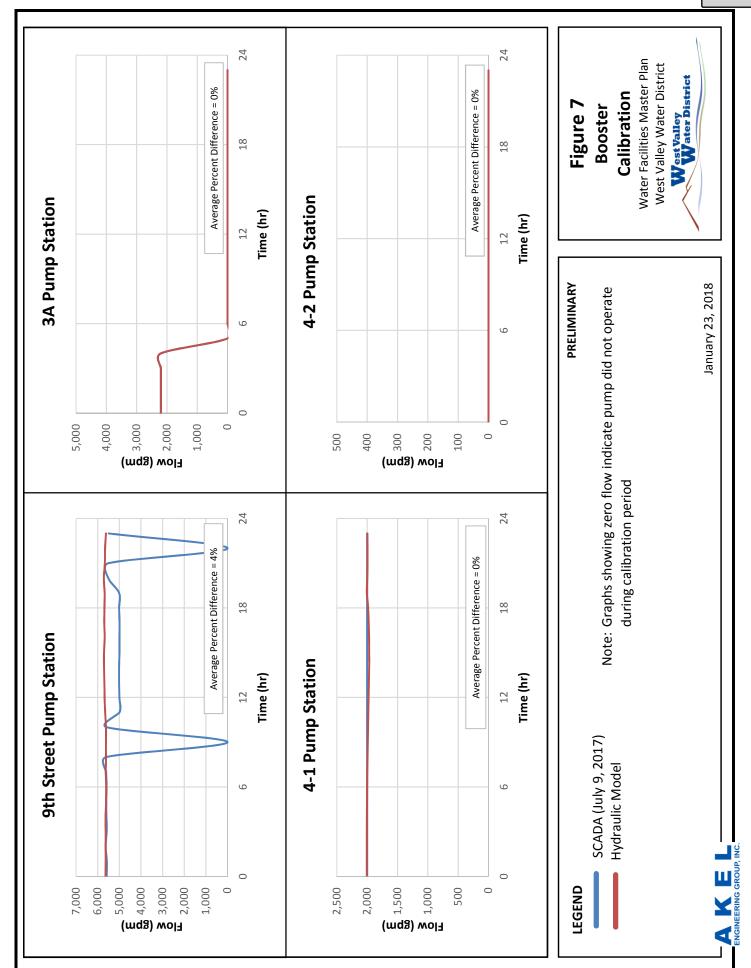


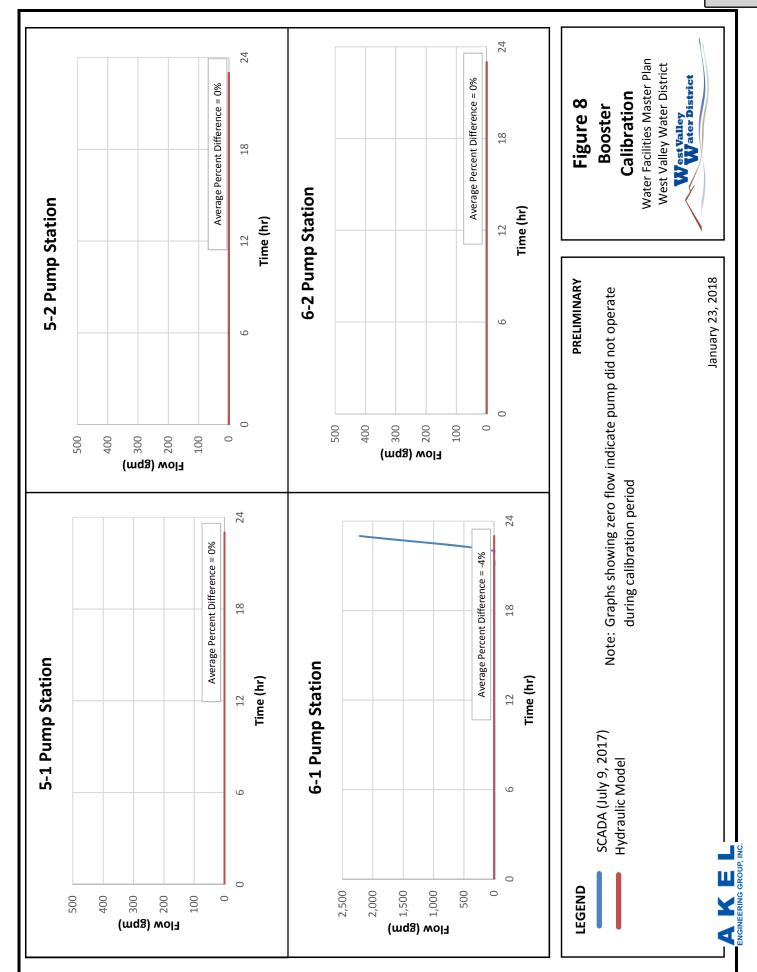


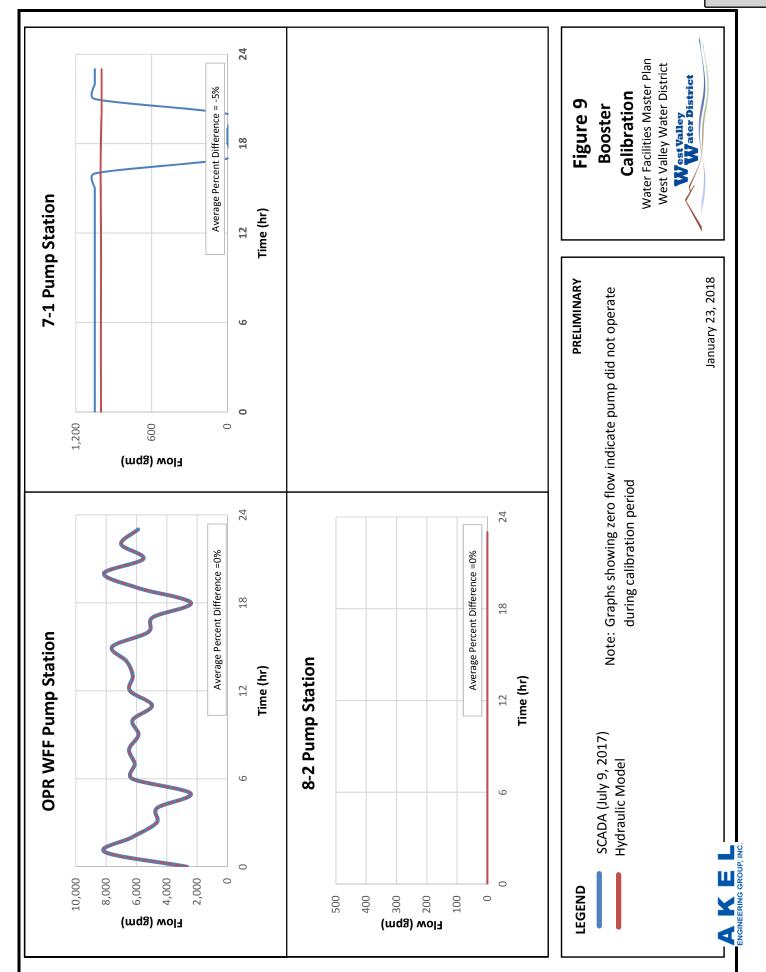














### BOARD OF DIRECTORS ENGINEERING AND PLANNING COMMITTEE STAFF REPORT

DATE: October 9, 2019

TO: Engineering and Planning Committee FROM: Clarence Mansell Jr., General Manager

SUBJECT: CONSIDER NOTICE OF COMPLETION RECORDATION FOR THE

RESERVOIR R2-3 SITE IMPROVEMENTS AND MODIFICATIONS

**PROJECT** 

### **BACKGROUND:**

The West Valley Water District ("District") is proposing to construct site improvements and modifications to the Reservoir R2-3 site due to erosion caused by stormwater runoff. The existing access road is used daily by the District's staff and is in need of repair. To eliminate future erosion problems, the District initiated a project that includes the construction of a paved access road, site pavement, fencing, catch basin, and v-ditch.

The District has filed a claim with our insurance company to see if they will be providing funds for the project. The insurance company has reviewed the documentation and funding will be provided.

### **DISCUSSION:**

On September 18, 2018, the District entered into a contract with G.M. Sager Construction Company, Inc. ("G.M. Sager") for the construction of the Reservoir R2-3 Site Improvements and Modifications Capital Improvement Project. Since their contract was established, G.M. Sager has successfully conducted the scope of work and provided deliverables as stated in the contract.

The District's Project Manager on the project, Rosa M. Gutierrez, P.E., has confirmed the substantial completion of the Reservoir R2-3 Site Improvements and Modifications Capital Improvement Project. Attached as **Exhibit A** is a copy of the certificate of substantial completion.

### **FISCAL IMPACT:**

No fiscal impact.

### STAFF RECOMMENDATION:

It is recommended that the Engineering, Operations, and Planning Committee authorize staff to file the Notice of Completion for the project.

Respectfully Submitted,

Clarence C. Mansellfr.

Clarence Mansell Jr, General Manager

RMG:ce

### **ATTACHMENT(S)**:

1. Exhibit A - Notice of Substantial Completion for Reservoir R2-3 Site Improvements and Modifications

### **EXHIBIT A**



Michael Baker Inte 3536 Concours,

Ontario, CA 91764 (909) 974-4900 Fax (909) 390-9817

3.d.a

### CERTIFICATE OF SUBSTANTIAL COMPLETION

Date:	8/7/19	Project Name:	Reservoir 2-3 Site Improvements
To:	West Valley Water District	Project No.:	
	855 W. Base Line Road Rialto, CA 92377	Contractor:	GM Sager Construction Company Inc
Attn:	Rosa Gutierrez	MBI JN.:	170108
From:	Patrick Hanify		

### **DEFINITION**

The date of Substantial Completion of the Work or designated portion thereof is the Date certified by the Construction Manager when construction is sufficiently complete, in accordance with the Contract Documents, so the Owner can occupy or utilize the Work or designated portion thereof for the use for which it is intended, as expressed in the Contract Documents.

PROJECT OR DESIGNATED PORTION THEREOF, INCLUDED IN THIS CERTIFICATE:

### **RESERVOIR 2-3 SITE IMPROVEMENTS**

The Work performed under this Contract has been reviewed and found to be substantially complete. The date of Substantial Completion of the Project or portion thereof designated above is hereby established as <u>August 7<sup>th</sup>, 2019</u> which is also the date of commencement of applicable warranties required by the Contract Documents.

A list of items to be completed or corrected is attached to this Certificate or will be transmitted after completion of the final job walk. The failure to include any items on such a list does not alter the responsibility of the Contractor to complete all work in accordance with the Contract Documents. The date of commencement of warranties of items will be the date of Notice of Completion unless otherwise agreed to in writing.

Recommended By:

Patrick Hanify Construction Manager

Agreed By:

Contractor

Acknowledged By:

DAR WYWE

1 out will superret, P

Date

Enclosures: Construction Punch-list

CC:



### BOARD OF DIRECTORS ENGINEERING AND PLANNING COMMITTEE STAFF REPORT

DATE: October 9, 2019

TO: Engineering and Planning Committee FROM: Clarence Mansell Jr., General Manager

SUBJECT: AUTHORIZATION TO APPROVE CHANGE ORDER NO. 3 FOR THE

CUSTOMER SERVICE FOYER RENOVATION PROJECT

### **BACKGROUND:**

The West Valley Water District ("District") has identified a need to improve the Customer Service Foyer and the Administration Foyer. The project goal is to make these areas inviting and comfortable for our customers and the general public with structural and aesthetic enhancements.

This item was presented to the Engineering and Planning Committee on March 15, May 23, and July 12, 2017. At the May 23rd meeting Ruhnau Clarke Architects ("Architect") was directed by the Engineering and Planning Committee Directors to prepare an update to the Conceptual Design options for both Foyers. These options were presented to the committee on July 12th were Option 2B was selected as the preferred Customer Service Foyer layout and Options 3A and 5A for the Administration Foyer. The Engineering and Planning Committee directed District Staff to have this item considered by the full Board of Directors. This item was presented to the full Board of Directors at the Board Meeting on August 3, 2017. At the August 3rd meeting the Board of Directors selected Options 2B and 3A and authorized the General Manager to negotiate a scope of work and fee with the Architect for a final design services task order based on Board selected options.

On May 29, 2018 the City of Rialto ("City") approved the drawings for the Foyers Project prepared by the Architect.

District Staff was directed to remove the Administration Foyer from the scope of work and bidding documents. On June 25, 2018 the District Staff and Architect held a meeting to discuss separating the phasing schedule and scope of work Not-In-Contract ("NIC"). The Architect was directed to prepare an updated set of plans and specifications identifying which areas are NIC.

On July 2, 2018 a Request for Bids ("RFB") was posted on PlanetBids for the Foyer Renovation Project. On July 30, 2018 one (1) bid was received. This item was presented to the Engineering and Planning Committee on August 8, 2018. At the August 8th, 2018 meeting District Staff was directed to re-bid the Customer Service Foyer Renovation Project ("Foyer Project") and publically advertise in a newspaper.

On September 25, 2018 a Request for Bids ("RFB") was posted on PlanetBids for the Foyer Project to general building contractors and eight (8) construction firms. On September 28, 2018 the Bid Notice Inviting Bids for the Foyer Project was published in the San Bernardino County Sun newspaper. On October 30, 2018 four (4) bid were received.

### **DISCUSSION:**

On January 25, 2019, the District entered into a contract with Caltec Corporation for the construction of the Customer Service Foyer Renovation Capital Improvement Project. While demolishing and preparing the site for improvements, the Contractor encountered the following unforeseen items that needed to be added to the scope of work and adjusted in the field to complete the project:

Adjust exterior concrete flatwork slopes that are incompatible with the installation of the new customer entrance sliding door

Add tube steel support for stability of the pony wall

Provide access door at the existing junction box above the ceiling

Provide wood header for ceiling transition to match existing wood beams between corridor and the customer service back of house

Remove existing quarry tile discovered under the flooring for installation of the new flooring Coordinate the foundation and framing details for the added arch outside of the main entrance

The items listed above the original contract amount was required. Caltec Corporation has submitted Change Order No. 3 to cover the cost for this additional work.

### **FISCAL IMPACT:**

This project was a budgeted item in the Fiscal Year 2019/20 Capital Improvement Budget under the W17040 Customer Service Foyer Renovation. This change order will increase the contract amount by \$23,378.70 for a total of \$637,672.00. A copy of Change Order No. 3 is attached as **Exhibit A**. Additional funds will be needed. The District's budget for Contingency has funds available to transfer. A summary of the requested budget transfer is as follows:

CIP FY 2019-2020 Project Name	Current Budget	Construction Cost	Transfer From/To	Remaining Budget
CONT Contingency	\$344,140.70	\$0.00	(\$23,378.70)	\$320,762.00
W17040 Customer Service Foyer Renovation	\$0.00	\$23,378.70	\$23,378.70	\$0.00

### **STAFF RECOMMENDATION:**

It is recommended that the Engineering, Operations, and Planning Committee approve Change Order No. 3 for the Customer Service Foyer Renovation Construction Capital Improvement Project in the amount of \$23,378.70 and have this item considered by the full Board of Directors at a future meeting and authorize the General Manager to execute the necessary documents.

Respectfully Submitted,

Classes C. Manselly.

Clarence Mansell Jr, General Manager

RMG:ce

### ATTACHMENT(S):

1. Exhibit A - Caltec Corporation Change Order No. 3

### **EXHIBIT A**

### SECTION 2.11 of PROCEDURAL DOCUMENTS

### **CHANGE ORDER**

**OWNER:** West Valley Water District

**CONTRACTOR:** Caltec Corporation

8732 Westminster Blvd. Suite 2

Westminster, CA 92683

**PROJECT:** West Valley Water District Foyer

Renovation

Change Order No. 3 Agreement Date: January 25, 2019

Date: 10/02/2019 Sheet 1 of 3

The following changes are hereby made to the Contract Documents:

### I. EXTRA WORK

	<u>ADD</u>	<u>DEDUCT</u>
1. COR 02R2 – Exterior Concrete Flatwork – RFI 19R2	\$10,617.87	
2. COR 17R1 – Pony Wall Support – AFO 06R1	\$ 1,277.38	
3. COR 18R1 – Access Door – RFI 44	\$ 483.64	
4. COR 19 – Wood Header – RFI 35	\$ 1,594.01	
5. COR 20 – Remove Quarry Tile and Patch – RFI 13R1	\$ 1,405.80	
6. COR 22 – Concrete Footing and Wood Framing	\$ 8,000.00	
Total, for Item I	\$23,378.70	- \$0.00
TOTAL FOR CHANGE ORDER NO. 1	\$23,378.70	

### II. CONTRACT TIME

Increased 0 calendar days

### III. JUSTIFICATION:

- 1. COR 02R2 Exterior flatwork slopes are incompatible with installation of new customer entrance sliding door.
- 2. COR 17R1 Adding tube steel support for stability of the pony wall.
- 3. COR 18R1 Providing access door at existing junction box above ceiling.
- 4. COR 19 Provide wood header for ceiling transition to match existing wood beams between corridor and Customer Service back of house
- 5. COR 20 Quarry tile discovered under flooring need to be removed for installation of the new flooring.
- 6. COR 22 Coordination of foundation and framing details for added arch outside of the main entrance.

CHANGE	TO	CONTRACT PRICE:	

Original Contract Price	\$ <u>567,000.00</u>
Current Contract Price Adjusted by Previous Change Order(s)	\$ <u>47,293.30</u>
Contract Price Due to This Change Order will be Increased by	\$ <u>23,378.70</u>
New Contract Price, including This Change Order	\$ 637,672.00
CHANGE TO CONTRACT TIME:	
Contract Time will be increased	0 Working Days
Date of Completion of All Work	September 17, 2019 (Date)
REQUIRED APPROVALS:	
To be effective, this Change Order must be required by the Supplemental General Condition	approved by the Owner, or as may otherwise be ons.
Requested By (Contractor)	Date
Recommended By (Project Manager)	Date
Recommended By (Asst. Gen. Manager)	Date
Accepted By (Owner)	Date



### BOARD OF DIRECTORS ENGINEERING AND PLANNING COMMITTEE STAFF REPORT

DATE: October 9, 2019

TO: Engineering and Planning Committee FROM: Clarence Mansell Jr., General Manager

SUBJECT: CONSIDER RELEASE OF OVERLYING EASEMENT ON APN 0131-131-02,

08 AND 09

### **BACKGROUND:**

Attached for committee review is a proposed Quitclaim Deed to release an overlying easement on 7.62 acres of land within APN 0131-131-02, 08 and 09, located on the northeast corner of Randall Avenue and Acacia Avenue, in the City of Rialto, to GDC-RCH Santiago LP, a Delaware LLC ("Applicant").

The subject parcels are outside of the District's service area and are not within its sphere of influence. Staff has reviewed the Applicant's request, and did not identify any conflicting facilities within the Applicant's proposed project area, nor is this location identified as part of the District's long term supply operations.

A figure depicting the location of the overlying easement area is attached as **Exhibit A.** Attached for committee review and approval is a copy of the Quitclaim Deed labeled **Exhibit B**.

### **FISCAL IMPACT:**

In accordance with West Valley Water District's Rules and Regulations Article 20, section 2019, the applicant will be charged \$50.00 per acre or any portion thereof with a minimum fee of \$500.00 for the review and processing of documents related to right-of-way and easement release.

### **STAFF RECOMMENDATION:**

It is recommended that the Engineering, Operations and Planning Committee authorize the release of the overlying easement on APN 0131-131-02, 08 and 09, and have this item considered by the full Board of Directors at a future meeting.

Respectfully Submitted,

Clarence C. Mansellfr.

Clarence Mansell Jr, General Manager

DG:ce

### ATTACHMENT(S):

- 1. Exhibit A Aerial Map
- 2. Exhibit B Quitclaim Deed

### **EXHIBIT A**







### **EXHIBIT B**

When recorded mail to:

<u>GDC-RCH Santiago LP, a Delaware LLC</u>

1428 E. Chapman Ave

<u>Orange, CA 92866</u>

### (SPACE ABOVE THE LINE FOR RECORDER'S OFFICE USE ONLY)

Project: A.P.N. <u>0131-131-02</u>, 08 and 09

### QUITCLAIM DEED

FOR VALUABLE CONSIDERATION, receipt of which is hereby acknowledged, **West Valley Water District**, a county water district, formerly known as West San Bernardino County Water District, and successor-in-interest to the Semi Tropic Land and Water Company does hereby remise, release and forever quitclaim to GDC-RCH Santiago LP, a Delaware Limited Liability Company, all right, title and interest in those certain easement rights granted or reserved in the real property recorded on December 24, 1890 described on Book 122, Page 331, of Deeds, Official Records of San Bernardino County, California, ONLY AS TO THAT CERTAIN REAL PROPERTY described in Exhibit "A" and depicted in Exhibit "B" attached hereto and incorporated herein by this reference, located in the City of Rialto, County of San Bernardino, State of California.

Dated	, 2019	a county water district
		By_
		Clarence C. Mansell, Jr. General Manager
		By
		Crystal L. Escalera, Interim Board Secretary

A notary public or other officer completing this certificate verifies only the identity of the individual who signed the document to which this certificate is attached, and not the truthfulness, accuracy, or validity of that document.

STATE OF CALIFORNIA	)	0.0		
COUNTY OF SAN BERNARDINO	)	SS		
On	execute nature(s) execute	d the san ) on the i ed the ins	ne in his/her/their aut nstrument the person trument.	thorized n(s), or the entity
I certify under PENALTY OF PER foregoing paragraph is true and correct.	<b>URY</b>	under the	e laws of the State of	California that the
WITNESS my hand and official se	al.		(Seal)	
STATE OF CALIFORNIA COUNTY OF SAN BERNARDINO	) )	SS		
On	whose na execute nature(s) execute	ame(s) is, d the san ) on the i ed the ins	/are subscribed to the ne in his/her/their aut nstrument the person trument.	e within instrument thorized n(s), or the entity
WITNESS my hand and official se	al.		(Seal)	

### **EXHIBIT A**

Order Number: NHSC-5597370 (tc)

Page Number: 6

### **LEGAL DESCRIPTION**

Real property in the City of , County of San Bernardino, State of California, described as follows:

PARCEL 1: (APN: 131-131-02 AND 131-131-03)

ALL THAT PORTION OF LOT 148, SOUTH OF BASELINE ACCORDING TO MAP SHOWING SUBDIVISION OF LAND BELONGING TO THE SEMI TROPIC LAND AND WATER CO., IN THE CITY OF RIALTO, COUNTY OF SAN BERNARDINO, STATE OF CALIFORNIA, AS PER MAP RECORDED IN BOOK 4, PAGE 11 OF MAPS, IN THE OFFICE OF THE COUNTY RECORDER OF SAID COUNTY, DESCRIBED AS FOLLOWS:

COMMENCING AT THE POINT OF INTERSECTION OF THE CENTER LINE OF ACACIA AVENUE WITH THE NORTHERLY LINE EXTENDED TO SAID LOT 148; THENCE SOUTH ALONG THE CENTER LINE OF ACACIA AVENUE 165 FEET; THENCE AT RIGHT ANGLES EAST 660 FEET; THENCE NORTH PARALLEL TO THE CENTER LINE OF ACACIA AVENUE 165 FEET TO THE NORTH LINE OF SAID LOT 148; THENCE WEST 660 FEET TO THE POINT OF BEGINNING. (SAID LAND IS ALSO SHOWN ON THE MAP OF THE TOWN OF RIALTO AND ADJOINING SUBDIVISION RECORDED IN BOOK 4, PAGE 11 OF MAPS, RECORDS OF SAID COUNTY.

PARCEL 2: (APN: 131-131-08)

PORTION OF THE WEST ONE-HALF OF LOT 148, TOWN OF RIALTO AND ADJOINING SUBDIVISION, AS PER PLAT RECORDED IN <u>BOOK 4 OF MAPS</u>, <u>PAGE 11</u>, RECORDS OF SAID COUNTY, DESCRIBED AS FOLLOWS:

COMMENCING AT THE SOUTHWEST CORNER OF SAID LOT 148, SAID SOUTHWEST CORNER BEING THE CENTER LINE OF ACACIA AVENUE AS SHOWN ON SAID MAP; THENCE EASTERLY ALONG THE SOUTHERLY LINE OF SAID LOT, 660 FEET TO THE CENTER LINE OF SAID LOT 148; THENCE NORTHERLY ALONG THE CENTER LINE OF SAID LOT 148, 247.5 FEET; THENCE WESTERLY AND PARALLEL WITH THE SOUTHERLY LINE OF SAID LOT, 660 FEET TO THE CENTER LINE OF ACACIA AVENUE; THENCE SOUTHERLY 247.5 FEET ALONG THE CENTER LINE OF SAID ACACIA AVENUE TO THE POINT OF BEGINNING.

EXCEPT THEREFROM THAT PORTION AS CONVEYED TO THE CITY OF RIALTO FOR STREET AND HIGHWAY PURPOSES BY DEED RECORDED APRIL 10, 1970, IN <u>BOOK 7422, PAGE 231</u>, OFFICIAL RECORDS.

PARCEL 3: (APN: 131-131-09)

THE WEST 1/2 LOT 148, TOWN OF RIALTO AND ADJOINING SUBDIVISION, IN THE CITY OF RIALTO, COUNTY OF SAN BERNARDINO, STATE OF CALIFORNIA, AS PER PLAT RECORDED IN BOOK 4, PAGE 11 OF MAPS, RECORDS OF SAID COUNTY.

EXCEPTING THEREFROM THAT PORTION DESCRIBED AS FOLLOWS:

COMMENCING AT THE POINT OF INTERSECTION OF THE CENTER LINE OF ACACIA AVENUE, WITH THE NORTHERLY LINE EXTENDED OF SAID LOT 148; THENCE SOUTH ALONG THE CENTER LINE OF ACACIA AVENUE, 165 FEET; THENCE AT RIGHT ANGLES EAST 660 FEET; THENCE NORTH PARALLEL TO THE CENTER LINE OF ACACIA AVENUE 165 FEET TO THE NORTH LINE OF SAID LOT 148; THENCE WEST 660 FEET TO THE POINT OF BEGINNING.

Order Number: NHSC-5597370 (tc)

Page Number: 7

### ALSO EXCEPTING THEREFROM THAT PORTION DESCRIBED AS FOLLOWS:

COMMENCING AT THE SOUTHWEST CORNER OF SAID LOT 148, SAID SOUTHWEST CORNER BEING IN THE CENTER LINE OF ACACIA AVENUE AS SHOWN ON SAID MAP; THENCE EASTERLY ALONG THE SOUTHERLY LINE OF SAID LOT, 660 FEET TO THE CENTER LINE OF SAID LOT 148; THENCE NORTHERLY ALONG THE CENTER LINE OF SAID LOT 148, 247.5 FEET; THENCE WESTERLY AND PARALLEL WITH THE SOUTHERLY LINE OF SAID LOT, 660 FEET TO THE CENTER LINE OF ACACIA AVENUE; THENCE SOUTHERLY 247.5 FEET ALONG THE CENTER LINE OF SAID ACACIA AVENUE TO THE POINT OF BEGINNING.

### **EXHIBIT B**

## SURVEYOR'S NOTES

THE BASIS OF BEARINGS FOR THIS MAP IS THE CENTER LINE OF ACACIA AVENUE TAKEN AS NORTH 00°00'42" EAST PER MB 167/5-8 OF MAPS, RECORDS OF SAN BERNARDINO COUNTY, CALIFORNIA.

100

- Ņ (R1) ~ INDICATES RECORD DATA PER MB 167/5-8.
- (R2) ~ INDICATES RECORD DATA PER RS 24/14.
- ယ 4. (R3) ~ INDICATES RECORD DATA PER MB 339/7-10.
- 57 (R4) ~ INDICATES RECORD DATA PER MB 240/67-69.
- 6 (R5) ~ INDICATES RECORD DATA PER MB 230/81-82.
- 7. (R6) ~ INDICATES RECORD DATA PER LLA.
- œ INDICATES FOUND MONUMENT AS NOTED.
- 0 INDICATES 1" I.D. IRON PIPE WITH PLASTIC PLUG STAMPED "LS 7843", SET FLUSH, UNLESS NOTED OTHERWISE.
- 10.  $\triangleright$ INDICATES 4 3/4" GEAR SPIKE & WASHER STAMPED "LS 7843", SET FLUSH IN FINISHED SURFACE OF PAVEMENT.

(N)

.3(R6)

1(R5)

<u>;</u> SET 1/2" x 18" IRON RE-BAR WITH PLASTIC PLUG STAMPED "LS 7843", FLUSH AT ALL REAR LOT CORNERS. IN THE EVENT THE ABOVE TYPE OF MONUMENT CANNOT BE SET DUE TO UNFORESEEN CIRCUMSTANCES, THEN A TAG STAMPED "LS 7843" WILL BE AFFIXED IN CONCRETE, STONE, WOOD OR METAL AT THE TRUE POSITION CALLED FOR

TR 10625 MB 167 / 5-8

- 12. SET NAIL AND TAG "LS 7843" ON TOP OF CURB ON PROJECTION OF SIDE LOT LINES IN LIEU OF FRONT LOT CORNERS.
- 13. THIS MAP HAS 50 LOTS AND 18 LETTERED LOTS.
- 15. 14. THIS MAP CONTAINS 7.62 ACRES, MORE OR LESS, WITHIN THE DISTINCTIVE BORDER.
- S.F.N. ~ INDICATES SEARCHED FOUND NOTHING.
- <u>1</u>6.  $2/2/2 \sim 1$  INDICATES VEHICULAR ACCESS RESTRICTION DEDICATED TO THE CITY OF RIALTO HEREON.

### (g) MONUMENT NOTES

- FOUND 1" I.P. WITH PIN IN CONCRETE, DOWN 8", ACCEPTED AS FOUND I.P. PER R1, R2, R4 & R5 FOR THE CENTER LINE INTERSECTION OF RANDALL AVENUE AND ACACIA AVENUE. SET TAG, "LS 7843". FOUND 1" I.P. TAGGED "RCE 9101", DOWN 3" PER R1 & R4 FOR THE N.W. CORNER OF LOT 137 PER

- SEARCHED, FOUND NOTHING. ESTABLISHED BY PROPORTION PER R1 FOR THE N.W. CORNER OF LOT 148 PER MB 4/11.

  FOUND C.NAIL AND TIN, FLUSH IN PAVEMENT, PER R3 FOR THE CENTER LINE INTERSECTION OF RANDALL AVENUE AND EUCALYPTUS AVENUE.

  SEARCHED, FOUND NOTHING. ESTABLISHED BY EQUAL PROPORTION FOR THE S.E. CORNER OF THE WEST ONE-HALF OF LOT 148 PER MB 4/11.

  FOUND GEAR SPIKE AND WASHER STAMPED "LS 5411", FLUSH PER CORNER RECORD 16 0250, FILED WITH THE SAN BERNARDINO COUNTY SURVEYOR JULY 26, 2016 AS DOC. No. 325 077, ACCEPTED AS THE N.E. CORNER OF LOT 148 PER R2.

  FOUND 1" I.P. WITH PIN IN CONCRETE, FLUSH, ACCEPTED AS FOUND I.P. PER R5 FOR THE CENTER LINE INTERSECTION OF RANDALL AVENUE AND PINE AVENUE. SET TAG, "LS 7843".

# EASEMENT NOTES AN EASEMENT SHOWN OR DEDICATED FOR ROAD AND INCIDENTAL PURPOSES AS SHOW ON THE MAP OF THE TOWN OF RIALTO AND ADJOINING SUBDIVISIONS, AS SHOWN BY MAP ON FILE IN BOOK 4, PAGE 11 OF MAPS, RECORDS OF SAID SAN BERNARDINO COUNTY, CALIFORNIA AND IS SHOWN HEREON.

22. THAT PORTION AS CONVEYED TO THE CITY OF RIALTO FOR STREET AND HIGHWAY PURPOSES BY DEED RECORDED APRIL 10, 1970, IN BOOK 7422, PAGE 231 OF OFFICIAL RECORDS AND IS SHOWN HEREON.

- ώ AN EASEMENT IN FAVOR OF SEMI-TROPIC LAND AND WATER COMPANY FOR PIPELINES, INGRESS AND EGRESS AND INCIDENTAL PURPOSES, RECORDED DECEMBER 24, 1890 IN BOOK 122, PAGE 331 OF DEEDS, THE LOCATION OF SAID EASEMENT CANNOT BE DETERMINED FROM RECORD INFORMATION.
- 4. VARIOUS AGREEMENTS, CONDITIONS AND RESERVATIONS AS TO WATER AND RIGHTS OF WAY AND EASEMENTS OF UNDISCLOSED ROUTE AND WIDTH, AS SET FORTH IN THE DEED FROM JOHN H. CARRUTHERS TO FONTANA DEVELOPMENT COMPANY, A CORPORATION, RECORDED MARCH 30, 1901 IN BOOK 290, PAGE 83 OF DEEDS. OTHER DOCUMENTS MODIFYING SAID AGREEMENTS ARE RECORDED IN BOOK 290, PAGE 100 OF DEEDS AND IN BOOK 290, PAGE 115 OF DEEDS. THE LOCATION OF SAID EASEMENT CANNOT BE TERMINED FROM RECORD INFORMATION
- AN EASEMENT FOR STORM DRAIN PURPOSES DEDICATED HEREON.

IN THE CITY OF RIALTO, COUNTY 읶 SAN BERNARDINO, STATE OF CALIFORNIA

SHEET 2 OF 4 SHEETS

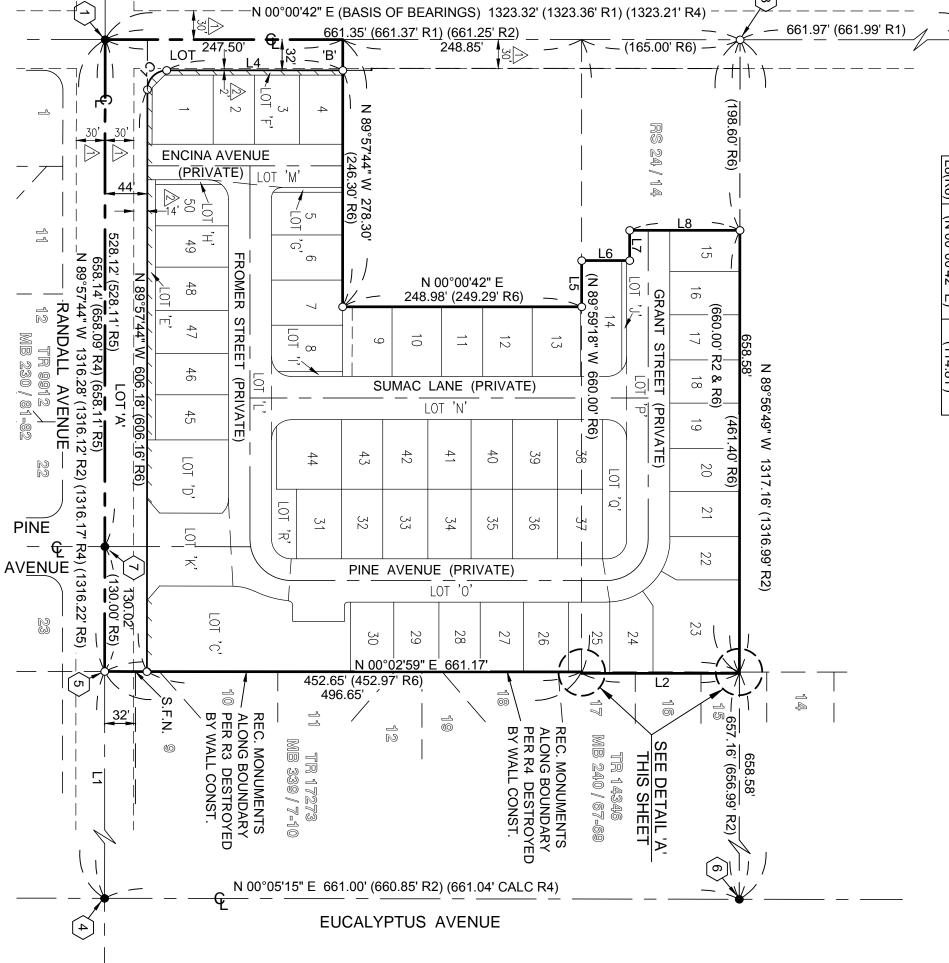
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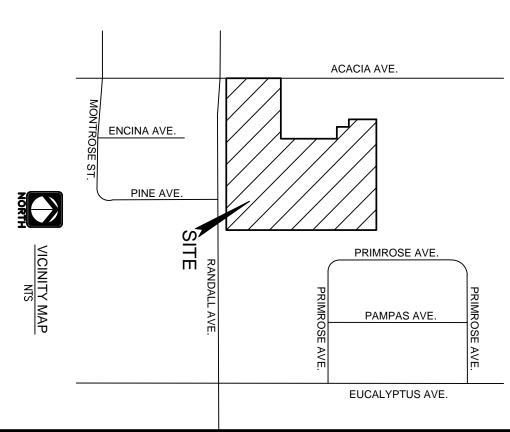
BEING A SUBDIVISION O RIALTO AND ADJOINING OF SAN BERNARDINO C OF A PORTION OF THE SOUTH ONE-HALF OF LOT 148 OF THE MAP OF THE TOWN OF G SUBDIVISIONS AS SHOWN BY MAP ON FILE IN BOOK 4, PAGE 11 OF MAPS, RECORDS COUNTY, CALIFORNIA.

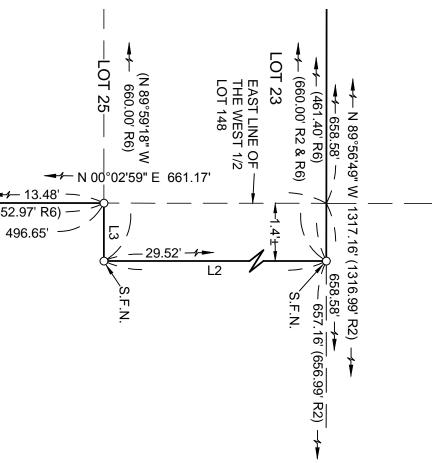
BLAINE A. WOMER CIVIL ENGINEERING, INC. (SEC. 13, T.1S., R.5W., S.B.M.)

LINE DATA				<b>CURVE DATA</b>	Α	
BEARING	DISTANCE	CURVE	RADIUS	LENGTH	DELTA	TANGENT
N 89°57'44" W	658.14'	C1	20.00'	31.41'	89°58'26"	19.99'
	(658.08')	C1(R6)	(20.00')	(31.41')	(89°58'24")	(19.99')
	(658.08')					
	(658.11')					
N 00°00'42" E	164.52'					
	(164.87')					
	(164.37')					
N 89°59'18" W	1.53'					
	(1.54')					
(N 00°00'42" E)	(183.51')					
N 89°59'18" W)	(48.30')					
(N 00°00'42" E)	(50.00')					
N 89°59'18" W)	(31.40')					
N 00°00'42" E	114.86'					
(N 00°00'42" E)	(114.81')					



ACACIA AVENUE





DETAIL 'A'



### BOARD OF DIRECTORS ENGINEERING AND PLANNING COMMITTEE STAFF REPORT

DATE: October 9, 2019

TO: Engineering and Planning Committee FROM: Clarence Mansell Jr., General Manager

SUBJECT: CONSIDER WATER SYSTEM INFRASTRUCTURE INSTALLATION

AND CONVEYANCE AGREEMENT WITH CALATLANTIC GROUP, INC

CASA GRANDE AVENUE AND SIERRA AVENUE WATERLINE

CROSSING

### **BACKGROUND:**

Calatlantic Group, Inc. ("Developer") is the owner of land located at the southwest corner of Casa Grande Avenue and Sierra Avenue in the City of Fontana, known as Tract No. 20213, Summit at Rosena ("Development"), as shown in **Exhibit A**. In developing this land, a 24" storm drain is required to be installed from the east side of the intersection of Sierra Avenue and Casa Grande Avenue to the new Development on the west side. During the utility investigation process, it was discovered that West Valley Water District has 12" and 6" CMLC waterlines in conflict with the proposed storm drain. It is the intention of the Developer to relocate the interfering portions of waterline by constructing a siphon under the storm drain to avoid the conflict. In addition to the siphon, the Developer will also be required to replace the interfering portions of 6" waterline with 12" waterline.

### **DISCUSSION:**

In order to proceed with the waterline relocation for the Development, West Valley Water District ("District") and the Developer must enter into a Developer-Installed Water System Infrastructure Installation and Conveyance Agreement ("Agreement"). This Agreement outlines the responsibilities of the Developer in constructing facilities, including insurance, indemnification and bonding requirements as well as conveyance and acceptance of the water system by the District. Attached as **Exhibit B** is a copy of the Water System Infrastructure Installation and Conveyance Agreement for this development.

### **FISCAL IMPACT:**

No fiscal impact to the District.

### **STAFF RECOMMENDATION:**

It is recommended that the Engineering, Operations and Planning Committee approve the Water System Infrastructure Installation and Conveyance Agreement with Calatlantic, Inc. and have this item considered by the full Board of Directors at a future meeting.

Respectfully Submitted,

Clarence C. Manselly

Clarence Mansell Jr, General Manager

DG:ce

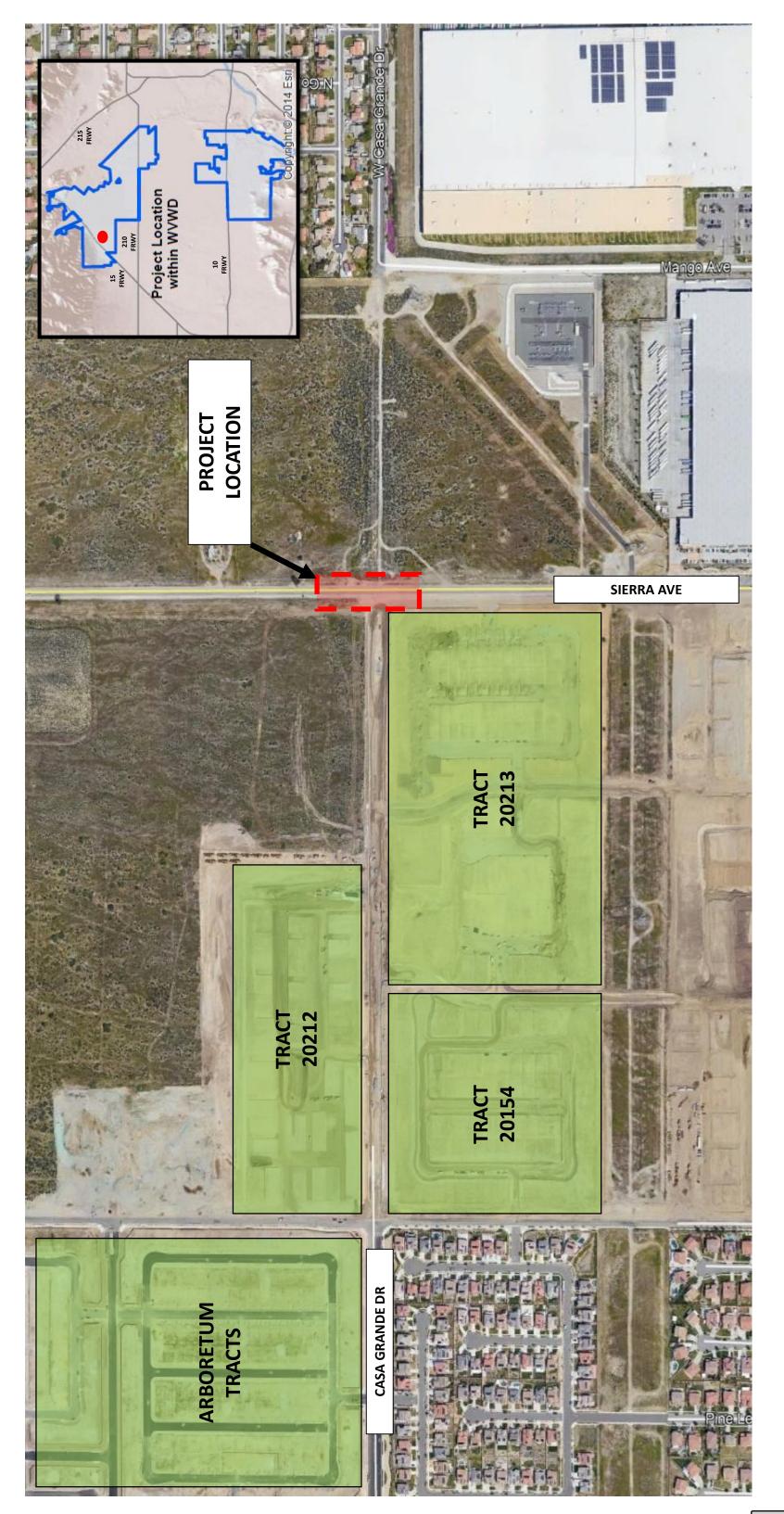
### **ATTACHMENT(S)**:

- 1. Exhibit A Aerial Map
- 2. Exhibit B Water System Infrastructure Installation and Conveyance Agreement with Calatlantic Group, Inc

### **EXHIBIT A**

3.g.a





### **EXHIBIT B**

### WATER SYSTEM INFRASTRUCTURE INSTALLATION AND CONVEYANCE AGREEMENT

This water system infrastructure installation and conveyance agreement ("Agreement") is entered into and effective as of \_\_\_\_\_\_\_ by and between CALATLANTIC GROUP, INC ("Developer"), and WEST VALLEY WATER DISTRICT ("District") who agree as follows:

The Developer is the owner of certain land described as **TRACT 20213** and as more fully (or further) shown on <u>Exhibit "A"</u>. In developing this land, the Developer is desirous of obtaining a public water supply adequate for domestic uses and public fire protection purposes and is desirous of integrating that water system into the District's public water system.

In order to provide facilities for a water supply to said land, it is the intention of the parties to this Agreement that the Developer shall furnish and install those water mains, fire hydrants, service laterals, water meters and valves, valve boxes, and all other appurtenant fittings and facilities required for a complete water system to serve the land shown on <u>Exhibit "A"</u>.

In order to implement the foregoing and in consideration of the terms and conditions herein contained, the parties further agree as follows:

### 1. DESIGN

- 1.1. Developer shall design and construct, at the Developer's own expense, the water facilities and appurtenances required to serve the development in accordance with final District approved plans known as **CASA GRANDE AVENUE AND SIERRA AVENUE WATERLINE CROSSING**, as approved and attached herein as <u>Exhibit "B"</u> and in accordance with District-approved design standards and specifications, and the terms and conditions of this Agreement.
- 1.2. The water system design shall be by a Professional Engineer registered in the State of California, and in accordance with the District's most recent Rules and Regulations (the "Rules and Regulations"), the District's Standards for Domestic Water Facilities and Standard Drawings herein included by reference, all applicable District ordinances and policies and all City, County of San Bernardino, State of California, and Federal laws, ordinances, rules, regulations, codes and other legal requirements of all government bodies having jurisdiction over said construction and property (all of the foregoing requirements in this paragraph being collectively referred to herein at times as "Legal Requirements").
- 1.3. The District, at Developer's expense, shall review Developer's plans for the purpose of ensuring the adequacy of the design and conformance with the District's standards and specifications. The District reserves the right to add, delete, modify, change or amend any or all the plans and specifications.
- 1.4. In the event that the property to be developed includes multiple residential, condominiums, commercial or industrial uses, all site plans, grading plans, and any available plumbing plot plans shall be furnished to the District by Developer.

1.5. The District makes no warranties as to the correctness, accuracy or completeness of the plans and specifications. The accuracy, adequacy, suitability, and correctness of the water system design shall be the sole responsibility of the Developer.

### 2. CONSTRUCTION

- 2.1. Developer shall perform, or caused to be performed, all construction of the water system infrastructure installation pursuant to the approved water system plans, and all Legal Requirements.
- 2.2 The performance of this Agreement shall commence within ninety (90) calendar days from the date of this Agreement and shall be completed within one (1) year from the estimated construction start date.
- 2.3. Time is of the essence in this Agreement; provided that, in the event good cause is shown therefore, the general manager of the District ("General Manager") may extend the time for completion of the water system installation. Any such extension may be granted without the notice to Developer's surety, and extensions so granted shall not relieve the surety's liability on the bond to secure faithful performance of this Agreement. The General Manager shall be the sole and final judge as to whether or not good cause has been shown to entitle Developer to an extension.
- 2.4. The Developer and its contractor and subcontractors shall attend a preconstruction meeting with the District at the District's headquarters no less than five (5) working days prior to commencement of construction.
- 2.5. No work on water facilities shall commence prior to the completion of all required curbs and gutters.

### 3. LICENSES AND PERMITS

- 3.1. Developer, and all of Developer's contractors and subcontractors warrants it possesses, or shall obtain, and maintain during the term of this Agreement any and all licenses, permits, qualifications, insurance and approval of whatever nature that are legally required of Developer, its contractors, and all subcontractors to practice its profession, skill or business.
- 3.2. The work to be performed under this Agreement, except meter installations, shall be performed by Developer, or a contractor or subcontractor who is pre-approved by the District and is licensed under the laws of the State of California in the specialty Class of "C-34" Pipeline or Class "A" General Engineering. A copy of the contract between Developer and the selected pre-approved contractor and all subcontractors shall be submitted to the District for review and approval.
- 3.3. Excavation/resurfacing permits shall be secured by Developer at Developer's expense. Permits/easements to install, maintain and operate water system facilities in private property shall be secured by Developer at Developer's own expense prior to construction.

3.4. Developer shall, at Developer's own expense, be responsible for obtaining and adhering to a National Pollution Discharge Elimination System (NPDES) permit from the Regional Water Quality Board as required for construction or pipeline flushing and disinfection.

### 4. INSURANCE REQUIREMENTS

- 4.1. The following insurance requirements have been adopted by the District and shall be applicable to this Agreement. These requirements supersede the insurance requirements set forth in any other reference of the District, and to the extent of any conflict, the specified requirements herein shall prevail.
- 4.2. Developer shall ensure that Developer's contractors conform to the following insurance requirements and that all required documents are submitted to the District at the time of Agreement submittal: Developer shall ensure that its contractors and all subcontractors shall purchase and maintain insurance in amounts equal to the requirements set forth in (a) through (d) below, and shall not commence work under this Agreement until all insurance required under this heading is obtained in a form acceptable to the District, nor shall Developer allow any contractor or subcontractor to commence construction pursuant to a contract or subcontract until all insurance required of the contractor and any subcontractors has been obtained.
- a. <u>General Liability</u>: Developer shall ensure that its contractor and all subcontractors shall maintain during the life of this Agreement, a standard form of either Comprehensive General Liability insurance or Commercial General Liability insurance ("General Liability Insurance") providing the following minimum limits of liability: Combined single limit of \$1.0 million per occurrence for bodily injury, including death, personal injury, and property damage with \$2.0 million minimum aggregate, separate for this project as evidenced by endorsement. The insurance shall include coverage for each of the following hazards: Premises-Operations; Owners and Contractors Protective; Broad Form Property Damage contractual for Specific Contract; Severability of Interest or Cross-Liability; XCU Hazards; and Personal Injury With the "Employee" Exclusive Deleted.
- b. <u>Automotive/Vehicle Liability Insurance</u>: Developer shall ensure that its contractor and all subcontractors shall maintain a policy of automotive/vehicle liability insurance on a commercial auto liability form covering owned, non-owned and hired automobiles providing the following minimum limits of liability: Combined single limit of liability of \$1.0 million per accident for Bodily Injury, Death and Property Damage ("Automotive/Vehicle Liability Insurance").
- c. <u>Workers' Compensation Insurance</u>: Developer shall ensure that its contractor and all subcontractors shall provide such workers' compensation insurance with statutory minimum amounts of coverage, as required by the California *Labor Code* and other applicable law, and including employer's liability insurance with a minimum limit of \$1,000,000.00 ("Workers' Compensation Insurance"). Such Workers' Compensation Insurance shall be endorsed to provide for a waiver of subrogation against the District.
- d. <u>Excess Liability</u>: Developer shall ensure that its contractor and all subcontractors shall provide a policy providing excess coverage in a face amount necessary when

combined with the primary insurance, to equal the minimum requirements for General Liability Insurance and Automotive/Vehicle Liability Insurance.

- 4.3. The insurances provided for in Section 4.2 and its subsections above are subject to all of the following conditions:
- a. The insurance shall be issued and underwritten by insurance companies acceptable to the District, and shall be licensed by the State of California to do business on the lines of insurance specified. The insurers must also have an "A-" Policyholder's rating" and a "financial rating" of at least Class VII in accordance with the most current A.M. Best's Rating Guide.
- b. Developer's contractor and subcontractors may satisfy the limit requirements in a single policy or multiple policies. Any such additional policies written as excess insurance shall not provide any less coverage than that provided by the first or primary policy.
- c. Any costs associated with a self-insured program, deductibles, or premium rating programs that determine premium based on loss experience shall be for the account of Developer, Developer's contractor and subcontractors, and the District shall not be required to participate in any such loss. If any such programs exist, Developer, Developer's contractor and subcontractors, agree to protect and defend the District in the same manner as if such cost provisions were not applicable.
- d. Developer shall ensure that its contractor and all subcontractors shall have presented at the time of execution of the Agreement, the original policies of insurance and a certificate of insurance naming the District as the certificate holder and that such coverage is in force and complies with the terms and conditions outlined herein.
- e. If an insurance policy contains a general policy aggregate of less than the minimum limits specified, then the policy coverage shall be written with limits applicable solely to this Agreement, as specified, and shall not be reduced by or impaired by any other claims arising against Developer. These policy limits shall be set forth by separate endorsement to the policy.
- 4.4. Each such policy of General Liability Insurance and Automotive/Vehicle Liability Insurance shall contain endorsements providing the following:
- a. The District, their board members, officers, agents, employees, consultants, and engineers, are hereby declared to be additional insureds under the terms of this policy, but only with respect to the operations of the Developer at or upon any of the premises of the District in connection with the Agreement with the District, or acts or omissions of the additional insureds in connection with, but limited to its general supervision or inspection of said operations and save for any claims arising from the sole negligence or sole willful misconduct the District.
- b. No policy shall be canceled, limited, materially altered, or non-renewed by the insurer until thirty (30) days after receipt by the District of a written notice of such cancellation or reduction in coverage.
- c. This insurance policy is primary insurance and no insurance held or owned by the designated additional insureds shall be called upon to cover a loss under this policy.

### 5. BONDING REQUIREMENTS

- 5.1. Developer shall generate an engineer's cost estimate based on the water system plans provided to the District. The estimated costs, attached herein as <u>Exhibit "C"</u>, will be submitted to the District for review and approval, and shall be used as the basis for bonding requirements for the water system described in the plans provided to the District by the Developer and approved for construction by the District. The sole intent and purpose of the engineer's estimate is to establish a cost valuation for bonding purposes only.
- 5.2. Performance Bond: The Developer's engineers estimate for the CASA GRANDE AVENUE AND SIERRA AVENUE WATERLINE CROSSING, is TWENTY-FIVE THOUSAND DOLLARS AND 00/100 (\$25,000.00). Developer shall and by this Agreement does guarantee the Developer's faithful performance of this Agreement and all of its terms and conditions by providing the following: Developer shall provide the District with either an irrevocable letter of credit from a recognized financial institution acceptable to the District or a performance bond, from a surety institution licensed by the State of California and authorized to do and doing business in said State, valid and renewable until such improvements are accepted by the District. The irrevocable letter of credit or performance bond shall be in the amount of TWENTY-FIVE THOUSAND DOLLARS AND 00/100 (\$25,000.00). equal to 100 percent of the approved Developer's estimate.
- 5.3. <u>Warranty Bond</u>: The Developer's pre-approved contractor shall furnish a two-year warranty bond for all work completed in accordance with the approved plans (<u>Exhibit "B"</u>). Before District's acceptance of the completed water facilities and appurtenances, such facilities and appurtenances shall be free from any and all liens and encumbrances and free from any and all defects in the materials or construction thereof. The two-year warranty shall be either an irrevocable letter of credit from a recognized financial institution acceptable to the District or a warranty bond beginning on the date of acceptance of the water facilities by the District.

### 6. MATERIALS

6.1. The water system facilities to be installed pursuant to this Agreement shall become an extension of the distribution system of the District. All materials used must conform to District specifications for such materials pursuant to all applicable Legal Requirements.

### 7. NOTICES

- 7.1. All notices herein required shall be in writing, and delivered in person or sent by registered mail, postage prepaid.
  - 7.2. Notices required shall be given to the **District** addressed as follows:

WEST VALLEY WATER DISTRICT

Attn: General Manager Post Office Box 920 Rialto, CA 92377

RE: CASA GRANDE AVE AND SIERRA AVE WATERLINE CROSSING

7.3. Notices required shall be given to **Developer** addressed as follows:

CALATLANTIC GROUP, INC ATTENTION: GEOFF SMITH

980 MENTECITO DRIVE, SUITE 206

CORONA, CA 92879

RE: CASA GRANDE AVE AND SIERRA AVE WATERLINE CROSSING

7.4. Notices required shall be given to **Surety** addressed as follows:

**SURETY NAME:** 

ATTN TO:

**ADDRESS** 

RE: CASA GRANDE AVE AND SIERRA AVE WATERLINE CROSSING

- 7.5. Provided that any party or Surety may change such address by notice in writing to the other party, and thereafter, notices shall be addressed and transmitted to the new address.
- 7.6. The Developer or its contractor shall provide the District forty-eight (48) hours advance notice of request for inspection or testing.
  - 7.7. The District is closed on the holidays listed in Exhibit "D".

### 8. NOTICE TO PROCEED TO CONSTRUCT WATER SYSTEM FACILITIES

8.1. Upon acceptance of the insurance and aforementioned bonds and/or irrevocable letters of credit in the amounts provided herein and approval by the District and upon payment of all applicable charges, the Agreement shall be signed by Developer and the District. The District shall return an original copy of the signed Agreement with a letter to Developer giving notice to proceed to construct the water system facilities.

### 9. INSPECTION

- 9.1. It is understood that the sole purpose and intent of the District's inspection and testing is to validate that the materials, workmanship, and construction of the water facilities are in compliance with the District-approved final plans, the District's Rules and Regulations, the Standards for Domestic Water Facilities, the Standard Drawings, and all other applicable District requirements. Developer acknowledges and represents that it assumes full and sole responsibility for the safety and management of the project.
- 9.2. Developer shall at all times maintain proper facilities and provide safe access for inspection by the District to all parts of the work and to the shops wherein the work is in preparation. Additionally, in connection with the performance of this Agreement, the District shall have the authority to enter the work site at any time for the purpose of identifying the existence of conditions, either actual or threatened, that may present a danger of hazard to any and all employees. Developer agrees that the District, in its sole authority and discretion, may order the immediate abatement of any and all conditions that may present an actual or threatened danger or hazard to any and all employees at the work site. Furthermore, Developer acknowledges the

provisions of California *Labor Code* Section 6400 et seq., which requires that employers shall furnish employment and a place of employment that is safe and healthful for all employees working therein. In the event the District identifies the existence of any condition that presents an actual or threatened danger or hazard to any or all employees at the work site, the District is hereby authorized to order an immediate abatement of that condition.

- 9.3. All work and materials shall be subject to inspection, testing, and acceptance by the District at Developer's expense. In the event Developer arranges to have materials fabricated for the project, Developer may be required to arrange for the District to inspect that material during fabrication at Developer's expense.
- 9.4. All material fabrications shall be preapproved by the District and must conform to District standards and specifications.
- 9.5. The District's inspectors shall have full, unlimited access to perform continuous inspection and have the authority to stop work at any time, by written notice, without any liability whatsoever to the District, if, in the inspectors' judgment, the work called for by this Agreement, or the District approved plans, or the specifications is not being installed or performed in a satisfactory and workmanlike manner according to District's standards and specifications and/or in the event the materials do not comply with the District's standards and specifications.
- 9.6. Final acceptance of all material to be purchased or fabricated by Developer under this Agreement shall be made only with the prior approval of the District. Approval by the District, however, shall not operate to relieve the material supplier or Developer of any guarantees, warranties, or the duty of compliance with any of the requirements of the approved plans and specifications or of this Agreement. All construction pursuant to this Agreement shall be inspected pursuant for conformity with District requirements. Developer shall pay actual costs for inspections.

### 10. TESTING AND DISINFECTION

10.1. All water system facilities and components constructed pursuant to this Agreement shall adhere to all requirements for testing, disinfection, and flushing pursuant to District standards and Legal Requirements.

### 11. RELOCATIONS, RECONSTRUCTIONS, AND DAMAGES

11.1. Developer accepts the responsibility for and the costs occasioned by any reconstruction, relocation, damages to, or changes of water services or facilities caused or contributed to directly or indirectly by any subsequent changes in the location of any of said facilities or water meters or water services.

### 12. AS-CONSTRUCTED DOCUMENTATION

12.1. In order for the District to accept the facilities, Developer shall provide all required documentation as specified in the Standards for Domestic Water Facilities, including As-Built drawings.

### 13. INDEMNIFICATION

- 13.1. Developer hereby agrees to and shall protect, defend, indemnify and hold the District and its board members, officers, agents, employees, and engineers free and harmless from any and all liability losses, damages, claims, liens, demands and cause of action of every kind and character including, but not limited to, the amounts of judgments, penalties, interests, court costs, attorney's/legal fees, and all other expenses incurred by the District arising in favor of any party, including claims, liens, debts, demands for lost wages or compensation, personal injuries, including employees or the District, death or damages to property (including property of the District) and without limitation by enumeration, all other claims or demands of every character occurring or in any way incident to, in connection with or arising directly or indirectly out of the obligations herein undertaken or out of the operations conducted by Developer save and except claims or litigation arising through the sole negligence or sole willful misconduct of the District or the District's agents and employees. Developer shall investigate, handle, respond to, provide defense for and defend any such claims, demand, or suit at the sole expense of Developer even if the claim or claims alleged are groundless, false or fraudulent. Developer agrees to, and shall defend the District and its members, directors, officers, agents, employees, and engineers from any suits or actions at law or in equity for damages caused, or alleged to have been caused, by reason of any of the aforesaid operations, provided as follows:
- a. That the District does not and shall not waive any rights against Developer which it may have by reason of the aforesaid hold harmless agreement, because of the acceptance by the District, or the deposit with District by Developer, or any of the insurance policies described in this Agreement.
- b. That the aforesaid hold harmless agreement by Developer shall apply to all damages and claims for damages of every kind suffered, or alleged to have been suffered, by reason of any or the aforesaid operations referred to in this subsection, regardless of whether or not District has prepared, supplied water system installation, or regardless of whether or not such insurance policies shall have been determined to be applicable to any such damages or claims for damages.

This provision is not intended to create any cause of action in favor of any third party against Developer or the District or to enlarge in any way Developer's liability but is intended solely to provide for indemnification of the District from liability for damage or injuries to third persons or property arising from Developer's performance hereunder.

13.2. Neither Developer nor any of Developer's agents, contractors or subcontractors are, or shall be, considered to be agents of the District in connection with the performance of Developer's obligations under this Agreement.

### 14. REPAIR OR RECONSTRUCTION OF DEFECTIVE WORK

14.1. If, within a period of two years after final acceptance of the work performed under this Agreement, any structure or part of any structure furnished and/or installed or constructed, or caused to be installed or constructed by Developer, or any of the work done under this Agreement, fails to fulfill any of the requirement of this Agreement or the specifications referred to herein, Developer shall, without delay and without any cost to District, repair or replace or reconstruct

any defective or otherwise unsatisfactory part or parts of the work structure. Should Developer fail to act promptly or in accordance with this requirement, or should the exigencies of the situation as determined by the District in the exercise of its sole discretion require repair, replacement or reconstruction before Developer can be notified, District may, at its option, make the necessary repairs or replacements or perform the necessary work, and Developer shall pay to the District the actual cost of such repairs.

### 15. COSTS AND FEES

- 15.1. Developer shall be responsible for all fees and deposits as required by the District. All fees and deposits shall be paid in full prior to the execution of this Agreement and before construction can take place.
- 15.2. Any additional costs and fees shall be paid in full prior to conveyance and acceptance of the water system.

### 16. CONVEYANCE AND ACCEPTANCE OF WATER SYSTEM

- 16.1. Upon completion of the water system in accordance with the approved water plans and submission of the required documentation, the Developer shall convey the water system to the District.
- 16.2. The Developer shall be responsible for insuring the pre-approved contractor furnish an irrevocable letter of credit to the District or a warranty bond (One Hundred (100%) of Developer's estimate) for a period of two years as stated in Sections 5.3 of this Agreement, asbuilt drawings with contractor redlines and AutoCAD files, materials list with quantities, labor, equipment, and materials, water system cost breakouts, compaction test report signed and sealed by a California Registered Engineer, notice of completion filed with San Bernardino County Recorder, fire flow tests of all hydrants, all required easements for water facilities and unconditional financial release from subcontractors and material providers, Upon compliance with all the terms and conditions of this Agreement, the District shall prepare the conveyance agreement accepting the water facilities and forward same to the address provided herein. Title to the ownership of said facilities and appurtenances shall thereby be conveyed to the District. The District shall thereafter operate and maintain said facilities so as to furnish water service to the development (Exhibit "A") in accordance with the District's ordinances, policies and Rules and Regulations.

### 17. PERMANENT WATER SERVICE

17.1. In no event shall permanent water services be provided to Developer's installed system until all applicable charges and fees have been paid by Developer and all facilities have been conveyed, free of all encumbrances, to the District, including any easements which may be required. Such conveyance shall occur in a timely manner in accordance with the terms of this Agreement.

### 18. BREACH OR DEFAULT OF AGREEMENT

- 18.1. If Developer refuses or fails to obtain prosecution of the work, or any severable part thereof, with such diligence as will insure its completion within the time specified, or any extension thereof, or fails to obtain completion of said work within such time, or if Developer should be adjudged as bankrupt, or Developer should make a general assignment for the benefit of Developer's creditors, or if a receiver should be appointed in the event of Developer's insolvency, or if Developer, or any of Developer's contractors, subcontractors, agents or employees, should violate any of the provisions of this Agreement, the District's General Manager or the General Manager's designee may serve written notice upon Developer and Developer's surety of breach of this Agreement, or of any portion therefore, and default of Developer.
- 18.2. In the event of any such notice, Developer's surety shall have the duty to take over and complete the work and the improvement herein specified; provided, however, that if the surety, within five (5) days after the serving upon of such notice of breach, does not give the District written notice of its intention to take over the performance of the contract, and does not commence performance thereof within five (5) days after notice to the District of such election, District may take over the work and prosecute the same to completion, by contract or by any other method District may deem advisable, for the account and at the expense of Developer, and Developer's surety shall be liable to the District for any excess cost or damages occasioned District thereby; and, in such event, District, without liability for so doing, may take possession of, and utilize in completing the work, such materials, appliances, plant and other property belonging to Developer as may be on the site of the work and necessary therefore.

### 19. SUCCESSORS BOUND

19.1. This Agreement shall be binding upon and inure to the benefit of each of the parties and their respective legal representatives, successors, heirs, and assigns.

### 20. ENFORCEMENT OF PROVISIONS

20.1. The District's failure to enforce any provisions of this Agreement or the waiver thereof in any instance shall not be construed as a general waiver or relinquishment on its part of any such provision, but the same shall nevertheless be and remain in full force and effect.

[CONTINUED ON NEXT PAGE]

IN WITNESS	WHEREOF,	the	parties	hereto	execute	this .	Agreement.
	- ,						0

### WEST VALLEY WATER DISTRICT

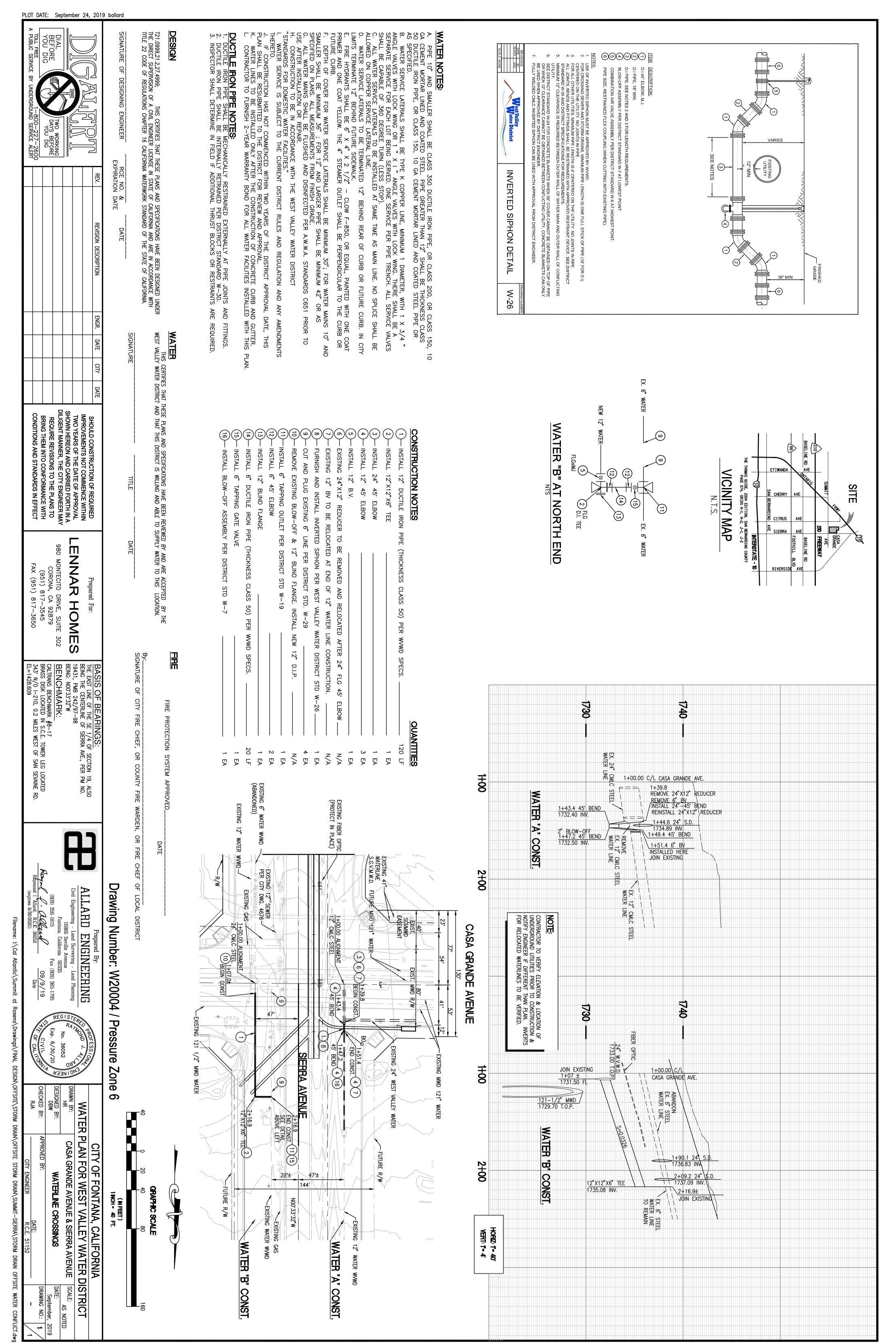
By:		Date:
•	Clarence C. Mansell, Jr., General Manager	_
DEVE	LOPER:	
CALA	TLANTIC GROUP, INC	
By:	CALATLANTIC GROUP, INC	
	a Delaware Corporation	
By:		Date:
	Geoff Smith	
	Authorized Agent	

### Exhibit A





### Exhibit B



### Exhibit C

### Gabbion Ranch Sierra and Casa Grande - LENNAR HOMES WEST VALLEY WATER DISTRICT Improvement Bond Calculation

30-Sep-19

Prepared in the office of **ALLARD ENGINEERING** 16866 Seville Avenue Fontana, CA. 92335

Phone: 909-356-1815

ITEM	Quantity	Unit	Price	TOTAL
MOBILIZATION	1	LS	\$5,000	\$5,000
FURNISH & INSTALL 12" DUCILE IRON PIPE,	120	LF	\$90	\$10,800
FURNISH & INSTALL 6" DUCILE IRON PIPE	20	LF	\$40	\$800
FURNING & INSTALL 12" BUTTERFLY VALVE	1	EA	\$2,000	\$2,000
FURNISH AND INSTALL BLOW OFF ASSEMBLY	1	EA	\$2,000	\$2,000
Sub-Total				\$20,600
Contingency (20%)	20%			\$4,120
TOTAL				\$24,720
BOND AMOUNT				\$25,000

### Exhibit D

### BOARD OF DIRECTORS

Dr. Michael Taylor
President, Board of Directors
Kyle Crowther
Vice President, Board of Directors
Dr. Clifford O. Young, Sr.
Director
Greg Young
Director
Donald Olinger
Director

Crystal L. Escalera

**Board Secretary** 



### ESTABLISHED AS A PUBLIC AGENCY IN 1952

WEST VALLEY WATER DISTRICT'S MISSION IS TO PROVIDE A RELIABLE, SAFE-DRINKING WATER SUPPLY TO MEET OUR CUSTOMERS' PRESENT AND FUTURE NEEDS AT A REASONABLE COST AND TO PROMOTE WATER-USE EFFICIENCY AND CONSERVATION.

ADMINISTRAT 3.g.b

Clarence C. Mansell, Jr.
General Manager
Ricardo Pacheco
Assistant General Manager
Jeremiah Brosowske
Assistant General Manager
Deborah L. Martinez
Human Resources and
Risk Manager

Shamindra K. Manbahal Chief Financial Officer

### **2018 HOLIDAY LIST**

MONDAY, DECEMBER 24 CHRISTMAS EVE TUESDAY, DECEMBER 25 CHRISTMAS MONDAY, DECEMBER 31 NEW YEAR'S EVE

### **2019 HOLIDAY LIST**

TUESDAY, JANUARY 1 **NEW YEAR'S DAY** MONDAY, JANUARY 21 MARTIN LUTHER KING, JR. MONDAY, FEBRUARY 18 PRESIDENT'S DAY MONDAY, MAY 27 **MEMORIAL DAY** INDEPENDENCE DAY THURSDAY, JULY 4 MONDAY, SEPTEMBER 2 LABOR DAY **VETERANS DAY (OBSERVED)** MONDAY, NOVEMBER 11 THANKSGIVING THURSDAY, NOVEMBER 28 FRIDAY, NOVEMBER 29 DAY AFTER THANKSGIVING TUESDAY, DECEMBER 24 CHRISTMAS EVE WEDNESDAY, DECEMBER 25 **CHRISTMAS** 

### **2020 HOLIDAY LIST**

**NEW YEAR'S EVE** 

WEDNESDAY, JANUARY 1 NEW YEAR'S DAY
MONDAY, JANUARY 20 MARTIN LUTHER KING, JR.

TUESDAY, DECEMBER 31



### BOARD OF DIRECTORS ENGINEERING AND PLANNING COMMITTEE STAFF REPORT

DATE: October 9, 2019

TO: Engineering and Planning Committee FROM: Clarence Mansell Jr., General Manager

SUBJECT: CONSIDER WATER SYSTEM INFRASTRUCTURE INSTALLATION

AND CONVEYANCE AGREEMENT WITH LENNAR HOMES OF

CALIFORNIA, INC FOR TRACT 20250

### **BACKGROUND:**

Lennar Homes of California, Inc. ("Developer") is the owner of land located at the northeast corner of Glen Helen Parkway and Sycamore Creek Drive in the unincorporated area San Bernardino County, known as Tract No. 20250, Rosena Ranch ("Development"), as shown in **Exhibit A**. The Development is part of a master-planned community and contains 78 residential lots requiring water services. In developing this land, the Developer is required to construct a new water main within the tract to allow for new domestic and irrigation connections.

### **DISCUSSION:**

In order to construct the water facilities needed to supply water to the Development, West Valley Water District ("District") and the Developer wish to enter into a Developer-Installed Water System Infrastructure Installation and Conveyance Agreement ("Agreement"). This Agreement outlines the responsibilities of the Developer in constructing facilities, including insurance, indemnification and bonding requirements as well as conveyance and acceptance of the water system by the District. Attached as **Exhibit B** is a copy of the Water System Infrastructure Installation and Conveyance Agreement for this development.

### **FISCAL IMPACT:**

No fiscal impact to the District.

### **STAFF RECOMMENDATION:**

It is recommended that the Engineering, Operations and Planning Committee approve the Water System Infrastructure Installation and Conveyance Agreement with Lennar Homes of California, Inc. and have this item considered by the full Board of Directors at a future meeting.

Respectfully Submitted,

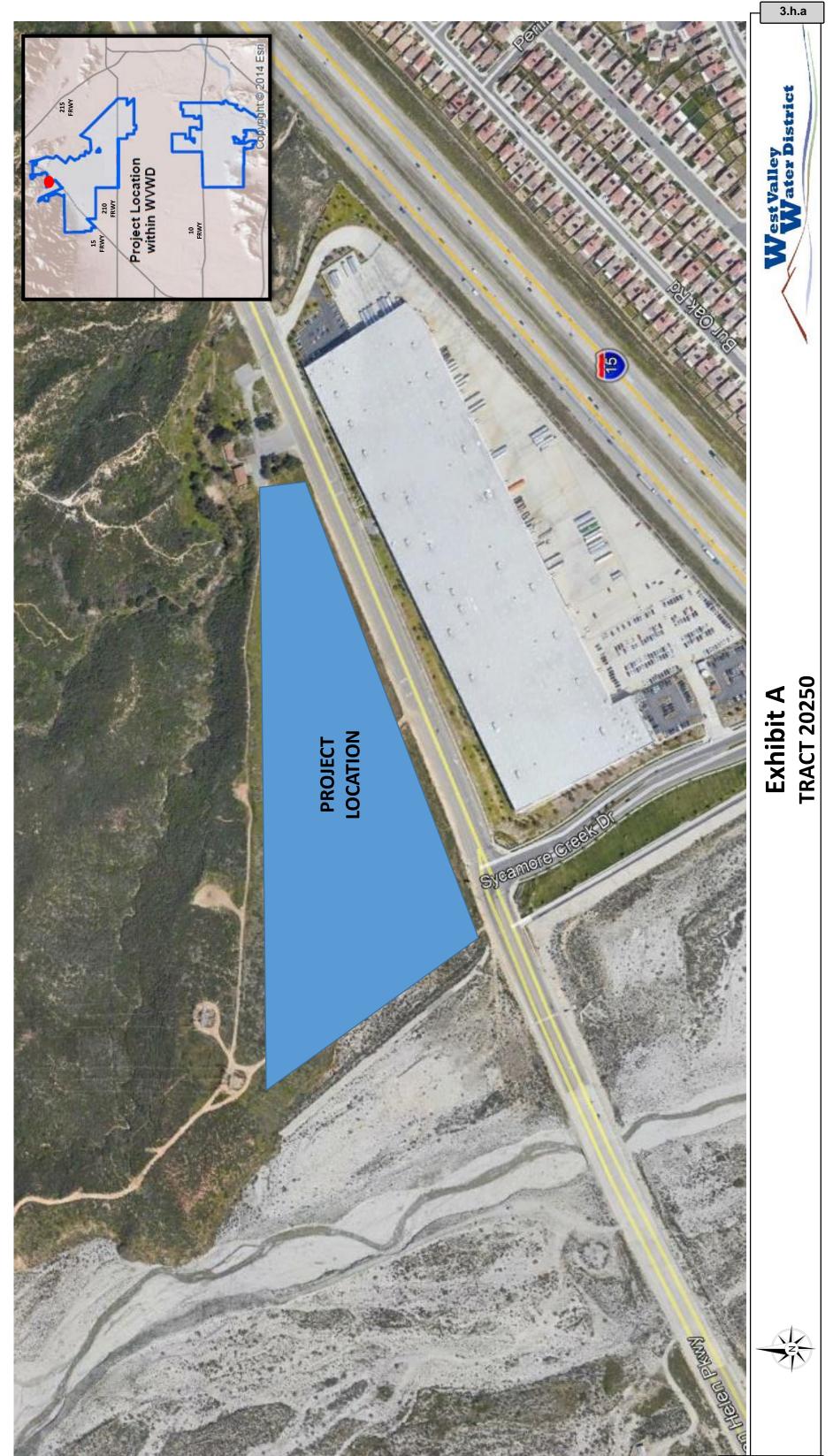
Clarence Mansell Jr, General Manager

DG:ce

### ATTACHMENT(S):

- 1. Exhibit A Aerial Map
- 2. Exhibit B Water System Infrastructure Installation and Conveyance Agreement with Lennar Homes of California, Inc

### **EXHIBIT A**



Packet Pg. 238

### **EXHIBIT B**

### WATER SYSTEM INFRASTRUCTURE INSTALLATION AND CONVEYANCE AGREEMENT

This water system infrastructure installation and conveyance agreement ("Agreement") is entered into and effective as of \_\_\_\_\_\_ by and between **LENNAR HOMES OF CALIFORNIA**, **INC** ("Developer"), and **WEST VALLEY WATER DISTRICT** ("District") who agree as follows:

The Developer is the owner of certain land described as **TRACT 20250** and as more fully (or further) shown on <u>Exhibit "A"</u>. In developing this land, the Developer is desirous of obtaining a public water supply adequate for domestic uses and public fire protection purposes and is desirous of integrating that water system into the District's public water system.

In order to provide facilities for a water supply to said land, it is the intention of the parties to this Agreement that the Developer shall furnish and install those water mains, fire hydrants, service laterals, water meters and valves, valve boxes, and all other appurtenant fittings and facilities required for a complete water system to serve the land shown on Exhibit "A".

In order to implement the foregoing and in consideration of the terms and conditions herein contained, the parties further agree as follows:

### 1. DESIGN

- 1.1. Developer shall design and construct, at the Developer's own expense, the water facilities and appurtenances required to serve the development in accordance with final District approved plans known as **WATER IMPROVEMENT PLAN FOR TRACT MAP NO. 20250**, as approved and attached herein as <u>Exhibit "B"</u> and in accordance with District-approved design standards and specifications, and the terms and conditions of this Agreement.
- 1.2. The water system design shall be by a Professional Engineer registered in the State of California, and in accordance with the District's most recent Rules and Regulations (the "Rules and Regulations"), the District's Standards for Domestic Water Facilities and Standard Drawings herein included by reference, all applicable District ordinances and policies and all City, County of San Bernardino, State of California, and Federal laws, ordinances, rules, regulations, codes and other legal requirements of all government bodies having jurisdiction over said construction and property (all of the foregoing requirements in this paragraph being collectively referred to herein at times as "Legal Requirements").
- 1.3. The District, at Developer's expense, shall review Developer's plans for the purpose of ensuring the adequacy of the design and conformance with the District's standards and specifications. The District reserves the right to add, delete, modify, change or amend any or all the plans and specifications.
- 1.4. In the event that the property to be developed includes multiple residential, condominiums, commercial or industrial uses, all site plans, grading plans, and any available plumbing plot plans shall be furnished to the District by Developer.

1.5. The District makes no warranties as to the correctness, accuracy or completeness of the plans and specifications. The accuracy, adequacy, suitability, and correctness of the water system design shall be the sole responsibility of the Developer.

### 2. CONSTRUCTION

- 2.1. Developer shall perform, or caused to be performed, all construction of the water system infrastructure installation pursuant to the approved water system plans, and all Legal Requirements.
- 2.2 The performance of this Agreement shall commence within ninety (90) calendar days from the date of this Agreement and shall be completed within one (1) year from the estimated construction start date.
- 2.3. Time is of the essence in this Agreement; provided that, in the event good cause is shown therefore, the general manager of the District ("General Manager") may extend the time for completion of the water system installation. Any such extension may be granted without the notice to Developer's surety, and extensions so granted shall not relieve the surety's liability on the bond to secure faithful performance of this Agreement. The General Manager shall be the sole and final judge as to whether or not good cause has been shown to entitle Developer to an extension.
- 2.4. The Developer and its contractor and subcontractors shall attend a preconstruction meeting with the District at the District's headquarters no less than five (5) working days prior to commencement of construction.
- 2.5. No work on water facilities shall commence prior to the completion of all required curbs and gutters.

### 3. LICENSES AND PERMITS

- 3.1. Developer, and all of Developer's contractors and subcontractors warrants it possesses, or shall obtain, and maintain during the term of this Agreement any and all licenses, permits, qualifications, insurance and approval of whatever nature that are legally required of Developer, its contractors, and all subcontractors to practice its profession, skill or business.
- 3.2. The work to be performed under this Agreement, except meter installations, shall be performed by Developer, or a contractor or subcontractor who is pre-approved by the District and is licensed under the laws of the State of California in the specialty Class of "C-34" Pipeline or Class "A" General Engineering. A copy of the contract between Developer and the selected pre-approved contractor and all subcontractors shall be submitted to the District for review and approval.
- 3.3. Excavation/resurfacing permits shall be secured by Developer at Developer's expense. Permits/easements to install, maintain and operate water system facilities in private property shall be secured by Developer at Developer's own expense prior to construction.

3.4. Developer shall, at Developer's own expense, be responsible for obtaining and adhering to a National Pollution Discharge Elimination System (NPDES) permit from the Regional Water Quality Board as required for construction or pipeline flushing and disinfection.

### 4. INSURANCE REQUIREMENTS

- 4.1. The following insurance requirements have been adopted by the District and shall be applicable to this Agreement. These requirements supersede the insurance requirements set forth in any other reference of the District, and to the extent of any conflict, the specified requirements herein shall prevail.
- 4.2. Developer shall ensure that Developer's contractors conform to the following insurance requirements and that all required documents are submitted to the District at the time of Agreement submittal: Developer shall ensure that its contractors and all subcontractors shall purchase and maintain insurance in amounts equal to the requirements set forth in (a) through (d) below, and shall not commence work under this Agreement until all insurance required under this heading is obtained in a form acceptable to the District, nor shall Developer allow any contractor or subcontractor to commence construction pursuant to a contract or subcontract until all insurance required of the contractor and any subcontractors has been obtained.
- a. <u>General Liability</u>: Developer shall ensure that its contractor and all subcontractors shall maintain during the life of this Agreement, a standard form of either Comprehensive General Liability insurance or Commercial General Liability insurance ("General Liability Insurance") providing the following minimum limits of liability: Combined single limit of \$1.0 million per occurrence for bodily injury, including death, personal injury, and property damage with \$2.0 million minimum aggregate, separate for this project as evidenced by endorsement. The insurance shall include coverage for each of the following hazards: Premises-Operations; Owners and Contractors Protective; Broad Form Property Damage contractual for Specific Contract; Severability of Interest or Cross-Liability; XCU Hazards; and Personal Injury With the "Employee" Exclusive Deleted.
- b. <u>Automotive/Vehicle Liability Insurance</u>: Developer shall ensure that its contractor and all subcontractors shall maintain a policy of automotive/vehicle liability insurance on a commercial auto liability form covering owned, non-owned and hired automobiles providing the following minimum limits of liability: Combined single limit of liability of \$1.0 million per accident for Bodily Injury, Death and Property Damage ("Automotive/Vehicle Liability Insurance").
- c. <u>Workers' Compensation Insurance</u>: Developer shall ensure that its contractor and all subcontractors shall provide such workers' compensation insurance with statutory minimum amounts of coverage, as required by the California *Labor Code* and other applicable law, and including employer's liability insurance with a minimum limit of \$1,000,000.00 ("Workers' Compensation Insurance"). Such Workers' Compensation Insurance shall be endorsed to provide for a waiver of subrogation against the District.
- d. <u>Excess Liability</u>: Developer shall ensure that its contractor and all subcontractors shall provide a policy providing excess coverage in a face amount necessary when

combined with the primary insurance, to equal the minimum requirements for General Liability Insurance and Automotive/Vehicle Liability Insurance.

- 4.3. The insurances provided for in Section 4.2 and its subsections above are subject to all of the following conditions:
- a. The insurance shall be issued and underwritten by insurance companies acceptable to the District, and shall be licensed by the State of California to do business on the lines of insurance specified. The insurers must also have an "A-" Policyholder's rating" and a "financial rating" of at least Class VII in accordance with the most current A.M. Best's Rating Guide.
- b. Developer's contractor and subcontractors may satisfy the limit requirements in a single policy or multiple policies. Any such additional policies written as excess insurance shall not provide any less coverage than that provided by the first or primary policy.
- c. Any costs associated with a self-insured program, deductibles, or premium rating programs that determine premium based on loss experience shall be for the account of Developer, Developer's contractor and subcontractors, and the District shall not be required to participate in any such loss. If any such programs exist, Developer, Developer's contractor and subcontractors, agree to protect and defend the District in the same manner as if such cost provisions were not applicable.
- d. Developer shall ensure that its contractor and all subcontractors shall have presented at the time of execution of the Agreement, the original policies of insurance and a certificate of insurance naming the District as the certificate holder and that such coverage is in force and complies with the terms and conditions outlined herein.
- e. If an insurance policy contains a general policy aggregate of less than the minimum limits specified, then the policy coverage shall be written with limits applicable solely to this Agreement, as specified, and shall not be reduced by or impaired by any other claims arising against Developer. These policy limits shall be set forth by separate endorsement to the policy.
- 4.4. Each such policy of General Liability Insurance and Automotive/Vehicle Liability Insurance shall contain endorsements providing the following:
- a. The District, their board members, officers, agents, employees, consultants, and engineers, are hereby declared to be additional insureds under the terms of this policy, but only with respect to the operations of the Developer at or upon any of the premises of the District in connection with the Agreement with the District, or acts or omissions of the additional insureds in connection with, but limited to its general supervision or inspection of said operations and save for any claims arising from the sole negligence or sole willful misconduct the District.
- b. No policy shall be canceled, limited, materially altered, or non-renewed by the insurer until thirty (30) days after receipt by the District of a written notice of such cancellation or reduction in coverage.
- c. This insurance policy is primary insurance and no insurance held or owned by the designated additional insureds shall be called upon to cover a loss under this policy.

### 5. BONDING REQUIREMENTS

- 5.1. Developer shall generate an engineer's cost estimate based on the water system plans provided to the District. The estimated costs, attached herein as <u>Exhibit "C"</u>, will be submitted to the District for review and approval, and shall be used as the basis for bonding requirements for the water system described in the plans provided to the District by the Developer and approved for construction by the District. The sole intent and purpose of the engineer's estimate is to establish a cost valuation for bonding purposes only.
- 5.2. Performance Bond: The Developer's engineers estimate for the WATER IMPROVEMENT PLAN FOR TRACT MAP NO. 20250, is THREE HUNDRED EIGHTY-SEVEN THOUSAND FIVE HUNDRED TWENTY-FOUR DOLLARS AND 88/100 (\$387,524.88). Developer shall and by this Agreement does guarantee the Developer's faithful performance of this Agreement and all of its terms and conditions by providing the following: Developer shall provide the District with either an irrevocable letter of credit from a recognized financial institution acceptable to the District or a performance bond, from a surety institution licensed by the State of California and authorized to do and doing business in said State, valid and renewable until such improvements are accepted by the District. The irrevocable letter of credit or performance bond shall be in the amount of THREE HUNDRED EIGHTY-SEVEN THOUSAND FIVE HUNDRED TWENTY-FOUR DOLLARS AND 88/100 (\$387,524.88). equal to 100 percent of the approved Developer's estimate.
- 5.3. <u>Warranty Bond</u>: The Developer's pre-approved contractor shall furnish a two-year warranty bond for all work completed in accordance with the approved plans (<u>Exhibit "B"</u>). Before District's acceptance of the completed water facilities and appurtenances, such facilities and appurtenances shall be free from any and all liens and encumbrances and free from any and all defects in the materials or construction thereof. The two-year warranty shall be either an irrevocable letter of credit from a recognized financial institution acceptable to the District or a warranty bond beginning on the date of acceptance of the water facilities by the District.

### 6. MATERIALS

6.1. The water system facilities to be installed pursuant to this Agreement shall become an extension of the distribution system of the District. All materials used must conform to District specifications for such materials pursuant to all applicable Legal Requirements.

### 7. NOTICES

- 7.1. All notices herein required shall be in writing, and delivered in person or sent by registered mail, postage prepaid.
  - 7.2. Notices required shall be given to the **District** addressed as follows:

WEST VALLEY WATER DISTRICT

Attn: General Manager Post Office Box 920 Rialto, CA 92377

RE: WATER IMPROVEMENT PLAN FOR TRACT MAP NO. 20250

7.3. Notices required shall be given to **Developer** addressed as follows:

LENNAR HOMES OF CALIFORNIA, INC ATTENTION: GEOFF SMITH 980 MENTECITO DRIVE, SUITE 206 CORONA, CA 92879

RE: WATER IMPROVEMENT PLAN FOR TRACT MAP NO. 20250

7.4. Notices required shall be given to **Surety** addressed as follows:

**SURETY NAME:** 

ATTN TO:

**ADDRESS** 

RE: WATER IMPROVEMENT PLAN FOR TRACT MAP NO. 20250

- 7.5. Provided that any party or Surety may change such address by notice in writing to the other party, and thereafter, notices shall be addressed and transmitted to the new address.
- 7.6. The Developer or its contractor shall provide the District forty-eight (48) hours advance notice of request for inspection or testing.
  - 7.7. The District is closed on the holidays listed in Exhibit "D".

### 8. NOTICE TO PROCEED TO CONSTRUCT WATER SYSTEM FACILITIES

8.1. Upon acceptance of the insurance and aforementioned bonds and/or irrevocable letters of credit in the amounts provided herein and approval by the District and upon payment of all applicable charges, the Agreement shall be signed by Developer and the District. The District shall return an original copy of the signed Agreement with a letter to Developer giving notice to proceed to construct the water system facilities.

### 9. INSPECTION

- 9.1. It is understood that the sole purpose and intent of the District's inspection and testing is to validate that the materials, workmanship, and construction of the water facilities are in compliance with the District-approved final plans, the District's Rules and Regulations, the Standards for Domestic Water Facilities, the Standard Drawings, and all other applicable District requirements. Developer acknowledges and represents that it assumes full and sole responsibility for the safety and management of the project.
- 9.2. Developer shall at all times maintain proper facilities and provide safe access for inspection by the District to all parts of the work and to the shops wherein the work is in preparation. Additionally, in connection with the performance of this Agreement, the District shall have the authority to enter the work site at any time for the purpose of identifying the existence of conditions, either actual or threatened, that may present a danger of hazard to any and all employees. Developer agrees that the District, in its sole authority and discretion, may order

the immediate abatement of any and all conditions that may present an actual or threatened danger or hazard to any and all employees at the work site. Furthermore, Developer acknowledges the provisions of California *Labor Code* Section 6400 et seq., which requires that employers shall furnish employment and a place of employment that is safe and healthful for all employees working therein. In the event the District identifies the existence of any condition that presents an actual or threatened danger or hazard to any or all employees at the work site, the District is hereby authorized to order an immediate abatement of that condition.

- 9.3. All work and materials shall be subject to inspection, testing, and acceptance by the District at Developer's expense. In the event Developer arranges to have materials fabricated for the project, Developer may be required to arrange for the District to inspect that material during fabrication at Developer's expense.
- 9.4. All material fabrications shall be preapproved by the District and must conform to District standards and specifications.
- 9.5. The District's inspectors shall have full, unlimited access to perform continuous inspection and have the authority to stop work at any time, by written notice, without any liability whatsoever to the District, if, in the inspectors' judgment, the work called for by this Agreement, or the District approved plans, or the specifications is not being installed or performed in a satisfactory and workmanlike manner according to District's standards and specifications and/or in the event the materials do not comply with the District's standards and specifications.
- 9.6. Final acceptance of all material to be purchased or fabricated by Developer under this Agreement shall be made only with the prior approval of the District. Approval by the District, however, shall not operate to relieve the material supplier or Developer of any guarantees, warranties, or the duty of compliance with any of the requirements of the approved plans and specifications or of this Agreement. All construction pursuant to this Agreement shall be inspected pursuant for conformity with District requirements. Developer shall pay actual costs for inspections.

### 10. TESTING AND DISINFECTION

10.1. All water system facilities and components constructed pursuant to this Agreement shall adhere to all requirements for testing, disinfection, and flushing pursuant to District standards and Legal Requirements.

### 11. RELOCATIONS, RECONSTRUCTIONS, AND DAMAGES

11.1. Developer accepts the responsibility for and the costs occasioned by any reconstruction, relocation, damages to, or changes of water services or facilities caused or contributed to directly or indirectly by any subsequent changes in the location of any of said facilities or water meters or water services.

### 12. AS-CONSTRUCTED DOCUMENTATION

12.1. In order for the District to accept the facilities, Developer shall provide all required documentation as specified in the Standards for Domestic Water Facilities, including As-Built drawings.

### 13. INDEMNIFICATION

- 13.1. Developer hereby agrees to and shall protect, defend, indemnify and hold the District and its board members, officers, agents, employees, and engineers free and harmless from any and all liability losses, damages, claims, liens, demands and cause of action of every kind and character including, but not limited to, the amounts of judgments, penalties, interests, court costs, attorney's/legal fees, and all other expenses incurred by the District arising in favor of any party, including claims, liens, debts, demands for lost wages or compensation, personal injuries, including employees or the District, death or damages to property (including property of the District) and without limitation by enumeration, all other claims or demands of every character occurring or in any way incident to, in connection with or arising directly or indirectly out of the obligations herein undertaken or out of the operations conducted by Developer save and except claims or litigation arising through the sole negligence or sole willful misconduct of the District or the District's agents and employees. Developer shall investigate, handle, respond to, provide defense for and defend any such claims, demand, or suit at the sole expense of Developer even if the claim or claims alleged are groundless, false or fraudulent. Developer agrees to, and shall defend the District and its members, directors, officers, agents, employees, and engineers from any suits or actions at law or in equity for damages caused, or alleged to have been caused, by reason of any of the aforesaid operations, provided as follows:
- a. That the District does not and shall not waive any rights against Developer which it may have by reason of the aforesaid hold harmless agreement, because of the acceptance by the District, or the deposit with District by Developer, or any of the insurance policies described in this Agreement.
- b. That the aforesaid hold harmless agreement by Developer shall apply to all damages and claims for damages of every kind suffered, or alleged to have been suffered, by reason of any or the aforesaid operations referred to in this subsection, regardless of whether or not District has prepared, supplied water system installation, or regardless of whether or not such insurance policies shall have been determined to be applicable to any such damages or claims for damages.

This provision is not intended to create any cause of action in favor of any third party against Developer or the District or to enlarge in any way Developer's liability but is intended solely to provide for indemnification of the District from liability for damage or injuries to third persons or property arising from Developer's performance hereunder.

13.2. Neither Developer nor any of Developer's agents, contractors or subcontractors are, or shall be, considered to be agents of the District in connection with the performance of Developer's obligations under this Agreement.

### 14. REPAIR OR RECONSTRUCTION OF DEFECTIVE WORK

14.1. If, within a period of two years after final acceptance of the work performed under this Agreement, any structure or part of any structure furnished and/or installed or constructed, or caused to be installed or constructed by Developer, or any of the work done under this Agreement, fails to fulfill any of the requirement of this Agreement or the specifications referred to herein, Developer shall, without delay and without any cost to District, repair or replace or reconstruct any defective or otherwise unsatisfactory part or parts of the work structure. Should Developer fail to act promptly or in accordance with this requirement, or should the exigencies of the situation as determined by the District in the exercise of its sole discretion require repair, replacement or reconstruction before Developer can be notified, District may, at its option, make the necessary repairs or replacements or perform the necessary work, and Developer shall pay to the District the actual cost of such repairs.

### 15. COSTS AND FEES

- 15.1. Developer shall be responsible for all fees and deposits as required by the District. All fees and deposits shall be paid in full prior to the execution of this Agreement and before construction can take place.
- 15.2. Any additional costs and fees shall be paid in full prior to conveyance and acceptance of the water system.

### 16. CONVEYANCE AND ACCEPTANCE OF WATER SYSTEM

- 16.1. Upon completion of the water system in accordance with the approved water plans and submission of the required documentation, the Developer shall convey the water system to the District.
- 16.2. The Developer shall be responsible for insuring the pre-approved contractor furnish an irrevocable letter of credit to the District or a warranty bond (One Hundred (100%) of Developer's estimate) for a period of two years as stated in Sections 5.3 of this Agreement, asbuilt drawings with contractor redlines and AutoCAD files, materials list with quantities, labor, equipment, and materials, water system cost breakouts, compaction test report signed and sealed by a California Registered Engineer, notice of completion filed with San Bernardino County Recorder, fire flow tests of all hydrants, all required easements for water facilities and unconditional financial release from subcontractors and material providers, Upon compliance with all the terms and conditions of this Agreement, the District shall prepare the conveyance agreement accepting the water facilities and forward same to the address provided herein. Title to the ownership of said facilities and appurtenances shall thereby be conveyed to the District. The District shall thereafter operate and maintain said facilities so as to furnish water service to the development (Exhibit "A") in accordance with the District's ordinances, policies and Rules and Regulations.

### 17. PERMANENT WATER SERVICE

17.1. In no event shall permanent water services be provided to Developer's installed system until all applicable charges and fees have been paid by Developer and all facilities have

been conveyed, free of all encumbrances, to the District, including any easements which may be required. Such conveyance shall occur in a timely manner in accordance with the terms of this Agreement.

### 18. BREACH OR DEFAULT OF AGREEMENT

- 18.1. If Developer refuses or fails to obtain prosecution of the work, or any severable part thereof, with such diligence as will insure its completion within the time specified, or any extension thereof, or fails to obtain completion of said work within such time, or if Developer should be adjudged as bankrupt, or Developer should make a general assignment for the benefit of Developer's creditors, or if a receiver should be appointed in the event of Developer's insolvency, or if Developer, or any of Developer's contractors, subcontractors, agents or employees, should violate any of the provisions of this Agreement, the District's General Manager or the General Manager's designee may serve written notice upon Developer and Developer's surety of breach of this Agreement, or of any portion therefore, and default of Developer.
- 18.2. In the event of any such notice, Developer's surety shall have the duty to take over and complete the work and the improvement herein specified; provided, however, that if the surety, within five (5) days after the serving upon of such notice of breach, does not give the District written notice of its intention to take over the performance of the contract, and does not commence performance thereof within five (5) days after notice to the District of such election, District may take over the work and prosecute the same to completion, by contract or by any other method District may deem advisable, for the account and at the expense of Developer, and Developer's surety shall be liable to the District for any excess cost or damages occasioned District thereby; and, in such event, District, without liability for so doing, may take possession of, and utilize in completing the work, such materials, appliances, plant and other property belonging to Developer as may be on the site of the work and necessary therefore.

### 19. SUCCESSORS BOUND

19.1. This Agreement shall be binding upon and inure to the benefit of each of the parties and their respective legal representatives, successors, heirs, and assigns.

### 20. ENFORCEMENT OF PROVISIONS

20.1. The District's failure to enforce any provisions of this Agreement or the waiver thereof in any instance shall not be construed as a general waiver or relinquishment on its part of any such provision, but the same shall nevertheless be and remain in full force and effect.

[CONTINUED ON NEXT PAGE]

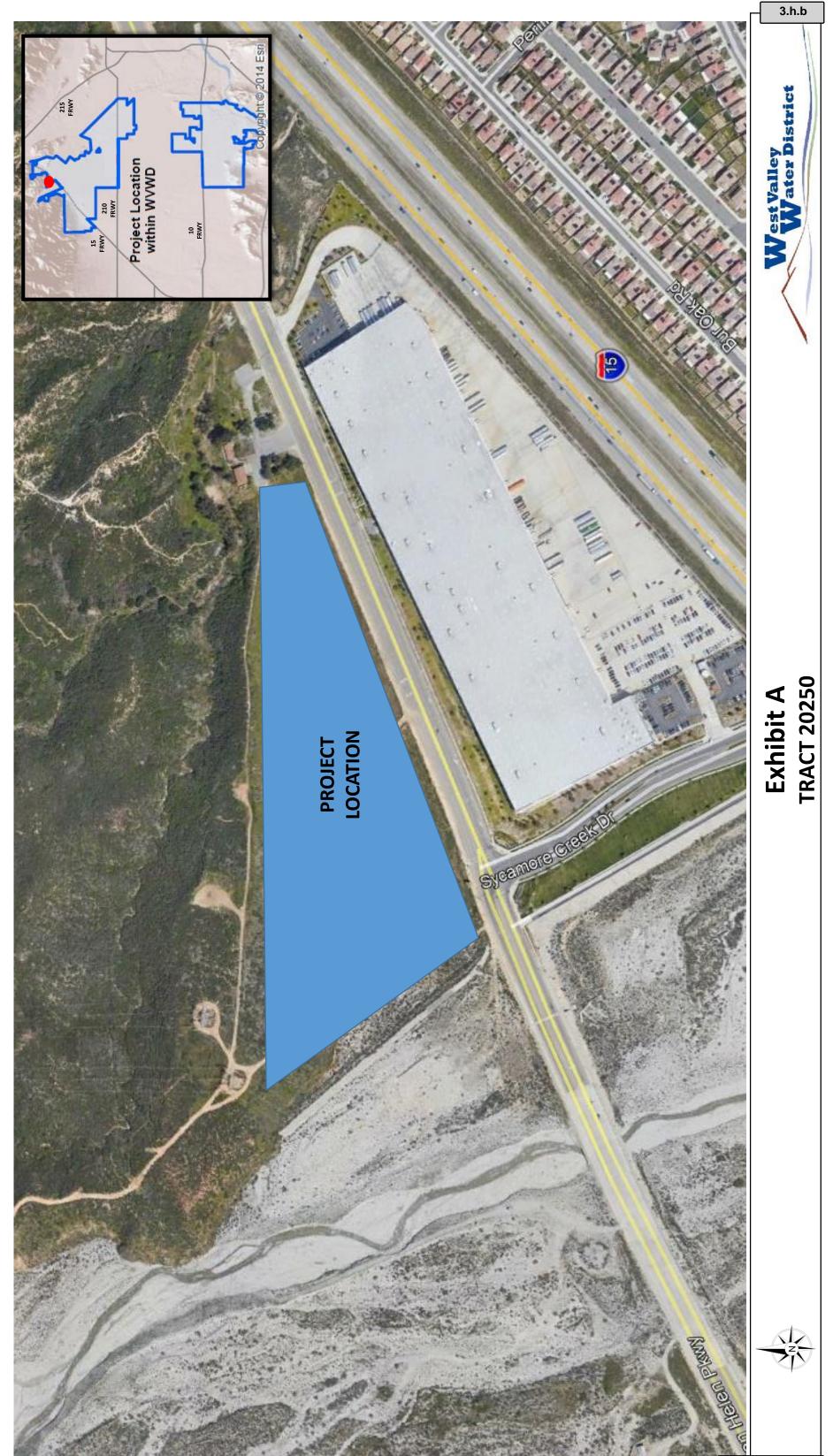
IN WIT	NESS WHEREOF, the parties hereto execute this Agreen	nent.
WEST	VALLEY WATER DISTRICT	
By:	Clarence C. Mansell, Jr., General Manager	Date:
	LOPER: AR HOMES OF CALIFORNIA, INC	

By: LENNAR HOMES OF CALIFORNIA, INC a California Corporation

By: Date:

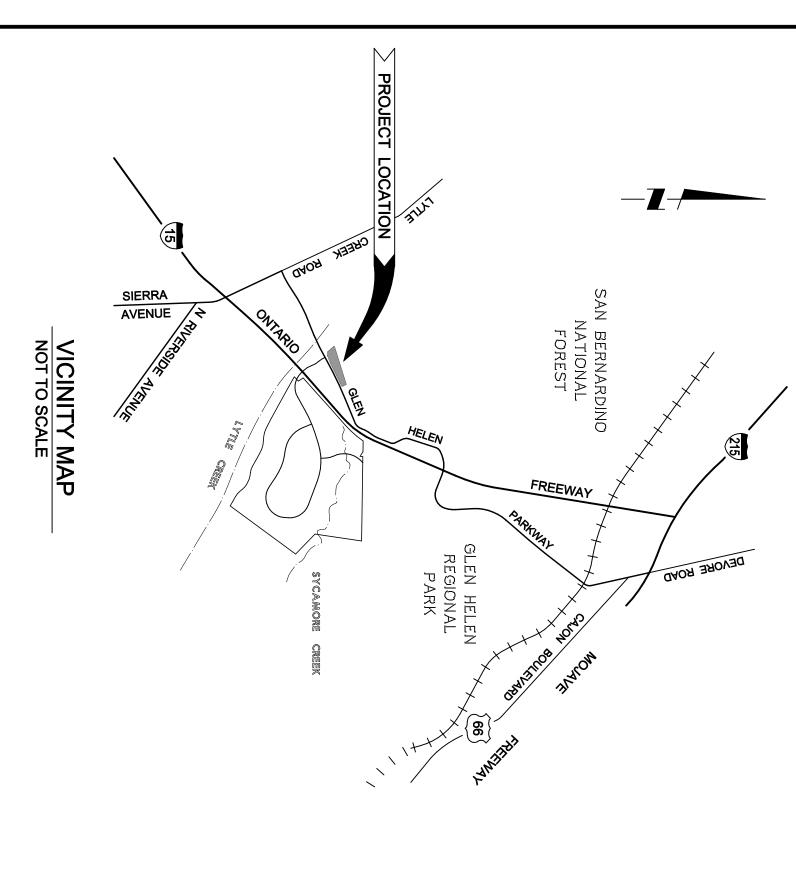
Geoff Smith Authorized Agent

### Exhibit A



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### Exhibit B



### (<del>-</del>) FURNISH & INSTALL 8", CLASS 350 DUCTILE IRON PIPE PER . DISTRICT SPECIFICATIONS WATER CONSTRUCTION ITEMS QUANTITIES 1,676 듞

@ 4 **6** 5 4 (e) CONSTRUCT CONCRETE THRUST BLOCK PER WEST VALLEY WATER. DISTRICT STANDARD DRAWING No. W-3A & W-3B TRENCH PATCH AND PAVEMENT REPAIR PER WEST VALLEY WATER . DISTRICT STANDARD DRAWING No. W-1 FURNISH & INSTALL 8" FULLY-SEATED RESILIENT GATE VALVE . . . . . . PER WEST VALLEY STANDARD DRAWING No. W-11 FURNISH & INSTALL FIRE HYDRANT ASSEMBLY PER WEST VALLEY WATER ... DISTRICT STANDARD DRAWING No. W-2 FURNISH & INSTALL 8" FITTING OR BEND, ANGLE AS INDICATED... FURNISH AND INSTALL 16" × 8" TAPPING SLEEVE WITH 8" R.S. GATE VALVE AND HOT TAP CONNECTION TO EXISTING 16" WATER MAIN PER WEST VALLEY STANDARD DRAWING No. W-28 9

(3)

 $\odot$ 

(%)

FURNISH & INSTALL 1" SERVICE LATERAL(S) WITH 1" METER(S) PER WEST A VALLEY WATER DISTRICT STANDARD DRAWING No. W-4 AND DETAIL  $\frac{A}{2}$ 

78

EA

ice)

E

**a** FURNISH AND INSTALL NEW 4" AIR VALVE PER WEST VALLEY WATER DISTRICT STANDARD DRAWING No. W-6B. SALVAGE EXISTING AIR VALVE AND LID AS DIRECTED BY WVWD INSPECTOR ALL DUCTILE IRON JOINTS AND FITTINGS SHALL BE MECHANICALLY RESTRAINED PER PLAN. DUCTILE IRON PIPE SHALL BE RESTRAINED PER WEST VALLEY STANDARD DRAWING No. W-30. INSPECTOR SHALL DETERMINE IN FIELD IF ADDITIONAL THRUST BLOCKS OR RESTRAINTS ARE REQUIRED

THE QUANTITIES SHOWN HEREON ARE PROVIDED ONLY FOR THE USE OF THE OWNER. PROSPECTIVE BIDDERS SHALL MAKE THEIR OWN DETERMINATION OF THE QUANTITIES IN REQUIRED, AND SHALL BASE THEIR BID ON THEIR OWN ESTIMATE.

## NOTICE TO CONTRACTOR(S)

THE EXISTENCE AND LOCATION OF ANY UNDERGROUND UTILITY PIPES, CONDUITS OR STRUCTURES SHOWN ON THESE PLANS WERE OBTAINED BY A SEARCH OF THE AVAILABLE RECORDS. TO THE BEST OF OUR KNOWLEDGE, THERE ARE NO EXISTING UTILITIES EXCEPT AS SHOWN ON THESE PLANS. THE CONTRACTOR IS REQUIRED TO TAKE DUE PRECAUTIONARY MEASURES TO PROTECT THE UTILITY LINES SHOWN ON THESE DRAWINGS. THE CONTRACTOR FURTHER ASSUMES ALL LIABILITY AND RESPONSIBILITY FOR THE UTILITY PIPES, CONDUITS OR STRUCTURES SHOWN OR RESPONSIBILITY FOR THE UTILITY F

CONTRACTOR AGREES THAT HE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY; THAT THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS; THAT THE CONTRACTOR SHALL DEFEND, INDEMNIFY AND HOLD THE COUNTY, THE OWNER, AND THE ENGINEER HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT EXCEPTING FOR LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR THE ENGINEER.

## MONUMENT PRESERVATION NOTE

SURVEY MONUMENTS THAT EXIST AS SHOWN ON RECORD MAPS, HIGHWAY MAPS, OR POINTS THAT PROVIDE SURVEY CONTROL WITHIN THE CONSTRUCTION AREA, SHALL BE LOCATED AND REFERENCED OUT BY A LICENSED LAND SURVEYOR OR REGISTERED CIVIL ENGINEER (AUTHORIZED TO PRACTICE LAND SURVEYING), AND CORNER RECORDS SHALL BE FILED WITH THE COUNTY SURVEYOR PRIOR TO THE START OF CONSTRUCTION. THESE CORNER RECORDS SHALL DESCRIBE THE MONUMENTS FOUND WITH TIE DISTANCES TO REFERENCE POINTS FOR RESETTING OF THE SURVEY MONUMENT. WHEN CONSTRUCTION IS COMPLETED, MONUMENTS SHALL BE SET AND CORNER RECORDS SHALL BE FILED WITH THE COUNTY SURVEYOR SHOWING THE NEW MONUMENTS.

### ennar Communities at Rosena Ranch **&** RACT MAP No. **4** APN: 1116-011-13 & 14 **67 8 ® (5**) **6 6** $\bigcirc$ (3) ۵ **(5**) (a) (7) **(** (<del>5</del>) (8) **(3**) **®** (3) 20250 **(2)** 25 (3) (8) (2) 27 (8) (28) GLEN (3) **(2)** (S) HEL **55** III Z (37) **(22)** PARKWAY (8) **(4)** (36) **(53) (52) (42)** <u>4</u> <u>51</u> න **(£)**

LOT "B"

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SHEET

**4** 

/<del>(4</del>)

**(4)** 

(3)

(2)

(<u>7</u>)

(2)

8

(S)

SHEET

## WATER NOTES

INDEX MAP

SCALE: 1" = 80'

INDICATES UNIT NUMB

BER

- PIPE 10" AND SMALLER SHALL BE CLASS 350 DUCTILE IRON PIPE, OR CLASS 200, OR CLASS 150 10 GA CEMENT MORTAR LINED AND COATED STEEL. PIPE GREATER THAN 12" SHALL BE THICKNESS CLASS 50 DUCTILE IRON PIPE, OR CLASS 150, 10 GA CEMENT MORTAR LINED AND COATED STEEL PIPE OR AS SPECIFIED.
- WATER SERVICE LATERALS SHALL BE TYPE K COPPER LINE, MINIMUM 1" DIAMETER, WITH 1" × ¾" ANGLE VALVES WITH LOCK WING OR 1"×1" ANGLE VALVES WITH LOCK WING.
  THERE SHALL BE A SEPARATE SERVICE FOR EACH LOT BEING SERVED. ONE SERVICE PER PIPE TRENCH. ALL SERVICE VALVES SHALL BE CAPABLE OF 360 DEGREE TURN (less stop).
- ALL WATER SERVICE LATERALS TO BE INSTALLED AT SAME TIME AS MAIN LINE. NO SPLICE SHALL BE ALLOWED ON COPPER SERVICE LATERAL LINE.
- WATER SERVICE LATERALS TO BE TERMINATED 12" BEHIND REAR OF CURB OR FUTURE CURB IN CITY LIMITS, TERMINATE 12" BEHIND FUTURE SIDEWALK.
- DEPTH OF COVER FOR WATER SERVICE LATERALS SHALL BE MINIMUM 30"; FOR WATER MAINS 10" AND SMALLER SHALL BE MINIMUM 36"; FOR 12" AND LARGER PIPE SHALL BE MINIMUM 42" OR AS SPECIFIED ON PLANS. ALL MEASUREMENTS FROM FINISHED GRADE. FIRE HYDRANTS SHALL BE 6"  $\times$  4"  $\times$  2 ½" - CLOW F-850, OR EQUAL, PAINTED WITH ONE COAT PRIMER AND ONE COAT YELLOW. THE 4" STEAMER OUTLET SHALL BE PERPENDICULAR TO THE CURB OR FUTURE CURB.
- ALL WATER MAINS SHALL BE FLUSHED AND DISINFECTED PER AWWA STANDARD C651 PRIOR TO USE AFTER INSTALLATION OR REPAIR.
- CONSTRUCTION TO BE IN ACCORDANCE WITH THE WEST VALLEY WATER DISTRICT "STANDARDS FOR DOMESTIC WATER FACILITIES."
- WATER SERVICE IS SUBJECT TO THE CURRENT DISTRICT RULES AND REGULATIONS AND ANY AMENDMENTS THERETO.

- WATER LINES TO BE CONSTRUCTED ONLY AFTER THE CONSTRUCTION OF ND GUTTER.

# IF CONSTRUCTION HAS NOT COMMENCED WITHIN TWO YEARS OF THE DISTRICT APPROVAL DATE, THIS PLAN SHALL BE RESUBMITTED TO THE DISTRICT FOR REVIEW AND APPROVAL.

- CONTRACTOR TO FURNISH 2-YEAR WARRANTY BOND FOR ALL WATER FACILITIES INSTALLED WITH THIS PLAN.

# DESIGN CERTIFICATION

THIS CERTIFIES THAT THESE PLANS AND SPECIFICATIONS HAVE BEEN DESIGNED UNDER THE DIRECT SUPERVISION OF A CIVIL ENGINEER LICENSED IN THE STATE OF CALIFORNIA AND ARE IN ACCORDANCE WITH TITLE 22 CODE OF REGULATIONS CHAPTER 16 CALIFORNIA WATERWORK STANDARD OF THE STATE OF CALIFORNIA.

ROBERT R. OTTE, RCE 44120, Expire

WATER CERTIFICATION

THIS CERTIFIES THAT THESE PLANS AND SPECIFICATIONS HAVE BEEN REVIEWED BY AND ARE ACCEPTED BY THE WEST VALLEY WATER DISTRICT AND THAT THIS DISTRICT IS WILLING AND ABLE TO SUPPLY WATER TO THIS LOCATION.

SIGNATURE TITE

FIRE PROTECTION SYSTEM APPROVED FIRE CERTIFICATION

SIGNATURE OF COUNTY FIRE WARDEN

LENNAR HOMES of CA, Inc. SOILS ENGINEER 40880-R COUNTY CENTER DRIVE TEMECULA, CALIFORNIA 92591 PETRA GEOTECHNICAL, Inc.

OWNER

980 MONTECITO DRIVE #302 CORONA, CALIFORNIA 92879

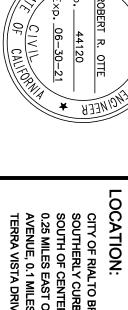
CONTACT: Mr. BLAINE HUMBLES (951) 817-3582

CONTACT: Mr. DOUG JOHNSTON, CEG (951) 600-9271

WATER IMPROVEMENT PLANS Tract Map No. 20205 TITLE SHEET Rosena Ranch DRAWING D20002 / PRESSURE ZONE: 8 ROAD No. FILE No.

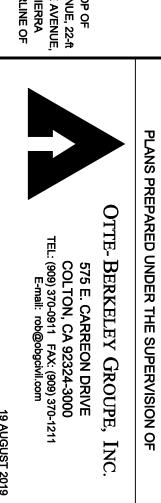
Underground Service Alert Call: TOLL FREE 227-2600 1-800

TWO WORKING DAYS BEFORE YOU DIG



BENCHMARK: 042-88 (Rialto)





FIELD

**BOOK NOTES** 

1924.262 (04/01/89)

DRAWN BY
R. OTTE

NED BY R. OTTE

**WEST** 

/ALLEY WATER DISTRICT

APPROVED BY

DATE

This plan is copyrig

Packet Pg. 255

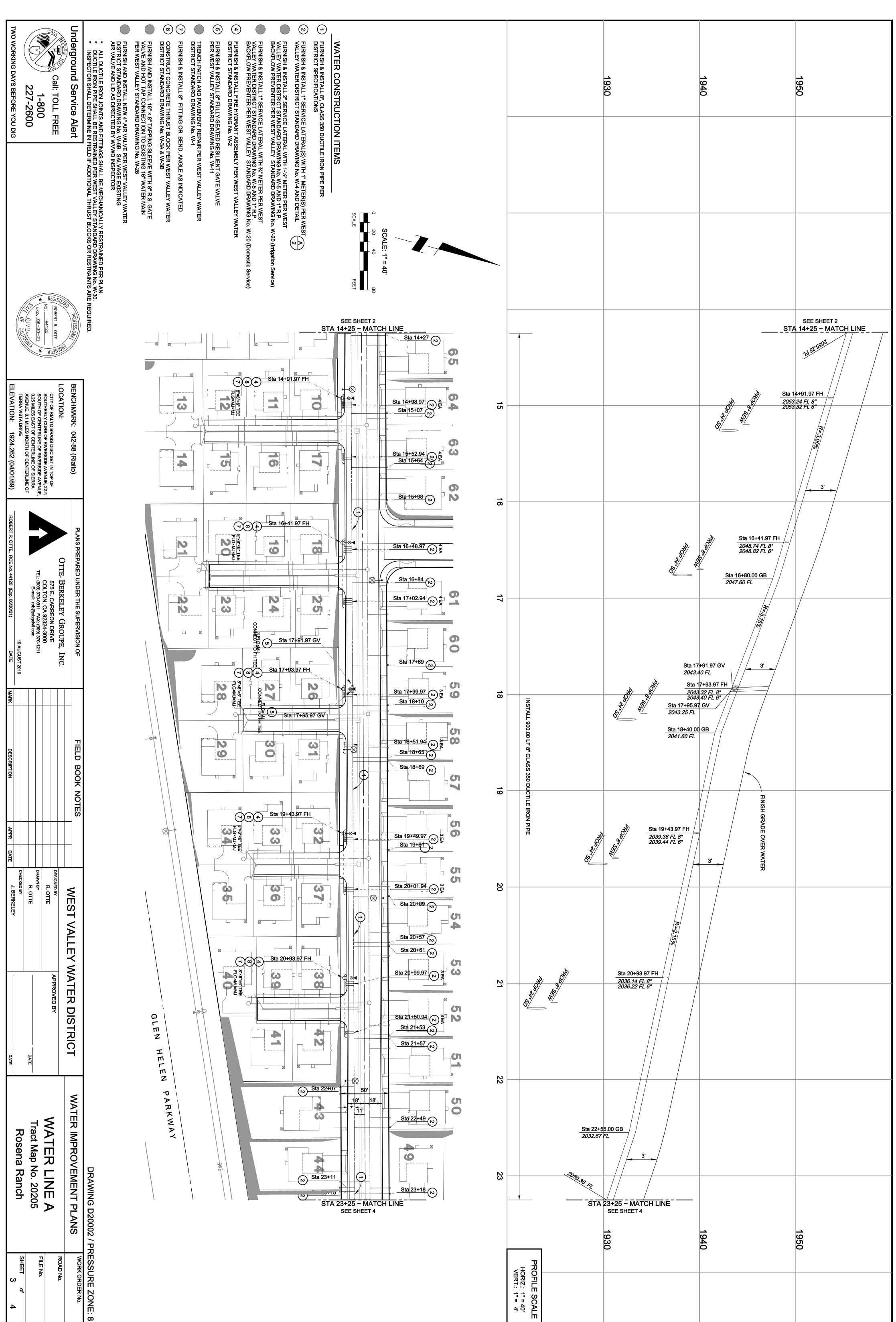
Underground Service Alert TWO WORKING DAYS BEFORE YOU DIG **©** @ (7) 6 4 ω (2) <u>-</u> FURNISH & INSTALL 1" SERVICE LATERAL(S) WITH 1" METER(S) PER WEST A VALLEY WATER DISTRICT STANDARD DRAWING No. W-4 AND DETAIL  $\frac{A}{2}$ FURNISH & INSTALL 8", CLASS 350 DUCTILE IRON PIPE PER DISTRICT SPECIFICATIONS FURNISH AND INSTALL NEW 4" AIR VALVE PER WEST VALLEY WATER DISTRICT STANDARD DRAWING No. W-6B. SALVAGE EXISTING AIR VALVE AND LID AS DIRECTED BY WVWD INSPECTOR CONSTRUCT CONCRETE THRUST BLOCK PER WEST VALLEY WATER DISTRICT STANDARD DRAWING No. W-3A & W-3B TRENCH PATCH AND PAVEMENT REPAIR PER WEST VALLEY WATER DISTRICT STANDARD DRAWING No. W-1 FURNISH & INSTALL 8" FULLY-SEATED RESILIENT GATE VALVE PER WEST VALLEY STANDARD DRAWING No. W-11 FURNISH & INSTALL FIRE HYDRANT ASSEMBLY PER WEST VALLEY WATER DISTRICT STANDARD DRAWING No. W-2 FURNISH & INSTALL 1" SERVICE LATERAL WITH ¾" METER PER WEST VALLEY WATER DISTRICT STANDARD DRAWING No. W-5 AND 1" R.P. BACKFLOW PREVENTER PER WEST VALLEY STANDARD DRAWING No. W-20 (Do FURNISH AND INSTALL 16" × 8" TAPPING SLEEVE WITH 8" R.S. GATE VALVE AND HOT TAP CONNECTION TO EXISTING 16" WATER MAIN PER WEST VALLEY STANDARD DRAWING No. W-28 FURNISH & INSTALL 8" FITTING OR BEND, ANGLE AS INDICATED FURNISH & INSTALL 2" SERVICE LATERAL WITH 1-½" METER PER WEST VALLEY WATER DISTRICT STANDARD DRAWING No. W-5 AND 1" R.P. BACKFLOW PREVENTER PER WEST VALLEY STANDARD DRAWING No. W-20 (Irriga WATER CONSTRUCTION ITEMS ALL DUCTILE IRON JOINTS AND FITTINGS SHALL BE MECHANICALLY RESTRAINED PER PLAN. DUCTILE IRON PIPE SHALL BE RESTRAINED PER WEST VALLEY STANDARD DRAWING No. W-30. INSPECTOR SHALL DETERMINE IN FIELD IF ADDITIONAL THRUST BLOCKS OR RESTRAINTS ARE REQUIRED. Call: TOLL FREE 2065 SCALE: 1" = 40' 2045 2055 1-800 227-2600 OF CALIFORN 44120 06-30-21 SYCAMORE CREEK DRIVE CAUTION: EXISTING ELECTRICAL CONDUIT: PROTECT IN PLACE EXISTING 16" AND 24" WATERLINES PER
DISTRICT DWG. No. W4-1315 BENCHMARK: 042. LOCATION: CITY OF RIALTO BRASS DISC SET IN TOP OF SOUTHERLY CURB OF RIVERSIDE AVENUE, 22-ft SOUTH OF CENTERLINE OF RIVERSIDE AVENUE, 0.25 MILES EAST OF CENTERLINE OF SIERRA AVENUE, 0.1 MILES NORTH OF CENTERLINE OF TERRA VISTA DRIVE 1924.262 (04/01/89) -88 (Rialto) GLEN HELEN PARKWAY (10) 2 EA 000 Sta 10+00.00 JOIN 10 (2057.65 FL 16") 2057.98 FL 8" OTTE-BERKELEY GROUPE, INC. VERIFY LOCATION BEFORE (9)
COMMENCING CONSTRUCTION. 575 E. CARREON DRIVE COLTON, CA 92324-3000 L: (909) 370-0911 FAX: (909) 370-1211 E-mail: rob@obgcivil.com UNDER THE SUPERVISION OF CONTRACTOR SHALL POTHOLE TO VERIFY LOCATION & ELEVATION OF EXISTING WATER LINE PRIOR TO COMMENCING CONSTRUCTION. **=**  $\bigcirc$ (0) 5' Ø, ထ္ 12 350 DUCTILE FIELD IRON PIPE Sta 12+65.47 Sta 12+75.47 ANG PNT BOOK 2056.88 FL MATCH LINE <del>1</del>3 NOTES Sta 12+75.47 Ang. Pnt 7 Sta 13+48.97(2) 4 EA Sta 14+03.94 2 5 EA Sta 13+41.97 FH (4) 8 7 Sta 13+48.97 2 EA 6 7 2 Sta 13+63 Sta 14+23 (2) Sta 12+87 2 Sta 12+89 (0) Sta 13+41.97 FH 2056.62 FL 8" 2056.70 FL 6" 0 Sta 13+85.00 GB (1) 2056.45 FL NED BY R. OTTE 4 R. OTTE **WEST** 3' STA 14+25 ~ MATCH LINE **/ALLEY WATER DISTRICT** WATER METER DETAIL # EA. AS NOTED

\* STA SHOWN F
CENTER OF TH

METER BO.
STANDARD WATER IMPROVEMENT PLANS \* STA SHOWN REPRESENTS CENTER OF THE CLUSTER METER BOX PER DISTRICT STANDARD W-4 WATER LINE A Tract Map No. 20205 Rosena Ranch DRAWING D20002 / PRESSURE ZONE: 8 2045 2055 2065 ROAD No. PROFILE SCALE HORIZ.: 1" = 40' VERT.: 1" = 4'

y Otte-Berkeley Groupe, Inc. - 2019

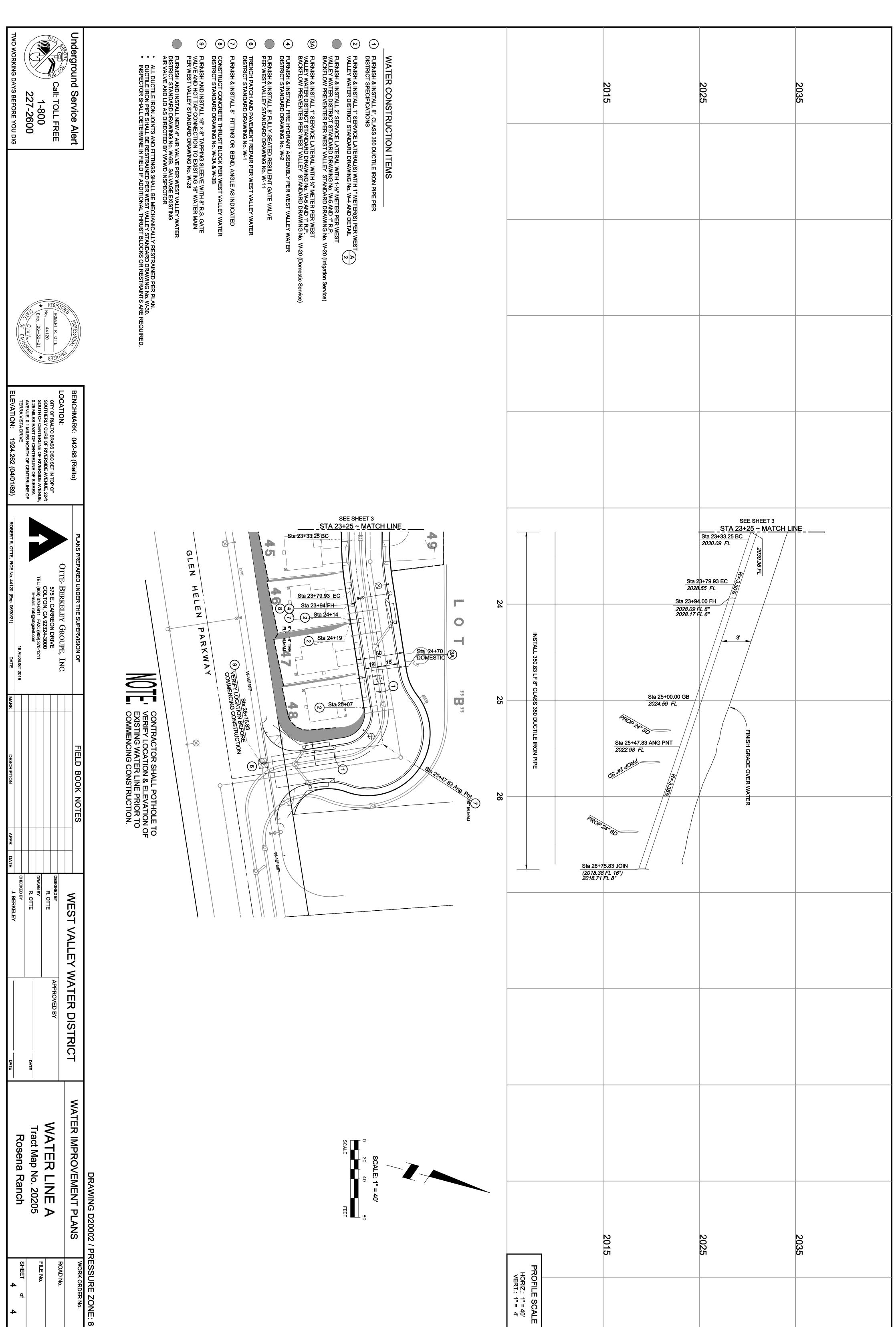
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### Exhibit C

### 9/26/2019

### **OPINION OF PROBABLE COST**

Prepared by Otte-Berkeley Groupe, Inc.

### **WATER IMPROVEMENTS**

Item	Description	Quantity	Unit	Unit \$	Total \$
3.0 CONS	TRUCTION ITEMS				
3.01	Install 8" Class 350 Ductile Iron Pipe	1,676	LF	\$90.00	\$150,840.00
3.02	Install 1" Water Service Lateral with 1" Water Meter per West Valley Water District Standard Drawing No. W-4	78	EA	\$1,500.00	\$117,000.00
3.03	Install 2" Water Service Lateral with 1½" Water Meter per West Valley Water District Standard Drawing No. W-5 & 1" Reduced Pressure Backflow Preventer per West Valley Water District Standard Drawing No. W-20 (Irrig. Service)	1	EA	\$5,000.00	\$5,000.00
3.03A	Install 1" Water Service Lateral with 3/4" Water Meter per West Valley Water District Standard Drawing No. W-5 & 1" Reduced Pressure Backflow Preventor per West Valley Water District Standard Drawing No. W-20 (Domm. Service)	1	EA	\$4,000.00	\$4,000.00
3.04	Install Fire Hydrant Assembly per West Valley Water District Standard Drawing No. W-2	7	EA	\$2,300.00	\$16,100.00
3.05	Install 8" Fully-Seated Resilient Flanged Gate Valve	2	EA	\$1,200.00	\$2,400.00
3.06	Construct Trench Patch and Pavement Repair per West Valley Water District Standard Drawing No. W-1	360	SF	\$10.00	\$3,600.00
3.07	Install Fitting or Bend, Angle as Indicated on Plan	9	EA	\$300.00	\$2,700.00
3.08	Construct Concrete Thrust Block per West Valley Water District Standard Drawing Nos. W-3A & W-3B	11	EA	\$100.00	\$1,100.00
3.09	Install 16' $\times$ 8" tapping outlet with 8" R.S. Gate Valve and Hot Tap connection to existing 16" Water Main	2	EA	\$5,000.00	\$10,000.00
3.10	Install 4" Air Valve Assembly per West Valley Water District Standard Drawing No. W-6B	2	EA	\$3,500.00	\$7,000.00
		Construction Total Clean up		\$319,740.00 \$3,197.40	
		Sub Total Contingency at 20%		\$322,937.40 \$64,587.48	
		Total			\$387,524.88

### Exhibit D

### **BOARD OF DIRECTORS**

Dr. Michael Taylor President, Board of Directors **Kyle Crowther** Vice President, Board of Directors Dr. Clifford O. Young, Sr. Director **Greg Young** Director **Donald Olinger** Director

Crystal L. Escalera

**Board Secretary** 



### ESTABLISHED AS A PUBLIC AGENCY IN 1952

WEST VALLEY WATER DISTRICT'S MISSION IS TO PROVIDE A RELIABLE, SAFE-DRINKING WATER SUPPLY TO MEET OUR CUSTOMERS' PRESENT AND FUTURE NEEDS AT A REASONABLE COST AND TO PROMOTE WATER-USE EFFICIENCY AND CONSERVATION.

ADMINISTRAT

3.h.b Clarence C. Mansell, Jr. General Manager Ricardo Pacheco

Assistant General Manager Jeremiah Brosowske Assistant General Manager

Deborah L. Martinez Human Resources and Risk Manager

Shamindra K. Manbahal Chief Financial Officer

### 2018 HOLIDAY LIST

CHRISTMAS EVE MONDAY, DECEMBER 24 TUESDAY, DECEMBER 25 **CHRISTMAS** MONDAY, DECEMBER 31 **NEW YEAR'S EVE** 

### **2019 HOLIDAY LIST**

TUESDAY, JANUARY 1 **NEW YEAR'S DAY** 

MONDAY, JANUARY 21 MARTIN LUTHER KING, JR.

MONDAY, FEBRUARY 18 PRESIDENT'S DAY

MONDAY, MAY 27 **MEMORIAL DAY** 

INDEPENDENCE DAY THURSDAY, JULY 4

MONDAY, SEPTEMBER 2 LABOR DAY

**VETERANS DAY (OBSERVED)** MONDAY, NOVEMBER 11

THANKSGIVING THURSDAY, NOVEMBER 28

FRIDAY, NOVEMBER 29 DAY AFTER THANKSGIVING

TUESDAY, DECEMBER 24 CHRISTMAS EVE

WEDNESDAY, DECEMBER 25 **CHRISTMAS** 

TUESDAY, DECEMBER 31 **NEW YEAR'S EVE** 

### **2020 HOLIDAY LIST**

WEDNESDAY, JANUARY 1 **NEW YEAR'S DAY** MONDAY, JANUARY 20 MARTIN LUTHER KING, JR.